June 2, 2017

Mr. Craig Dixon

Chairman
New Bedford Conservation Commission
New Bedford City Hall
133 Williams Street
New Bedford, MA 02744

# RE: Response Letter <br> NWD Trucking - 100 Duchaine Blvd. <br> New Bedford, Massachusetts 

Dear Mr. Nixon,
We have enclosed a response letter, revised HydroCAD calculations, revised Site Plan sheets and accompanying documentation in response to the comment letter prepared by Nitsch Engineering dated April 28, 2017 in regards to their review of the Site Plans and attached documents.

We trust the attachments noted above and included herewith will provide the necessary documentation to address their comments. If you should have any questions, please feel free to contact us.

Very Truly Yours,
FARLAND CORPORATION, INC.

## Chistian A. Farland

Christian A. Farland , P.E., LEED AP Principal Engineer and President

## Nitsch Engineering Comments

## Comment \#1:

Test holes were provided at the locations of the infiltration facilities. The test hole performed in the vicinity of the underground infiltration facility indicates seasonal high groundwater at approximately elevation 78.3. This elevation seems consistent with the surrounding wetlands elevations. The calculations and detail show the bottom of the system at elevation 78.5. Per the Standards, 2 feet of separation between the bottom of the infiltration facility and seasonal high groundwater is required.

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Farland Corp. has designed this system to meet the maximum extent
practical for this project.
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## Comment \#2:

A CDS water quality unit was added to the plans to treat the water generated by the existing parking lot. The Applicant has not provided sizing information for this unit. Also, the unit was placed outside the existing parking lot adjacent to the wetlands pocket on the south side of the site. The unit is well within 25 feet of the wetlands line. A detail of this unit should be added to the plans.

## A detail of the CDS water quality unit has been added to the Details Sheet 8 along with a cut sheet of the specific model.

## Comment \#3:

The proposed discharge from the CDS unit, including rip-rap pads, should be shown on the plans. The current plan does not show a discharge pipe or rip-rap pad.

The CDS unit will be placed along an existing drain line with the slope and inverts of the pipe to remain the same.

## Comment \#4:

Revised hydrologic calculations were submitted to include the reaches and ponds. With regards to the calculations we have the following comments:
a.) Reach $P$-1 is modeled as a 20-inch pipe. The pipe is shown as an 18-inch pipe on the existing conditions plans and is full during all existing storms. The pipe should not be improved. However, this restriction will impact the modeling of the system downstream and should be modeled correctly.
b.) Reach P-2 is modeled as a 12-inch culvert although the existing conditions plans show it as an 18-inch culvert.
c.) The existing conditions hydrologic calculations show the pipe flowing from CB-9 surcharged during the 10-year storm in the existing and proposed condition.
d.) In the proposed condition plans, stormwater from drainage areas S-1 and S-2 are directed towards the proposed detention basin on the west side of the parking lot. The calculations should be revised to direct that flow through the detention basin.
e.) It is unclear whether water collected by CB-2 in the calculations (CB-3 in the plans) is routed properly. The calculations show this catch basin being discharged to the southerly wetland, but the culvert that this catch basin is connected to in the existing condition has been removed.
f.) The proposed conditions hydrologic calculations show the pipes in the parking area discharging from catch basins 8 and 9 to be surcharged during the 2-year storm.
g.) There are errors in the hydrologic model associated with the southerly and northerly wetlands, apparently because of the surcharged piping upstream.
a.) Reach P-1 is in fact an $18^{\prime \prime}$ corrugated steel pipe and the HydroCAD calculations have been revised to reflect this.
b.) Reach P-2 exists as a $12^{\circ}$ pipe and has been revised in the existing conditions plan to reflect this.
c.) The pipe flowing from CB-9 will be upgraded to a size that can handle the expected flow of a 10-yr storm.
d.) Calculations for drainage areas S-1 and S-2 have been re-directed to the correct detention basin.
e.) The catch basin has been re-titied CB-2 in the plans to remain consistent with the calculations. CB-2 connects to an existing 120 corrugated steel pipe that directs the flow to the southerly wetland as described, and will remain as it exists.
f.) The pipes within the parking lot which collect and discharge flow from CB-8 and CB-9 will be upgraded to a size that is appropriate for the post-development conditions.
g.) The surcharged piping upstream has been upgraded and will help clear up any errors found within the modeling.

## Comment \#5:

It is unclear where the water from CB-3 will be directed under the proposed condition. The existing condition plan shows it connecting to a culvert that appears to be removed. The Applicant should clarify where water captured by this catch basin will be discharged.

> CB-2 (formally CB-3 in the plans) currently connects to a $12^{\prime \prime}$ steel pipe which will remain, and directs stormwater to water quality basin \#2, which then outlets to the southerly wetland as described.

## Comment \#6:

We recommend that the seasonal high groundwater elevation be added to the infiltration field detail

## Seasonal high groundwater has been added to the infiltration field detail.

## Comment \#7:

We recommend a stone overflow be added from the forebay of water quality basin 1 .

## A stone overflow has been added as recommended.

## Comment \#8:

Consistent with the Standards, we recommend that a foot of freeboard be provided between the 100-year storm peak elevation and/or overflow from the basins and the top of berm elevation in the basins.

> Farland Corp. has designed the top of berm elevations to meet the maximum extent practical for this project.

## Comment \#9:

Pipe sizing calculations were prepared using the Rational Method. However, the results of these calculations show all pipes flowing freely, which is not consistent with the hydrologic calculations submitted.
HydrocAD calculations have been revised to reflect suitable
conditions for stormwater treatment.

If you have any questions or require any further information please contact this office at (508) 717-3479.


## Summary for Subcatchment S-1: Tributary to South Culvert

Runoff $=\quad 0.75 \mathrm{cfs} @ 12.14 \mathrm{hrs}$, Volume $=\quad 0.064$ af, Depth= $1.06{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 2-yr Rainfall=3.40"


## Summary for Subcatchment S-10: Tributary toward CB-8

Runoff $=\quad 2.83 \mathrm{cfs} @ 12.08 \mathrm{hrs}$, Volume= $\quad 0.226$ af, Depth= 3.17"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-yr Rainfall=3.40"

|  | Area (sf) | CN |
| :--- | ---: | :--- | Description | * |
| :--- |
| 37,250 |
| 37,250 |$\quad$ Impervious Area $\quad$ Paved Paring


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- | Description | Direct Entry, Min. Tc |
| :--- |

## Summary for Subcatchment S-11: Tributary to Northerly Wetland

Runoff $=1.92$ cfs @ 12.09 hrs, Volume= 0.137 af, Depth= $1.63{ }^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 2-yr Rainfall=3.40"


## Summary for Subcatchment S-2: Tributary to North Culvert

Runoff $=0.81$ cfs @ 12.12 hrs, Volume= $\quad 0.065$ af, Depth= $1.11^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-yr Rainfall=3.40"


## Summary for Subcatchment S-3: Tributary to Water Quality Inlet

Runoff $=\quad 2.03$ cfs @ 12.09 hrs, Volume $=0.145$ af, Depth $=2.26{ }^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}$, $\mathrm{dt}=0.01 \mathrm{hrs}$ Type III 24-hr 2-yr Rainfall=3.40"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 7,500 | 98 | Water Quality Inlet |  |  |
| * | 14,700 | 98 | Roadway |  |  |
|  | 11,350 | 70 | Woods, Good, HSG C |  |  |
|  | 33,550 | 89 | Weighted Average |  |  |
|  | 11,350 |  | Pervious Area Impervious Area |  |  |
|  | 22,200 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{aligned} & \text { Capacity } \\ & \text { (cfs) } \end{aligned}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment S-3a: Tributary to Southerly Wetland

