11D Industrial Drive P.O. Box 1178 Mattapoisett, MA 02739 Tel. (508) 758-2749 Fax (508) 758-2849 The Crocker Building 4 Court Street, Suite 104 Taunton, MA 02780 Tel. (508) 824-9279 Fax (508) 824-9276

Project Memo

Date: 5/4/16

New Bedford Conservation Commission

To:

From: Rich Riccio

Re: 200 Theodore Rice Boulevard - Americae OOC Request Updated Drainage Calculations

The purpose of this memorandum is to summarize the results of the updated Post Development Hydrologic Calculations that have been performed to support a Request for an Amended Order of Conditions for the above referenced project. The previously approved Pre Development Hydrologic Analysis remains unchanged from the Stormwater Management System Report Addendum 2 prepared for NSTAR and dated 11/9/13. The results of those calculations are presented below for comparison.

RICCIO III

CIVIL

Job No. 1948

Previously Approved Pre Development Hydrologic Summary

Storm Event	Analysis	Analysis	Analysis	Analysis
	Point	Point	Point	Point
	AP-1	AP-2	AP-3	AP-4
	Rate of Flow	Rate of Flow	Rate of Flow	Rate of Flow
	(c.f.s.)	(c.f.s.)	(c.f.s.)	(c.f.s.)
2-year storm	11.89	15.35	0.77	4.32
10-year storm	20.82	27.81	2.34	8.11
25-year storm	26.61	36.12	3.52	10.77
100-year storm	37.01	51.15	5.84	15.73

As described in the Request for an Amended Order of Conditions, the applicant is proposing to construct a smaller project than previously approved and the Post Development Hydrologic Analysis has been re-visited to address the design changes shown on the plans. A summary of the results of the updated Post Development Hydrologic Analysis is presented below.

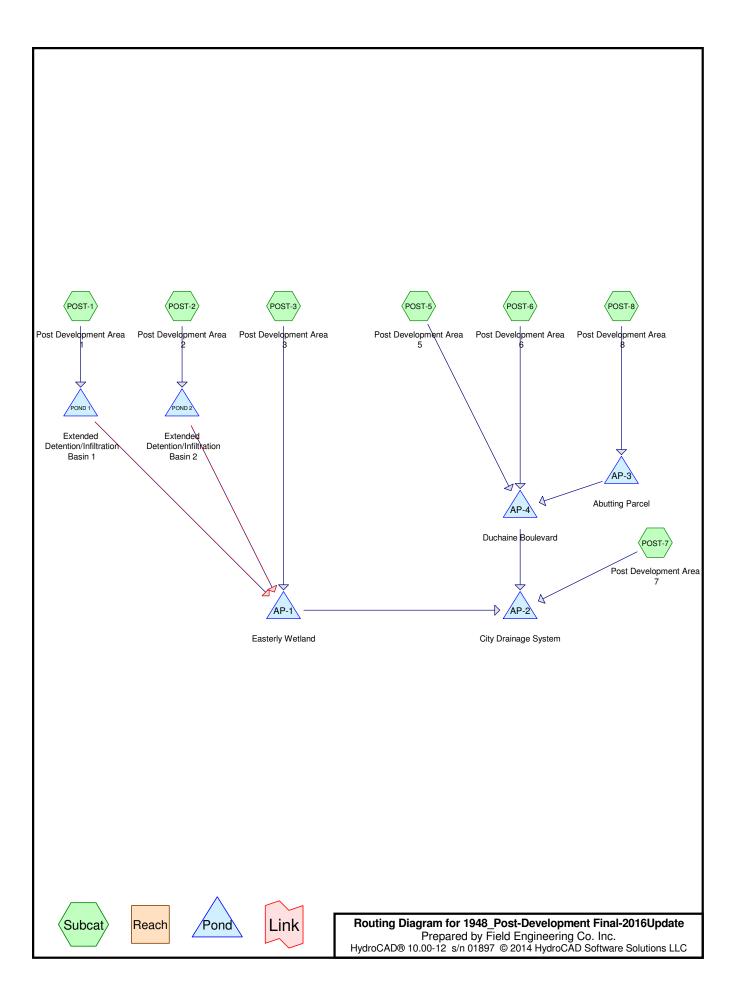
Updated Post Development Hydrologic Summary