Runoff $=\quad 1.06$ cfs @ 12.09 hrs, Volume $=0.076$ af, Depth= $1.93^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"

|  | Area (sf) | CN | Description |
| :--- | ---: | ---: | :--- |
| $*$ | 9,465 | 98 | Wetland |
| $*$ | 1,527 | 98 | Roadway |
| 9,498 | 70 | Woods, Good, HSG C |  |
| 20,490 | 85 | Weighted Average |  |
| 9,498 |  | Pervious Area |  |
|  | 10,992 |  | Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

## Summary for Subcatchment S-4: Tributary to CB-1

Runoff $=\quad 0.11$ cfs @ 12.08 hrs, Volume= $\quad 0.009$ af, Depth= 3.17"

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 2-yr Rainfall=3.40"


## Summary for Subcatchment S-5: Tributary to CB-2

Runoff $=\quad 0.11$ cfs @ 12.08 hrs, Volume $=\quad 0.008$ af, Depth= 3.17"

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ Type III 24-hr 2-yr Rainfall=3.40"


## Summary for Subcatchment S-6: Tributary Off-Site

Runoff $=1.12$ cfs @ 12.08 hrs, Volume $=\quad 0.087$ af, Depth= $3.06{ }^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 2-yr Rainfall=3.40"

|  | Area (sf) | CN |
| ---: | ---: | :--- | Description $\quad$| $*$ | 14,625 | 98 |
| ---: | ---: | :--- |
| Paved Parking |  |  |
| 335 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 14,960 | 97 | Weighted Average |
| 335 |  | Pervious Area |
| 14,625 |  | Impervious Area |


|  | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $(\min )$ | (feet) | (ft/ft) | (ft/sec) | (cfs) |  |

## Direct Entry, Min. Tc

## Summary for Subcatchment S-7: Tributary toward CB-7

Runoff $=\quad 1.07$ cfs @ 12.08 hrs , Volume $=0.086 \mathrm{af}$, Depth= $3.17^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 2-yr Rainfall=3.40"


## Summary for Subcatchment S-8: Tributary to SRS-2

Runoff $=\quad 2.12$ cfs @ 12.08 hrs, Volume $=\quad 0.170$ af, Depth= $3.17^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ Type III 24-hr 2-yr Rainfall=3.40"

|  | Area (sf) | CN | Description |
| :--- | ---: | :--- | :--- |
| 28,000 | 98 | Rooftop |  |
|  |  | Impervious Area |  |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- | Description | Direct Entry, Min. Tc |
| :--- |

## Summary for Subcatchment S-8a: Tributary toward WQI

Runoff $=0.99$ cfs @ 12.08 hrs, Volume= 0.076 af, Depth= $2.95{ }^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 2-yr Rainfall=3.40"
$\left.\begin{array}{lrrll} & \text { Area (sf) } & \text { CN } & \text { Description } \\ * & 10,000 & 98 & \text { Paved Parking } \\ * & 940 & 74 & >75 \% \text { Grass cover, Good, HSG C } \\ 2,535 & 98 & \text { Water Quality Inlet }\end{array}\right]$

## Summary for Subcatchment S-9: Tributary toward CB-9

Runoff $=3.46$ cfs @ 12.08 hrs , Volume $=\quad 0.276$ af, Depth= $3.17^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 2-yr Rainfall=3.40"

| Area (sf) |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 45,550 | 98 P | aved Park |  |  |
|  | 45,550 |  | pervious | Area |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

## Summary for Reach CB-1: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow Area $=$ | $0.033 \mathrm{ac}, 100.00 \%$ Impervious, Inflow Depth $=3.17 "$ for $2-\mathrm{yr}$ event |  |
| :--- | :--- | :--- |
| Inflow | $=$ | 0.11 cfs @ 12.08 hrs, Volume $=$ |
| Outflow | $=$ | $0.11 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=1.52 \mathrm{fps}$, Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=0.51 \mathrm{fps}$, Avg. Travel Time $=0.5 \mathrm{~min}$
Peak Storage= 1 cf @ 12.09 hrs, Average Depth at Peak Storage= 0.14 '
Bank-Full Depth=1.25', Capacity at Bank-Full= 4.41 cfs
15.0" Diameter Pipe, n= 0.013

Length= 15.0' Slope= 0.0047 '/'
Inlet Invert= 76.37', Outlet Invert= 76.30'


## Summary for Reach CB-2: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=1.38 \mathrm{fps}$, Min. Travel Time $=0.6 \mathrm{~min}$
Avg. Velocity $=0.45 \mathrm{fps}$, Avg. Travel Time $=1.8 \mathrm{~min}$
Peak Storage $=4$ cf @ 12.09 hrs, Average Depth at Peak Storage $=0.15^{\prime}$
Bank-Full Depth= 1.00', Capacity at Bank-Full= 2.05 cfs
12.0" Diameter Pipe, $n=0.025$ Corrugated metal Length=48.0' Slope= 0.0123 '/'
Inlet Invert= 76.09', Outlet Invert= 75.50'


## Summary for Reach CB-7: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow Area = | $0.324 \mathrm{ac}, 100.00 \%$ Impervious, | pth $=3.17$ " for $2-y r$ event |
| :---: | :---: | :---: |
| Inflow | 1.07 cfs @ 12.08 hrs, Volume= | 0.086 af |
| Outflow | 1.07 cfs @ 12.09 hrs , Volume= | 0.086 af, Atten $=0 \%$, Lag $=0.1 \mathrm{~min}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=6.43 \mathrm{fps}$, Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=2.10 \mathrm{fps}$, Avg. Travel Time $=0.5 \mathrm{~min}$
Peak Storage= 11 cf @ 12.09 hrs, Average Depth at Peak Storage= $0.26^{\prime}$
Bank-Full Depth= 1.00', Capacity at Bank-Full= 6.98 cfs
12.0" Diameter Pipe, n= 0.013

Length=66.0' Slope= 0.0383 '/'
Inlet Invert= 78.71', Outlet Invert= 76.18'


## Summary for Reach CB-8: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Max. Velocity $=3.89 \mathrm{fps}$, Min. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=1.51 \mathrm{fps}$, Avg. Travel Time $=0.8 \mathrm{~min}$
Peak Storage= 58 cf @ 12.09 hrs, Average Depth at Peak Storage= 0.68'
Bank-Full Depth= 1.50', Capacity at Bank-Full= 7.18 cfs
18.0" Diameter Pipe, n= 0.013

Length=75.0' Slope= 0.0047 '/'
Inlet Invert= 75.45', Outlet Invert= 75.10'


## Summary for Reach CB-9: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated
[63] Warning: Exceeded Reach CB-8 INLET depth by 0.25 ' @ 12.09 hrs

| Inflow Area $=$ | $3.234 \mathrm{ac}, 80.82 \%$ | Impervious, Inflow Depth $=2.55^{\prime \prime}$ | for 2 -yr event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $6.47 \mathrm{cfs} @$ | 12.09 hrs, Volume $=$ |
| Outflow | $=$ | $6.46 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=4.71 \mathrm{fps}, \mathrm{Min}$. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=1.79 \mathrm{fps}$, Avg. Travel Time $=0.7 \mathrm{~min}$
Peak Storage= 103 cf @ 12.09 hrs, Average Depth at Peak Storage= 1.09'
Bank-Full Depth= 1.50', Capacity at Bank-Full= 7.38 cfs
18.0" Diameter Pipe, n= 0.013

Length= 75.0 ' Slope $=0.0049$ '/'
Inlet Invert= 75.29', Outlet Invert= 74.92'


## Summary for Reach P-1: 18" Culvert

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Max. Velocity= 1.89 fps , Min. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=0.76 \mathrm{fps}$, Avg. Travel Time $=0.8 \mathrm{~min}$
Peak Storage= 15 cf @ 12.14 hrs, Average Depth at Peak Storage= 0.42'
Bank-Full Depth= 1.50', Capacity at Bank-Full= 4.49 cfs
18.0" Diameter Pipe, $n=0.025$ Corrugated metal

Length= 37.0' Slope= 0.0068 '/'
Inlet Invert= 84.57', Outlet Invert= 84.32'


## Summary for Reach P-2: 12" Culvert

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=3.62 \mathrm{fps}$, Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=1.44 \mathrm{fps}$, Avg. Travel Time $=0.5 \mathrm{~min}$
Peak Storage $=9$ cf @ 12.12 hrs, Average Depth at Peak Storage $=0.33^{\prime}$
Bank-Full Depth= 1.00', Capacity at Bank-Full= 3.49 cfs
12.0" Diameter Pipe, $\mathrm{n}=0.025$ Corrugated metal

Length=42.0' Slope= 0.0355 '/'
Inlet Invert= 84.18', Outlet Invert= 82.69'


## Summary for Pond SRS-2: Subsurface Recharge System

| Inflow Area | $0.643 \mathrm{ac}, 100.00 \%$ Impervious, Inflow Depth = 3.17" for 2-yr event |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 2.12 cfs @ | 12.08 hrs , Volume= | 0.170 af |  |
| Outflow | 1.01 cfs @ | 12.24 hrs , Volume= | 0.108 af , A | Atten= 52\%, Lag= 9.2 min |
| Discarded | 0.01 cfs @ | 5.30 hrs , Volume= | 0.030 af |  |
| Primary | 1.00 cfs @ | 12.24 hrs , Volume= | 0.078 af |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs $/ 3$
Peak Elev= 80.82' @ 12.24 hrs Surf.Area= 2,074 sf Storage= 3,437 cf
Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time $=127.7$ min (882.8-755.1)


Discarded OutFlow Max=0.01 cfs @ 5.30 hrs HW=78.54' (Free Discharge)
-1=Exfiltration (Exfiltration Controls 0.01 cfs )
Primary OutFlow Max=1.00 cfs @ 12.24 hrs HW=80.82' (Free Discharge)
—2=Culvert (Barrel Controls 1.00 cfs @ 2.13 fps )

## Summary for Pond WET-1: Sortherly Wetland

[62] Warning: Exceeded Reach CB-2 OUTLET depth by 0.29 @ 12.35 hrs
[62] Warning: Exceeded Reach CB-9 OUTLET depth by 0.35 @ 12.56 hrs


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 75.88' @ 12.31 hrs Surf.Area= 8,003 sf Storage= 10,551 cf
Plug-Flow detention time $=108.1 \mathrm{~min}$ calculated for 0.867 af ( $94 \%$ of inflow)
Center-of-Mass det. time= $77.1 \mathrm{~min}(870.4-793.3)$


Discarded OutFlow Max=0.05 cfs @ 12.31 hrs HW=75.88' (Free Discharge)
—1=Exfiltration (Exfiltration Controls 0.05 cfs )
Primary OutFlow Max=4.60 cfs @ 12.31 hrs HW=75.88' (Free Discharge)
—2=Culvert (Barrel Controls 4.60 cfs @ 2.99 fps )

## Summary for Pond WET-2: Northerly Wetland

[87] Warning: Oscillations may require Finer Routing or smaller dt
[62] Warning: Exceeded Reach CB-7 OUTLET depth by 0.02' @ 12.59 hrs

| Inflow Area | $1.333 \mathrm{ac}, 53.47 \%$ Impervious, Inflow Depth = 2.00" for 2-yr event |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 2.99 cfs @ | 12.09 hrs , Volume= | 0.222 af |  |
| Outflow | 0.93 cfs @ | 12.51 hrs , Volume= | 0.222 af, | Atten $=69 \%$, Lag $=25.5 \mathrm{~min}$ |
| Discarded | 0.06 cfs @ | 12.43 hrs , Volume= | 0.037 af |  |
| Primary | 0.87 cfs @ | 12.51 hrs, Volume= | 0.185 af |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Peak Elev= 76.32' @ 12.43 hrs Surf.Area= 9,103 sf Storage $=2,793$ cf
Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time $=29.7 \min (835.0-805.4)$


Discarded OutFlow Max=0.06 cfs @ 12.43 hrs HW=76.32' (Free Discharge)
—2=Exfiltration (Exfiltration Controls 0.06 cfs )
Primary OutFlow Max=0.87 cfs @ 12.51 hrs HW=76.32' TW=75.90' (Dynamic Tailwater)

- $_{1=\text { Culvert }}$ (Outlet Controls 0.87 cfs @ 1.93 fps )


## Summary for Pond WQI-1: Water Quality Inlet



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 78.00 | 5,080 | 0 | 0 |
| 79.00 | 6,700 | 5,890 | 5,890 |
| 79.50 | 7,500 | 3,550 | 9,440 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Discarded | $78.00{ }^{\prime}$ | $0.270 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area |
| \#2 | Primary | 79.00' | 10.0' long x 10.0' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .001 .201 .401 .60 |
|  |  |  | Coef. (English) 2.492 .562 .702 .692 .682 .692 .672 .64 |

Discarded OutFlow Max=0.04 cfs @ 18.00 hrs HW=78.77' (Free Discharge)
_1=Exfiltration (Exfiltration Controls 0.04 cfs )
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=78.00' TW=74.00' (Dynamic Tailwater)
L-2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

## Summary for Pond WQI-2: Water Quality Inlet



Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Peak Elev=77.91' @ 13.23 hrs Surf.Area= 2,478 sf Storage $=1,990 \mathrm{cf}$
Plug-Flow detention time $=371.9$ min calculated for 0.040 af ( $52 \%$ of inflow)
Center-of-Mass det. time $=257.2 \min (1,029.6-772.3)$


Discarded OutFlow Max=0.02 cfs @ 13.23 hrs HW=77.91' (Free Discharge)
-1=Exfiltration (Exfiltration Controls 0.02 cfs )
Primary OutFlow Max=0.06 cfs @ 13.23 hrs HW=77.91' TW=75.42' (Dynamic Tailwater)
L2=Broad-Crested Rectangular Weir (Weir Controls 0.06 cfs @ 0.26 fps )

## Summary for Subcatchment S-1: Tributary to South Culvert

Runoff $=\quad 1.54$ cfs @ 12.13 hrs, Volume $=0.124$ af, Depth= $2.05^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 10-yr Rainfall=4.80"

|  | Area (sf) | CN | Description |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 25,975 | 70 | Woods, Good, HSG C |  |  |  |
|  | 3,300 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |  |
| * | 2,300 | 98 | Roadway |  |  |  |
|  | 31,575 | 72 | Weighted Average |  |  |  |
|  | 29,275 |  | Pervious Area |  |  |  |
|  | 2,300 |  | Impervious Area |  |  |  |
| Tc (min) | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |  |
| 5.6 | 50 | 0.1360 | 0.15 |  | Sheet Flow, |  |
|  |  |  |  |  | Woods: Light underbrush $\mathrm{n}=0.400$ | $\mathrm{P} 2=3.40$ |
| 3.5 | 220 | 0.0430 | 1.04 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |  |
| 9.1 | 270 | Total |  |  |  |  |

## Summary for Subcatchment S-10: Tributary toward CB-8

Runoff $=\quad 4.01 \mathrm{cfs} @ 12.08 \mathrm{hrs}$, Volume $=\quad 0.325 \mathrm{af}$, Depth $=4.56{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-yr Rainfall=4.80"

|  | Area (sf) | CN | Description |
| :--- | ---: | :--- | :--- |
| 37,250 98 Paved Parking <br> 37,250  Impervious Area |  |  |  |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- | Description | Direct Entry, Min. Tc |
| :--- |