Storm Event	Analysis	Analysis	Analysis	Analysis
	Point	Point	Point	Point
	AP-1	AP-2	AP-3	AP-4
	Rate of Flow	Rate of Flow	Rate of Flow	Rate of Flow
	(c.f.s.)	(c.f.s.)	(c.f.s.)	(c.f.s.)
2-year storm	4.50	11.05	0.67	5.89
10-year storm	13.71	26.86	2.09	10.26
25-year storm	17.43	35.26	3.14	13.14
100-year storm	23.06	48.13	5.20	18.47

Project Memo 200 Theodore Rice Boulevard Updated Drainage Calculations Page 2 of 2

The revised hydrologic analysis indicates that the stormwater management system design for the site still meets or reduces peak runoff rates for the 2, 10, 25, and 100 year, 24 hour, Type III storm events from the pre developed levels at the subject analysis points. It should be noted that in the Analysis, we ultimately combined the runoff from the easterly wetlands (Analysis Point 1) and the abutting properties (Analysis Point 3) with the areas contributing runoff the Duchaine Boulevard drainage system (Analysis Point 2), as all of this water ultimately discharges to the substantial culvert and drainage ditch running along Duchaine Boulevard. As the above tables and the analysis show, the proposed re-development of this property will not result in an increase in the rates of runoff to the Duchaine Boulevard drainage system south of Barnet Boulevard. The proposed redevelopment of this property will actually result in approximately 28% reduction in the contributing rates of runoff in the 2 year storm event and a 6% reduction in the 100 year storm event.

We had also added Analysis Point 4 to this study, which is the portion of the Duchaine Boulevard drainage system to the immediate west of the project site. We have added this Analysis Point at the request of Nitsch during their second review. As the tables above show, there is an increase in the rates of runoff to this portion of the Duchaine Boulevard Drainage system; however, the increase is less than was previously approved. We will work with the New Bedford Department of Public Infrastructure to ensure the continued functionality of this portion of the Duchaine Boulevard drainage system as a result of this project.



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 2

Time span=0.00-30.00 hrs, dt=0.03 hrs, 1001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment POST-1: Post Runoff Area=210,010 sf 81.60% Impervious Runoff Depth=2.54"

Tc=6.0 min CN=91 Runoff=13.98 cfs 1.021 af

Subcatchment POST-2: Post Development Runoff Area=29,717 sf 51.34% Impervious Runoff Depth=1.78"

Tc=6.0 min CN=82 Runoff=1.42 cfs 0.101 af

Subcatchment POST-3: Post Development Runoff Area=150,851 sf 2.08% Impervious Runoff Depth=0.71"

Flow Length=390' Slope=0.0100 '/' Tc=13.8 min CN=64 Runoff=1.78 cfs 0.203 af

Subcatchment POST-5: Post Development Runoff Area=66,789 sf 54.70% Impervious Runoff Depth=1.71"

Flow Length=300' Slope=0.0100 '/' Tc=9.9 min CN=81 Runoff=2.67 cfs 0.218 af

Subcatchment POST-6: Post Development Runoff Area=43,316 sf 78.89% Impervious Runoff Depth=2.45"

Tc=6.0 min CN=90 Runoff=2.80 cfs 0.203 af

Subcatchment POST-7: Post Runoff Area=109,739 sf 43.74% Impervious Runoff Depth=1.37"

Flow Length=487' Tc=13.6 min CN=76 Runoff=3.07 cfs 0.287 af

Subcatchment POST-8: Post Development Runoff Area=80,316 sf 0.00% Impervious Runoff Depth=0.49"

Tc=6.0 min CN=59 Runoff=0.67 cfs 0.076 af

Pond AP-1: Easterly Wetland Inflow=4.50 cfs 0.812 af

Primary=4.50 cfs 0.812 af

Pond AP-2: City Drainage System Inflow=11.05 cfs 1.595 af

Primary=11.05 cfs 1.595 af

Pond AP-3: Abutting Parcel Inflow=0.67 cfs 0.076 af

Primary=0.67 cfs 0.076 af

Pond AP-4: Duchaine Boulevard Inflow=5.89 cfs 0.497 af

Primary=5.89 cfs 0.497 af

Pond POND 1: Extended Peak Elev=86.57' Storage=18,524 cf Inflow=13.98 cfs 1.021 af