## Summary for Subcatchment S-11: Tributary to Northerly Wetland

Runoff $=\quad 3.32$ cfs @ 12.09 hrs, Volume $=\quad 0.236$ af, Depth= $2.81{ }^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 10-yr Rainfall=4.80"

|  | Area (sf) | CN D | Description |  |
| :---: | :---: | :---: | :---: | :---: |
|  | 1,175 | 98 R | Roadway |  |
|  | 15,750 | 98 W | Wetland |  |
|  | 27,025 | 70 W | Woods, Good, HSG C |  |
|  | 43,950 | 81 W | Weighted Average Pervious Area |  |
|  | 27,025 |  |  |  |
|  | 16,925 |  | Impervious Area |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) $\begin{array}{r}\text { Capacity } \\ \text { (cfs) }\end{array}$ | Description |
| 0.2 | 14 | 0.0200 | 0.96 | Sheet Flow, |
|  |  |  |  | Smooth surfaces $\mathrm{n}=0.011 \mathrm{P} 2=3.40$ ' |
| 4.7 | 36 | 0.1100 | 0.13 | Sheet Flow, <br> Woods: Light underbrush $n=0.400 \quad \mathrm{P} 2=3.40 "$ |
| 1.1 | 70 | 0.0420 | - 1.02 | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 6.0 | 120 | Total |  |  |

## Summary for Subcatchment S-2: Tributary to North Culvert

Runoff $=\quad 1.62$ cfs @ 12.12 hrs, Volume= $\quad 0.125$ af, Depth= $2.12^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-yr Rainfall=4.80"


## Summary for Subcatchment S-3: Tributary to Water Quality Inlet

Runoff $=3.15$ cfs @ 12.09 hrs, Volume= $\quad 0.230$ af, Depth= $3.58{ }^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}$, $\mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 10-yr Rainfall=4.80"


## Summary for Subcatchment S-3a: Tributary to Southerly Wetland

Runoff $=\quad 1.74$ cfs @ 12.09 hrs, Volume $=\quad 0.125$ af, Depth= $3.18^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.80"

|  | Area (sf) | CN | Description |
| :--- | ---: | ---: | :--- |
| $*$ | 9,465 | 98 | Wetland |
| $*$ | 1,527 | 98 | Roadway |
| 9,498 | 70 | Woods, Good, HSG C |  |
|  | 20,490 | 85 | Weighted Average |
| 9,498 |  | Pervious Area |  |
|  | 10,992 |  | Impervious Area |



## Summary for Subcatchment S-4: Tributary to CB-1

Runoff $=0.16$ cfs @ 12.08 hrs, Volume $=\quad 0.013$ af, Depth $=4.56{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 10-yr Rainfall=4.80"


## Summary for Subcatchment S-5: Tributary to CB-2

Runoff $=0.15 \mathrm{cfs} @ 12.08 \mathrm{hrs}$, Volume $=\quad 0.012 \mathrm{af}$, Depth $=4.56{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 10-yr Rainfall=4.80"

| Area (sf) | CN | Description |  |
| ---: | ---: | :--- | :--- |
| 1,400 | 98 | Paved parking \& roofs |  |
| 1,400 |  | Impervious Area |  |
| Tc  Length <br> (min) Slope <br> (feet) Velocity <br> (ft/ft) <br> (ft/sec)     | Capacity <br> (cfs) | Description |  |

## Summary for Subcatchment S-6: Tributary Off-Site

Runoff $=1.60$ cfs @ 12.08 hrs, Volume $=\quad 0.127$ af, Depth $=4.45{ }^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 10-yr Rainfall=4.80"

|  | Area (sf) | CN | Description |
| :---: | :---: | :---: | :---: |
| * | 14,625 | 98 | Paved Parking |
|  | 335 | 74 | >75\% Grass cover, Good, HSG C |
|  | 14,960 | 97 | Weighted Average |
|  | 335 |  | Pervious Area |
|  | 14,625 |  | Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- | Description | Direct Entry, Min. Tc |
| :--- |

## Summary for Subcatchment S-7: Tributary toward CB-7

Runoff $=1.52$ cfs @ 12.08 hrs , Volume $=0.123 \mathrm{af}$, Depth= $4.56^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 10-yr Rainfall=4.80"

| Area (sf) CN Description |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 14,125 | 88 Paved Parking |  |  |  |
|  | 14,125 | Impervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |

## Summary for Subcatchment S-8: Tributary to SRS-2

Runoff $=\quad 3.02$ cfs @ 12.08 hrs, Volume $=\quad 0.244$ af, Depth= 4.56"

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 10-yr Rainfall=4.80"

|  | Area (sf) | CN | Description |
| :--- | ---: | :--- | :--- |
| 28,000 | 98 | Rooftop |  |
|  |  | Impervious Area |  |


| Tc <br> $(\mathrm{min})$ | Length <br> (feet) | Slope <br> (ft/ft) | Velocity <br> (ft/sec) | Capacity <br> (cfs) |
| ---: | ---: | ---: | ---: | :--- | Description | Direct Entry, Min. Tc |
| :--- |

## Summary for Subcatchment S-8a: Tributary toward WQI

Runoff $=1.43$ cfs @ 12.08 hrs, Volume $=0.112$ af, Depth= 4.33"
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 10-yr Rainfall=4.80"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 10,000 | 98 P | Paved Parking |  |  |
|  | 940 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
| * | 2,535 | 98 V | Water Quality Inlet |  |  |
|  | 13,475 | 96 | Weighted Average |  |  |
|  | 940 |  | Pervious AreaImpervious Area |  |  |
|  | 12,535 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity <br> (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment S-9: Tributary toward CB-9

Runoff $=\quad 4.91$ cfs @ 12.08 hrs, Volume= 0.398 af, Depth= 4.56"
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 10-yr Rainfall=4.80"

| Area (sf) |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 45,550 | 98 P | aved Park |  |  |
|  | 45,550 |  | pervious | Area |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

## Summary for Reach CB-1: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow Area $=$ | $0.033 \mathrm{ac}, 100.00 \%$ Impervious, Inflow Depth $=4.56 \mathrm{ln}$ for $10-\mathrm{yr}$ event |  |  |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.16 \mathrm{cfs} @$ | 12.08 hrs, Volume $=$ |
| Outflow | $=$ | $0.16 \mathrm{cfs} @$ | 12.08 hrs , Volume $=$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Max. Velocity $=1.69 \mathrm{fps}$, Min. Travel Time $=0.1 \mathrm{~min}$
Avg. Velocity $=0.56 \mathrm{fps}$, Avg. Travel Time $=0.4 \mathrm{~min}$
Peak Storage $=1$ cf @ 12.08 hrs, Average Depth at Peak Storage $=0.16$ '
Bank-Full Depth=1.25', Capacity at Bank-Full= 4.41 cfs
15.0" Diameter Pipe, n= 0.013

Length= 15.0' Slope= 0.0047 '/'
Inlet Invert= 76.37', Outlet Invert= 76.30'


## Summary for Reach CB-2: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Max. Velocity $=1.53 \mathrm{fps}$, Min. Travel Time $=0.5 \mathrm{~min}$
Avg. Velocity $=0.50 \mathrm{fps}$, Avg. Travel Time $=1.6 \mathrm{~min}$
Peak Storage $=5$ cf @ 12.09 hrs, Average Depth at Peak Storage $=0.18^{\prime}$
Bank-Full Depth= 1.00', Capacity at Bank-Full= 2.05 cfs
12.0" Diameter Pipe, $n=0.025$ Corrugated metal Length=48.0' Slope= 0.0123 '/'
Inlet Invert= 76.09', Outlet Invert= 75.50'


## Summary for Reach CB-7: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Max. Velocity= 7.10 fps , Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=2.34 \mathrm{fps}$, Avg. Travel Time $=0.5 \mathrm{~min}$
Peak Storage= 14 cf @ 12.09 hrs, Average Depth at Peak Storage= 0.32'
Bank-Full Depth= 1.00', Capacity at Bank-Full= 6.98 cfs
12.0" Diameter Pipe, n= 0.013