Discarded=0.32 cfs 0.412 af Primary=3.20 cfs 0.609 af Secondary=0.00 cfs 0.000 af Outflow=3.52 cfs 1.021 af

Pond POND 2: Extended Detention/Infiltration Peak Elev=85.74' Storage=1,604 cf Inflow=1.42 cfs 0.101 af

Discarded=0.17 cfs 0.101 af Primary=0.00 cfs 0.000 af Secondary=0.00 cfs 0.000 af Outflow=0.17 cfs 0.101 af

Total Runoff Area = 15.857 ac Runoff Volume = 2.109 af Average Runoff Depth = 1.60" 55.34% Pervious = 8.776 ac 44.66% Impervious = 7.081 ac

Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 3

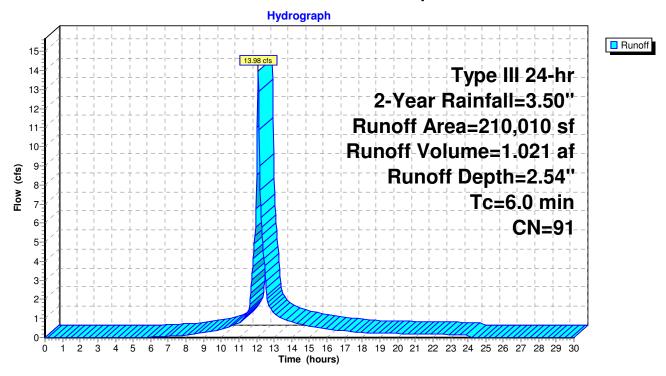
Summary for Subcatchment POST-1: Post Development Area 1

Runoff = 13.98 cfs @ 12.09 hrs, Volume= 1.021 af, Depth= 2.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Year Rainfall=3.50"

	A	rea (sf)	CN	Description						
	1	17,140	98	Roofs, HSC	βB					
		54,222	98	Paved park	ing, HSG B	В				
		38,648	61	>75% Gras	s cover, Go	lood, HSG B				
	2	10,010	91	91 Weighted Average						
		38,648		18.40% Pei	vious Area	a				
	1	71,362		81.60% lmp	pervious Ar	rea				
	Tc	Length	Slope	•	Capacity					
(ı	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry.				

Subcatchment POST-1: Post Development Area 1



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 4

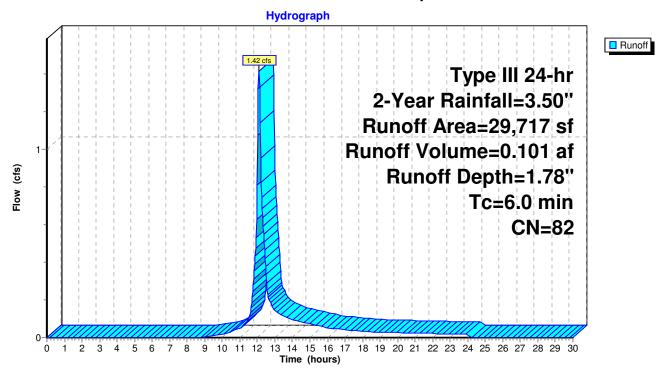
Summary for Subcatchment POST-2: Post Development Area 2

Runoff = 1.42 cfs @ 12.09 hrs, Volume= 0.101 af, Depth= 1.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Year Rainfall=3.50"

	Aı	rea (sf)	CN	Description						
		15,256	98	Paved park	ing, HSG B	В				
		10,651	61	>75% Gras	s cover, Go	lood, HSG B				
		3,810	80	>75% Gras	s cover, Go	lood, HSG D				
		29,717	82	Weighted Average						
		14,461		48.66% Per	vious Area	a				
		15,256		51.34% Imp	pervious Ar	rea				
	Tc	Length	Slope	,	Capacity	·				
(m	iin)	(feet)	(ft/ft) (ft/sec)	(cfs)					
(6.6					Direct Entry,				

Subcatchment POST-2: Post Development Area 2



Page 5

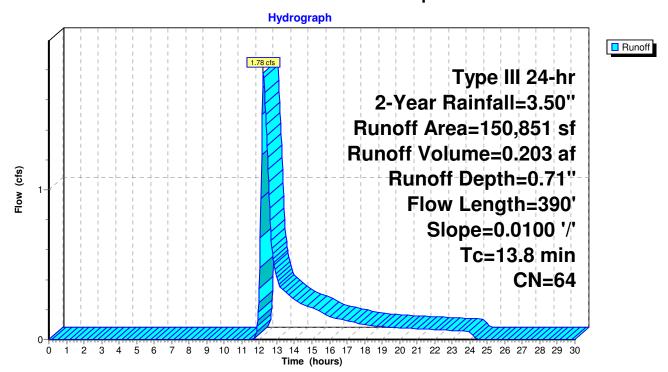
Summary for Subcatchment POST-3: Post Development Area 3

Runoff = 1.78 cfs @ 12.23 hrs, Volume= 0.203 af, Depth= 0.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Year Rainfall=3.50"

A	rea (sf)	CN I	Description		
	3,139	98 I	Paved park	ing, HSG B	}
	15,504	61 :	>75% Ġras	s cover, Go	ood, HSG B
	11,430	80 :	>75% Gras	s cover, Go	ood, HSG D
	81,786	55 \	Noods, Go	od, HSG B	
	38,992	77 \	Noods, Go	od, HSG D	
1	50,851	64 \	Neighted A	verage	
1	47,712	ę	97.92% Pei	rvious Area	
	3,139	2	2.08% Impe	ervious Area	a
Tc	Length	Slope	•	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.3	50	0.0100	0.08		Sheet Flow, A-B
					Grass: Dense n= 0.240 P2= 3.50"
3.5	340	0.0100	1.61		Shallow Concentrated Flow, B-C
					Unpaved Kv= 16.1 fps
13.8	390	Total			

Subcatchment POST-3: Post Development Area 3



Page 6

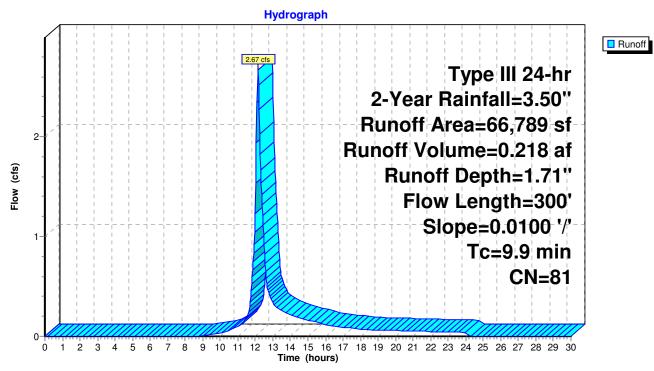
Summary for Subcatchment POST-5: Post Development Area 5

Runoff = 2.67 cfs @ 12.14 hrs, Volume= 0.218 af, Depth= 1.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Year Rainfall=3.50"

	Α	rea (sf)	CN D	Description		
		36,536	98 F	aved park	ing, HSG B	
		30,253	61 >	75% Gras	s cover, Go	ood, HSG B
		66,789	81 V	Veighted A	verage	
	30,253 45.30% Pervious Area				vious Area	
		36,536	5	4.70% lmp	pervious Ar	ea
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.1	50	0.0100	0.12		Sheet Flow, A-B
						Grass: Short n= 0.150 P2= 3.50"
	2.8	250	0.0100	1.50		Shallow Concentrated Flow, B-C
_						Grassed Waterway Kv= 15.0 fps
	99	300	Total			

Subcatchment POST-5: Post Development Area 5



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 7

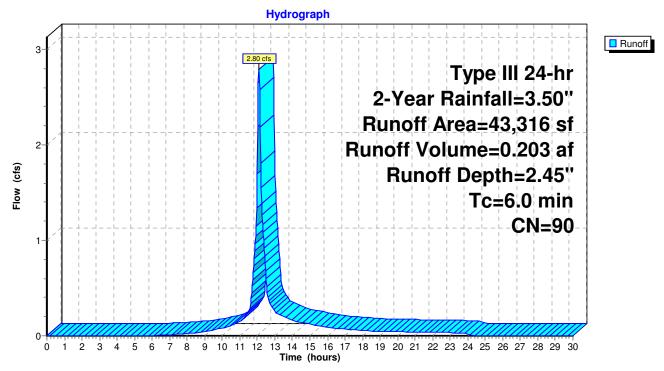
Summary for Subcatchment POST-6: Post Development Area 6