Length=66.0' Slope= 0.0383 '/'
Inlet Invert= 78.71', Outlet Invert= 76.18'


## Summary for Reach CB-8: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Max. Velocity $=4.26 \mathrm{fps}$, Min. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=1.71 \mathrm{fps}$, Avg. Travel Time $=0.7 \mathrm{~min}$
Peak Storage= 77 cf @ 12.10 hrs, Average Depth at Peak Storage= $0.85^{\prime}$
Bank-Full Depth= 1.50', Capacity at Bank-Full= 7.18 cfs
18.0" Diameter Pipe, n= 0.013

Length= $75.0^{\prime}$ Slope $=0.0047$ '/'
Inlet Invert= 75.45', Outlet Invert= 75.10'


## Summary for Reach CB-9: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated
[55] Hint: Peak inflow is $126 \%$ of Manning's capacity
[76] Warning: Detained 0.012 af (Pond w/culvert advised)
[63] Warning: Exceeded Reach CB-8 INLET depth by 0.64' @ 12.26 hrs

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=4.76 \mathrm{fps}$, Min. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=2.01 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 133 cf @ 12.05 hrs, Average Depth at Peak Storage= 1.50'
Bank-Full Depth= 1.50', Capacity at Bank-Full= 7.38 cfs
18.0" Diameter Pipe, n= 0.013

Length= 75.0' Slope= 0.0049 '/'
Inlet Invert= 75.29', Outlet Invert= 74.92'


## Summary for Reach P-1: 18" Culvert

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow Area $=$ | 0.725 ac, | $7.28 \%$ | Impervious, Inflow Depth $=2.05 "$ for $10-\mathrm{yr}$ event |  |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.54 \mathrm{cfs} @$ | 12.13 hrs, Volume $=$ | 0.124 af |
| Outflow | $=$ | $1.54 \mathrm{cfs} @$ | 12.14 hrs, Volume $=$ | 0.124 af , Atten $=0 \%$, Lag= 0.2 min |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Max. Velocity $=2.30 \mathrm{fps}$, Min. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=0.88 \mathrm{fps}$, Avg. Travel Time $=0.7 \mathrm{~min}$
Peak Storage= 25 cf @ 12.14 hrs, Average Depth at Peak Storage= $0.61^{\prime}$
Bank-Full Depth=1.50', Capacity at Bank-Full= 4.49 cfs
18.0" Diameter Pipe, $\mathrm{n}=0.025$ Corrugated metal

Length= 37.0' Slope= 0.0068 '/'
Inlet Invert= 84.57', Outlet Invert= 84.32'


## Summary for Reach P-2: 12" Culvert

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=4.36 \mathrm{fps}$, Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=1.66 \mathrm{fps}$, Avg. Travel Time $=0.4 \mathrm{~min}$
Peak Storage= 16 cf @ 12.12 hrs, Average Depth at Peak Storage= 0.48'
Bank-Full Depth= 1.00', Capacity at Bank-Full= 3.49 cfs
12.0" Diameter Pipe, $n=0.025$ Corrugated metal Length=42.0' Slope= 0.0355 '/'
Inlet Invert= 84.18', Outlet Invert= 82.69'


## Summary for Pond SRS-2: Subsurface Recharge System

| Inflow Area = | $0.643 \mathrm{ac}, 100.00 \%$ Impervious, Inflow Depth $=4.56$ " for 10-yr event |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 3.02 cfs @ | 12.08 hrs , Volume= | 0.244 af |  |
| Outflow | 2.47 cfs @ | 12.14 hrs , Volume= | 0.182 af, | Atten $=18 \%$, Lag $=3.3 \mathrm{~min}$ |
| Discarded | 0.01 cfs @ | 3.66 hrs, Volume= | 0.031 af |  |
| Primary | 2.46 cfs @ | 12.14 hrs , Volume= | 0.152 af |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$ Peak Elev= 81.09' @ 12.14 hrs Surf.Area= $2,074 \mathrm{sf}$ Storage= $3,834 \mathrm{cf}$

Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time= $98.2 \mathrm{~min}(846.9-748.7)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1 | 78.50' | 1,825 cf | 17.00 'W x 122.00'L x 3.75'H Prismatoid |
|  |  |  | 7,778 cf Overall - 3,215 cf Embedded $=4,562 \mathrm{cf} \times 40.0 \%$ Voids |
| \#2 | 79.00' | 3,215 cf | 52.6"W x 34.0"H x 7.50'L Cultec R-V8x 48 Inside \#1 |
| 5,040 cf Total Available Storage |  |  |  |
| Device | Routing | Invert Outlet Devices |  |
| \#1 | Discarded | 78.50 ' 0.27 | 0.270 in/hr Exfiltration over Surface area |
| \#2 | Primary | 80.50' | $6.0 "$ x 6.0' long Culvert X 5.00 |
|  |  |  | , square edge headwall, $\mathrm{Ke}=0.500$ |
|  |  |  | thvert= 80.44' S=0.0100 '/' Cc= $0.900 \quad \mathrm{n}=0.013$ |

Discarded OutFlow Max=0.01 cfs @ 3.66 hrs HW=78.54' (Free Discharge)
-1=Exfiltration (Exfiltration Controls 0.01 cfs )
Primary OutFlow Max=2.46 cfs @ 12.14 hrs HW=81.09' (Free Discharge)
—2=Culvert (Barrel Controls 2.46 cfs @ 2.67 fps )

## Summary for Pond WET-1: Sortherly Wetland

[61] Hint: Exceeded Reach CB-1 outlet invert by 0.06' @ 12.32 hrs
[63] Warning: Exceeded Reach CB-2 INLET depth by 0.16' @ 12.34 hrs
[63] Warning: Exceeded Reach CB-9 INLET depth by 0.27 @ 12.55 hrs


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Peak Elev= 76.36' @ 12.32 hrs Surf.Area= 9,211 sf Storage= 14,690 cf
Plug-Flow detention time $=82.1 \mathrm{~min}$ calculated for 1.461 af ( $97 \%$ of inflow )
Center-of-Mass det. time $=61.9$ min ( 853.8-791.9)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | 74.00 | $37,115 \mathrm{cf}$ | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 74.00 | 3,200 | 0 | 0 |
| 76.00 | 8,300 | 11,500 | 11,500 |
| 78.00 | 13,300 | 21,600 | 33,100 |
| 78.50 | 2,760 | 4,015 | 37,115 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Discarded | $74.00{ }^{\prime}$ | $0.270 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area |
| \#2 | Primary | $74.57{ }^{\prime}$ | 24.0" $\times$ 60.0' long Culvert CMP, projecting, no headwall, $\mathrm{Ke}=0.900$ |
|  |  |  | Outlet Invert= 74.53' S=0.0007 $/ / \mathrm{lc} \mathrm{l}^{\prime} \mathrm{Cc}=0.900 \mathrm{n}=0.013$ |

Discarded OutFlow Max=0.06 cfs @ 12.32 hrs HW=76.36' (Free Discharge)
1-1=Exfiltration (Exfiltration Controls 0.06 cfs )
Primary OutFlow Max=7.96 cfs @ 12.32 hrs HW=76.36' (Free Discharge)
${ }^{4}-\mathbf{2}=$ Culvert (Barrel Controls 7.96 cfs @ 3.54 fps )