Runoff = 2.80 cfs @ 12.09 hrs, Volume= 0.203 af, Depth= 2.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Year Rainfall=3.50"

A	rea (sf)	CN	Description						
	34,171	98	Paved park	ing, HSG B	В				
	9,145	61	>75% Gras	s cover, Go	ood, HSG B				
	43,316	90	Weighted A	verage					
	9,145		21.11% Per	rvious Area	a				
	34,171		78.89% lmp	pervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	•				
6.0					Direct Entry,				

Subcatchment POST-6: Post Development Area 6



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 8

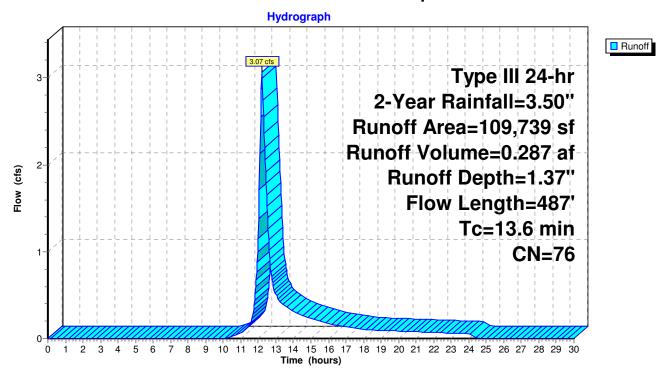
Summary for Subcatchment POST-7: Post Development Area 7

Runoff = 3.07 cfs @ 12.20 hrs, Volume= 0.287 af, Depth= 1.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Year Rainfall=3.50"

	Aı	rea (sf)	CN E	Description		
		47,999	98 F	aved park	ing, HSG B	
		7,096	39 >	75% Gras	s cover, Go	ood, HSG A
		54,644	61 >	75% Gras	s cover, Go	ood, HSG B
	1	09,739	76 V	Veighted A	verage	
		61,740	5	6.26% Per	vious Area	
		47,999	4	3.74% Imp	pervious Ar	ea
	Тс	Length	Slope	Velocity	Capacity	Description
((min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.1	50	0.0100	0.12		Sheet Flow, A-B
						Grass: Short n= 0.150 P2= 3.50"
	0.2	37	0.0300	2.79		Shallow Concentrated Flow, B-C
						Unpaved Kv= 16.1 fps
	6.3	400	0.0050	1.06		Shallow Concentrated Flow, B-C
						Grassed Waterway Kv= 15.0 fps
	13.6	487	Total			

Subcatchment POST-7: Post Development Area 7



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 9

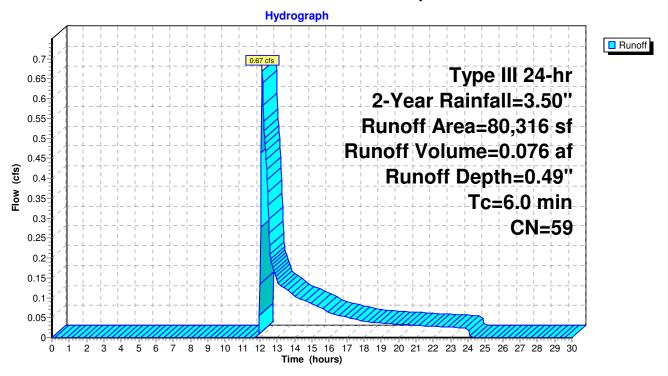
Summary for Subcatchment POST-8: Post Development Area 8

Runoff = 0.67 cfs @ 12.12 hrs, Volume= 0.076 af, Depth= 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Year Rainfall=3.50"

	Aı	rea (sf)	CN	Description						
		43,702	55	Woods, Go	od, HSG B					
		28,772	61	>75% Gras	s cover, Go	ood, HSG B				
		1,347	80	>75% Gras	s cover, Go	ood, HSG D				
		6,495	77	Woods, Go	od, HSG D					
		80,316	59	Weighted A	Weighted Average					
		80,316		100.00% P	ervious Are	a				
	Tc	Length	Slop	,	Capacity	Description				
(n	nin)	(feet)	(ft/f	t) (ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment POST-8: Post Development Area 8