## Summary for Pond WET-2: Northerly Wetland

[87] Warning: Oscillations may require Finer Routing or smaller dt
[62] Warning: Exceeded Reach CB-7 OUTLET depth by 0.19' @ 12.55 hrs

| Inflow Area = | $1.333 \mathrm{ac}, 53.47 \%$ Impervious, Inflow Depth $=3.24$ " for 10-yr event |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 4.84 cfs @ | 12.09 hrs , Volume= | 0.360 af |  |
| Outflow | 1.69 cfs @ | 12.46 hrs , Volume= | 0.360 af, A | Atten= 65\%, Lag= 22.3 min |
| Discarded | 0.06 cfs @ | 12.38 hrs , Volume= | 0.049 af |  |
| Primary | 1.63 cfs @ | 12.46 hrs , Volume= | 0.310 af |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Peak Elev= 76.52' @ 12.38 hrs Surf.Area= 9,612 sf Storage= 4,699 cf
Plug-Flow detention time $=$ (not calculated: outflow precedes inflow)
Center-of-Mass det. time $=33.6 \min (829.8-796.2)$


Discarded OutFlow Max=0.06 cfs @ 12.38 hrs HW=76.52' (Free Discharge)
L2=Exfiltration (Exfiltration Controls 0.06 cfs )
Primary OutFlow Max=1.63 cfs @ 12.46 hrs HW=76.52' TW=76.08' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.63 cfs @ 2.29 fps )

| Inflow Area = | 0.770 ac, $66.17 \%$ Impervious, Inflow Depth $=3.58$ " for 10-yr event |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 3.15 cfs @ | 12.09 hrs, Volume= | 0.230 af |  |
| Outflow | 0.21 cfs @ | 13.63 hrs , Volume= | 0.116 af, | Atten $=93 \%, L a g=92.6 \mathrm{~min}$ |
| Discarded | 0.04 cfs @ | 13.63 hrs , Volume= | 0.076 af |  |
| Primary | 0.17 cfs @ | 13.63 hrs , Volume= | 0.040 af |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Peak Elev= 79.04' @ 13.63 hrs Surf.Area= 6,757 sf Storage $=6,129$ cf
Plug-Flow detention time $=$ (not calculated: outflow precedes inflow)
Center-of-Mass det. time $=256.9 \min (1,052.7-795.8)$


Discarded OutFlow Max=0.04 cfs @ 13.63 hrs HW=79.04' (Free Discharge)
—1=Exfiltration (Exfiltration Controls 0.04 cfs)
Primary OutFlow Max=0.17 cfs @ 13.63 hrs HW=79.04' TW=75.51' (Dynamic Tailwater)
—2=Broad-Crested Rectangular Weir (Weir Controls 0.17 cfs @ 0.47 fps )

## Summary for Pond WQI-2: Water Quality Inlet



Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Peak Elev= 77.96' @ 12.20 hrs Surf.Area= $2,510 \mathrm{sf}$ Storage= $2,117 \mathrm{cf}$
Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time $=147.4 \min (910.7-763.4)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | 77.00 | $3,564 \mathrm{cf}$ | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 77.00 | 1,892 | 0 | 0 |
| 78.00 | 2,535 | 2,214 | 2,214 |
| 78.50 | 2,866 | 1,350 | 3,564 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Discarded | 77.00' | $0.270 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area |
| \#2 | Primary | 77.90' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .001 .201 .401 .60 |
|  |  |  | Coef. (English) 2.492 .562 .702 .692 .682 .692 .672 .64 |

Discarded OutFlow Max=0.02 cfs @ 12.20 hrs HW=77.96' (Free Discharge)
L-1=Exfiltration (Exfiltration Controls 0.02 cfs)
Primary OutFlow Max=0.76 cfs @ 12.20 hrs HW=77.96' TW=76.25' (Dynamic Tailwater)
L2=Broad-Crested Rectangular Weir (Weir Controls 0.76 cfs @ 0.62 fps )

## Summary for Subcatchment S-1: Tributary to South Culvert

Runoff $=\quad 2.93$ cfs @ 12.13 hrs, Volume $=0.231$ af, Depth= $3.83^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 100-yr Rainfall=7.00"


## Summary for Subcatchment S-10: Tributary toward CB-8

Runoff $=\quad 5.87$ cfs @ 12.08 hrs, Volume $=\quad 0.482$ af, Depth= 6.76"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-yr Rainfall=7.00"


## Summary for Subcatchment S-11: Tributary to Northerly Wetland

Runoff $=\quad 5.61$ cfs @ 12.09 hrs, Volume $=\quad 0.404$ af, Depth= 4.81"
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 100-yr Rainfall=7.00"


## Summary for Subcatchment S-2: Tributary to North Culvert

Runoff $=3.04$ cfs @ 12.11 hrs, Volume= $\quad 0.231$ af, Depth= 3.94"
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ Type III 24-hr 100-yr Rainfall=7.00"


## Summary for Subcatchment S-3: Tributary to Water Quality Inlet

Runoff $=\quad 4.90$ cfs @ 12.08 hrs, Volume= $\quad 0.366$ af, Depth= $5.71^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span $=0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ Type III 24-hr 100-yr Rainfall=7.00"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 7,500 | 98 | Water Quality Inlet |  |  |
| * | 14,700 | 98 | Roadway |  |  |
|  | 11,350 | 70 | Woods, Good, HSG C |  |  |
|  | 33,550 | 89 | Weighted Average |  |  |
|  | 11,350 |  | Pervious Area Impervious Area |  |  |
|  | 22,200 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{aligned} & \text { Capacity } \\ & \text { (cfs) } \end{aligned}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment S-3a: Tributary to Southerly Wetland

Runoff $=\quad 2.82$ cfs @ 12.09 hrs, Volume $=0.206$ af, Depth= $5.25^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"

|  | Area (sf) | CN | Description |
| :--- | ---: | ---: | :--- |
| $*$ | 9,465 | 98 | Wetland |
| $*$ | 1,527 | 98 | Roadway |
| 9,498 | 70 | Woods, Good, HSG C |  |
| 20,490 | 85 | Weighted Average |  |
| 9,498 |  | Pervious Area |  |
|  | 10,992 |  | Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

Summary for Subcatchment S-4: Tributary to CB-1
Runoff $=\quad 0.23$ cfs @ 12.08 hrs , Volume $=\quad 0.019$ af, Depth $=6.76{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-yr Rainfall=7.00"


## Summary for Subcatchment S-5: Tributary to CB-2

Runoff $=\quad 0.22$ cfs @ 12.08 hrs, Volume $=\quad 0.018$ af, Depth $=6.76{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ Type III 24-hr 100-yr Rainfall=7.00"


## Summary for Subcatchment S-6: Tributary Off-Site

Runoff $=\quad 2.35$ cfs @ 12.08 hrs, Volume= 0.190 af, Depth= $6.64{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 100-yr Rainfall=7.00"

|  | Area (sf) | CN | Description |
| :---: | :---: | :---: | :---: |
| * | 14,625 | 98 | Paved Parking |
|  | 335 | 74 | >75\% Grass cover, Good, HSG C |
|  | 14,960 | 97 | Weighted Average |
|  | 335 |  | Pervious Area |
|  | 14,625 |  | Impervious Area |


| Tc | Length <br> $(\mathrm{min})$ | Slope <br> $(\mathrm{feet})$ | Velocity <br> $(\mathrm{ft} / \mathrm{ft})$ |
| ---: | ---: | ---: | ---: |

## Summary for Subcatchment S-7: Tributary toward CB-7

Runoff $=\quad 2.23 \mathrm{cfs} @ 12.08 \mathrm{hrs}$, Volume $=\quad 0.183 \mathrm{af}$, Depth= $6.76^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 100-yr Rainfall=7.00"


## Summary for Subcatchment S-8: Tributary to SRS-2

Runoff $=\quad 4.41 \mathrm{cfs} @ 12.08 \mathrm{hrs}$, Volume $=\quad 0.362 \mathrm{af}$, Depth= $6.76^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ Type III 24-hr 100-yr Rainfall=7.00"


## Summary for Subcatchment S-8a: Tributary toward WQI

Runoff $=\quad 2.11$ cfs @ 12.08 hrs, Volume= 0.168 af, Depth= 6.52"
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 100-yr Rainfall=7.00"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 10,000 | 98 P | Paved Parking |  |  |
|  | 940 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
| * | 2,535 | 98 W | Water Quality Inlet |  |  |
|  | 13,475 | 96 W | Weighted Average |  |  |
|  | 940 |  | Pervious Area |  |  |
|  | 12,535 |  | Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment S-9: Tributary toward CB-9

Runoff $=\quad 7.18$ cfs @ 12.08 hrs , Volume $=\quad 0.589$ af, Depth $=6.76{ }^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Type III 24-hr 100-yr Rainfall=7.00"

| Area (sf) |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 45,550 | 98 P | aved Park |  |  |
|  | 45,550 |  | pervious | Area |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

[^0]
## Summary for Reach CB-1: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow Area $=$ | $0.033 \mathrm{ac}, 100.00 \%$ Impervious, Inflow Depth $=6.76 \mathrm{l}$ for $100-\mathrm{yr}$ event |  |  |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.23 \mathrm{cfs} @ 12.08 \mathrm{hrs}$, Volume $=$ | 0.019 af |
| Outflow | $=$ | $0.23 \mathrm{cfs} @ 12.08 \mathrm{hrs}$, Volume $=$ | 0.019 af , Atten $=0 \%$, Lag= 0.1 min |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=1.89 \mathrm{fps}$, Min. Travel Time $=0.1 \mathrm{~min}$
Avg. Velocity $=0.62 \mathrm{fps}$, Avg. Travel Time $=0.4 \mathrm{~min}$
Peak Storage $=2$ cf @ 12.08 hrs, Average Depth at Peak Storage $=0.19^{\prime}$
Bank-Full Depth=1.25', Capacity at Bank-Full= 4.41 cfs
15.0" Diameter Pipe, n= 0.013

Length= 15.0' Slope= 0.0047 '/'
Inlet Invert= 76.37', Outlet Invert= 76.30'


## Summary for Reach CB-2: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow Area $=$ | $0.032 \mathrm{ac}, 100.00 \%$ Impervious, Inflow Depth $=6.76 \mathrm{l}$ for $100-\mathrm{yr}$ event |  |  |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.22 \mathrm{cfs} @ 12.08 \mathrm{hrs}$, Volume $=$ | 0.018 af |
| Outflow | $=$ | $0.22 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=$ | 0.018 af , Atten $=0 \%$, Lag $=0.3 \mathrm{~min}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Max. Velocity $=1.71 \mathrm{fps}$, Min. Travel Time $=0.5 \mathrm{~min}$
Avg. Velocity $=0.56 \mathrm{fps}$, Avg. Travel Time $=1.4 \mathrm{~min}$
Peak Storage $=6$ cf @ 12.09 hrs, Average Depth at Peak Storage $=0.22^{\prime}$
Bank-Full Depth= 1.00', Capacity at Bank-Full= 2.05 cfs
12.0" Diameter Pipe, $n=0.025$ Corrugated metal Length=48.0' Slope= 0.0123 '/'
Inlet Invert= 76.09', Outlet Invert= 75.50'


## Summary for Reach CB-7: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Max. Velocity $=7.89 \mathrm{fps}$, Min. Travel Time $=0.1 \mathrm{~min}$
Avg. Velocity $=2.64 \mathrm{fps}$, Avg. Travel Time $=0.4 \mathrm{~min}$
Peak Storage= 19 cf @ 12.08 hrs, Average Depth at Peak Storage= 0.39'
Bank-Full Depth= 1.00', Capacity at Bank-Full= 6.98 cfs
12.0" Diameter Pipe, n= 0.013

Length=66.0' Slope= 0.0383 '/'
Inlet Invert= 78.71', Outlet Invert= 76.18'


## Summary for Reach CB-8: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow Area $=$ | $2.188 \mathrm{ac}, 71.65 \%$ Impervious, Inflow Depth $=5.51 " \mathrm{for} 100-\mathrm{yr}$ event |  |
| :--- | :--- | :--- |
| Inflow | $=$ | 6.60 cfs @ 12.10 hrs, Volume $=$ |
| Outflow | $=$ | $6.60 \mathrm{cfs} @ 12.11 \mathrm{hrs}$, Volume $=$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Max. Velocity= 4.61 fps , Min. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=1.94 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 107 cf @ 12.11 hrs, Average Depth at Peak Storage= 1.13'
Bank-Full Depth= 1.50', Capacity at Bank-Full= 7.18 cfs
18.0" Diameter Pipe, n= 0.013

Length= $75.0^{\prime}$ Slope $=0.0047$ '/'
Inlet Invert= 75.45', Outlet Invert= 75.10'


## Summary for Reach CB-9: Catch Basin

[52] Hint: Inlet/Outlet conditions not evaluated
[55] Hint: Peak inflow is $186 \%$ of Manning's capacity
[76] Warning: Detained 0.088 af (Pond w/culvert advised)
[63] Warning: Exceeded Reach CB-8 INLET depth by 0.69 @ 12.80 hrs
Inflow Area $=3.234$ ac, $80.82 \%$ Impervious, Inflow Depth $=5.91$ " for $100-$ yr event
Inflow $=\quad 13.69 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=1.594 \mathrm{af}$
Outflow = $7.89 \mathrm{cfs} @ 11.99 \mathrm{hrs}$, Volume $=1.594 \mathrm{af}$, Atten $=42 \%$, Lag= 0.0 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=4.76 \mathrm{fps}$, Min. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=2.26 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 133 cf @ 12.00 hrs, Average Depth at Peak Storage= 1.50'
Bank-Full Depth= 1.50', Capacity at Bank-Full= 7.38 cfs
18.0" Diameter Pipe, n= 0.013

Length= 75.0' Slope= 0.0049 '/'
Inlet Invert= 75.29', Outlet Invert= 74.92'


## Summary for Reach P-1: 18" Culvert

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=2.71 \mathrm{fps}$, Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=1.02 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 40 cf @ 12.13 hrs , Average Depth at Peak Storage= $0.88^{\prime}$
Bank-Full Depth= 1.50', Capacity at Bank-Full= 4.49 cfs
18.0" Diameter Pipe, $\mathrm{n}=0.025$ Corrugated metal

Length= 37.0' Slope= 0.0068 '/'
Inlet Invert= 84.57', Outlet Invert= 84.32'


## Summary for Reach P-2: 12" Culvert

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Max. Velocity $=5.00 \mathrm{fps}$, Min. Travel Time $=0.1 \mathrm{~min}$
Avg. Velocity $=1.91 \mathrm{fps}$, Avg. Travel Time $=0.4 \mathrm{~min}$
Peak Storage= 26 cf @ 12.12 hrs, Average Depth at Peak Storage= 0.72'
Bank-Full Depth= 1.00', Capacity at Bank-Full= 3.49 cfs
12.0" Diameter Pipe, $n=0.025$ Corrugated metal Length=42.0' Slope= 0.0355 '/'
Inlet Invert= 84.18', Outlet Invert= 82.69'


## Summary for Pond SRS-2: Subsurface Recharge System

Inflow Area $=\quad 0.643 \mathrm{ac}, 100.00 \%$ Impervious, Inflow Depth $=6.76$ " for 100-yr event
Inflow = 4.41 cfs @ 12.08 hrs , Volume=
0.362 af

Outflow =
Discarded = Primary =
3.63 cfs @ 12.14 hrs, Volume=
0.01 cfs @ 2.31 hrs, Volume=
3.62 cfs @ 12.14 hrs, Volume=
0.300 af, Atten $=18 \%$, Lag $=3.3 \mathrm{~min}$
0.031 af
0.269 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$ Peak Elev= 81.36' @ 12.14 hrs Surf.Area= 2,074 sf Storage $=4,195 \mathrm{cf}$

Plug-Flow detention time $=152.6 \mathrm{~min}$ calculated for 0.300 af ( $83 \%$ of inflow)
Center-of-Mass det. time= $81.6 \mathrm{~min}(824.6-743.0)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1 | 78.50' | 1,825 cf | 17.00 'W x 122.00'L x 3.75'H Prismatoid |
|  |  |  | 7,778 cf Overall - 3,215 cf Embedded $=4,562 \mathrm{cf} \times 40.0 \%$ Voids |
| \#2 | 79.00' | 3,215 cf | 52.6"W x 34.0"H x 7.50'L Cultec R-V8x 48 Inside \#1 |
| 5,040 cf Total Available Storage |  |  |  |
| Device | Routing | Invert Outlet Devices |  |
| \#1 | Discarded | 78.50 ' 0.27 | 0.270 in/hr Exfiltration over Surface area |
| \#2 | Primary | 80.50' | $6.0 "$ x 6.0' long Culvert X 5.00 |
|  |  |  | , square edge headwall, $\mathrm{Ke}=0.500$ |
|  |  |  | thvert= 80.44' S=0.0100 '/' Cc= $0.900 \quad \mathrm{n}=0.013$ |

Discarded OutFlow Max=0.01 cfs @ 2.31 hrs HW=78.54' (Free Discharge)
-1=Exfiltration (Exfiltration Controls 0.01 cfs )
Primary OutFlow Max=3.61 cfs @ 12.14 hrs HW=81.36' (Free Discharge)
—2=Culvert (Barrel Controls 3.61 cfs @ 3.68 fps )

## Summary for Pond WET-1: Sortherly Wetland

[63] Warning: Exceeded Reach CB-1 INLET depth by 0.52 ' @ 12.51 hrs
[63] Warning: Exceeded Reach CB-2 INLET depth by 0.79 @ 12.52 hrs
[63] Warning: Exceeded Reach CB-9 INLET depth by 0.79 @ 12.83 hrs

| Inflow Area = | 6.278 ac, 61.12\% Impervious, Inflow Depth = 4.90" for 100-yr event |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 18.59 cfs @ | 12.12 hrs , Volume= | 2.565 af |  |
| Outflow | 12.26 cfs @ | 12.47 hrs, Volume= | 2.512 af, | Atten $=34 \%, L a g=20.9 \mathrm{~min}$ |
| Discarded $=$ | 0.07 cfs @ | 12.47 hrs , Volume= | 0.084 af |  |
| Primary | 12.19 cfs @ | 12.47 hrs, Volume= | 2.428 af |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Peak Elev= 76.98' @ 12.47 hrs Surf.Area= 10,761 sf Storage $=20,884 \mathrm{cf}$
Plug-Flow detention time $=62.6$ min calculated for 2.512 af ( $98 \%$ of inflow)
Center-of-Mass det. time $=49.9 \mathrm{~min}(837.9-788.0)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $74.00^{\prime}$ | 37,115 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum. Store <br> (cubic-feet) |
| :---: | ---: | ---: | ---: |
| 74.00 | 3,200 | 0 | 0 |
| 76.00 | 8,300 | 11,500 | 11,500 |
| 78.00 | 13,300 | 21,600 | 33,100 |
| 78.50 | 2,760 | 4,015 | 37,115 |
|  |  |  |  |
| Device | Routing | Invert | Outlet Devices |

Discarded OutFlow Max=0.07 cfs @ 12.47 hrs HW=76.98' (Free Discharge)
L1=Exfiltration (Exfiltration Controls 0.07 cfs )
Primary OutFlow Max=12.19 cfs @ 12.47 hrs HW=76.98' (Free Discharge)
L2=Culvert (Barrel Controls 12.19 cfs @ 4.08 fps )

## Summary for Pond WET-2: Northerly Wetland

[87] Warning: Oscillations may require Finer Routing or smaller dt
[62] Warning: Exceeded Reach CB-7 OUTLET depth by 0.44' @ 12.44 hrs


Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 76.83' @ 12.32 hrs Surf.Area= 10,382 sf Storage= $7,781 \mathrm{cf}$
Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time $=36.4 \mathrm{~min}$ ( 822.5-786.1)


Discarded OutFlow Max=0.06 cfs @ 12.32 hrs HW=76.83' (Free Discharge)
②=Exfiltration (Exfiltration Controls 0.06 cfs )
Primary OutFlow Max=3.06 cfs @ 12.41 hrs HW=76.82' TW=76.35' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 3.06 cfs @ 2.70 fps )

| Inflow Area = | 0.770 ac, $66.17 \%$ Impervious, Inflow Depth = 5.71" for 100-yr event |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 4.90 cfs @ | 12.08 hrs, Volume= | 0.366 af |  |
| Outflow | 1.96 cfs @ | 12.30 hrs , Volume= | 0.251 af, | Atten $=60 \%$, Lag $=13.1 \mathrm{~min}$ |
| Discarded | 0.04 cfs @ | 12.30 hrs , Volume= | 0.081 af |  |
| Primary | 1.91 cfs @ | 12.30 hrs , Volume= | 0.169 af |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Peak Elev=79.18' @ 12.30 hrs Surf.Area= 6,989 sf Storage $=7,127 \mathrm{cf}$
Plug-Flow detention time $=$ (not calculated: outflow precedes inflow)
Center-of-Mass det. time $=135.8 \mathrm{~min}$ (919.0-783.2)


Discarded OutFlow Max=0.04 cfs @ 12.30 hrs HW=79.18' (Free Discharge)
L1=Exfiltration (Exfiltration Controls 0.04 cfs)
Primary OutFlow Max=1.91 cfs @ 12.30 hrs HW=79.18' TW=76.91' (Dynamic Tailwater)
L2=Broad-Crested Rectangular Weir (Weir Controls 1.91 cfs @ 1.06 fps )

## Summary for Pond WQI-2: Water Quality Inlet

| Inflow Area = | $0.309 \mathrm{ac}, 93.02 \%$ Impervious, Inflow Depth = 6.52" for 100-yr event |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 2.11 cfs @ | 12.08 hrs, Volume= | 0.168 af |  |
| Outflow | 2.00 cfs @ | 12.11 hrs, Volume= | 0.130 af, | Atten= 5\%, Lag= 1.6 min |
| Discarded | 0.02 cfs @ | 12.11 hrs , Volume= | 0.033 af |  |
| Primary | 1.99 cfs @ | 12.11 hrs, Volume= | 0.097 af |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= $0.01 \mathrm{hrs} / 3$
Peak Elev= 78.02' @ 12.11 hrs Surf.Area= 2,546 sf Storage $=2,256$ cf
Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time $=101.6 \mathrm{~min}$ ( 856.5-755.0 )

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | 77.00 | $3,564 \mathrm{cf}$ | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 77.00 | 1,892 | 0 | 0 |
| 78.00 | 2,535 | 2,214 | 2,214 |
| 78.50 | 2,866 | 1,350 | 3,564 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Discarded | 77.00' | $0.270 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area |
| \#2 | Primary | $77.90{ }^{\prime}$ | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .001 .201 .401 .60 |
|  |  |  | Coef. (English) $2.492 .562 .702 .692 .682 .692 .67 \quad 2.64$ |

Discarded OutFlow Max=0.02 cfs @ 12.11 hrs HW=78.02' (Free Discharge)
L-1=Exfiltration (Exfiltration Controls 0.02 cfs)
Primary OutFlow Max=1.99 cfs @ 12.11 hrs HW=78.02' TW=76.48' (Dynamic Tailwater)
L2=Broad-Crested Rectangular Weir (Weir Controls $1.99 \mathrm{cfs} @ 0.85 \mathrm{fps}$ )






[^0]:    6.0

    Direct Entry, Min. Tc