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 10

Summary for Pond AP-1: Easterly Wetland

Analysis Point 1 is taken at the Boundary of the Easterly Wetland System

[40] Hint: Not Described (Outflow=Inflow)

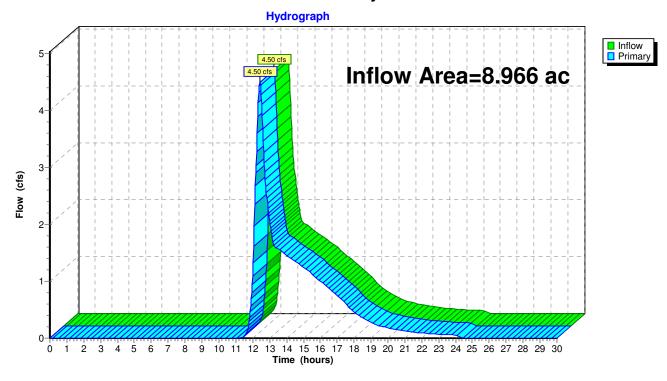
Inflow Area = 8.966 ac, 48.58% Impervious, Inflow Depth = 1.09" for 2-Year event

Inflow = 4.50 cfs @ 12.43 hrs, Volume= 0.812 af

Primary = 4.50 cfs @ 12.43 hrs, Volume= 0.812 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-1: Easterly Wetland



Page 11

Summary for Pond AP-2: City Drainage System

[40] Hint: Not Described (Outflow=Inflow)

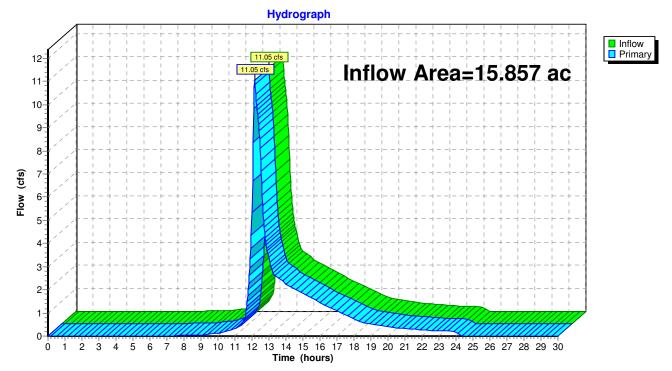
Inflow Area = 15.857 ac, 44.66% Impervious, Inflow Depth = 1.21" for 2-Year event

Inflow = 11.05 cfs @ 12.15 hrs, Volume= 1.595 af

Primary = 11.05 cfs @ 12.15 hrs, Volume= 1.595 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-2: City Drainage System



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 12

Summary for Pond AP-3: Abutting Parcel

[40] Hint: Not Described (Outflow=Inflow)

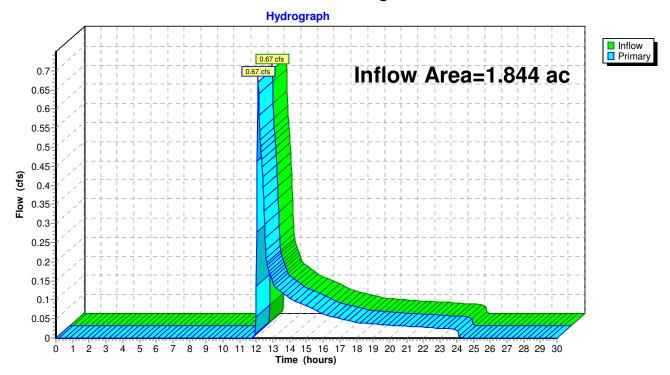
Inflow Area = 1.844 ac, 0.00% Impervious, Inflow Depth = 0.49" for 2-Year event

Inflow = 0.67 cfs @ 12.12 hrs, Volume= 0.076 af

Primary = 0.67 cfs @ 12.12 hrs, Volume= 0.076 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-3: Abutting Parcel



Page 13

Summary for Pond AP-4: Duchaine Boulevard

[40] Hint: Not Described (Outflow=Inflow)

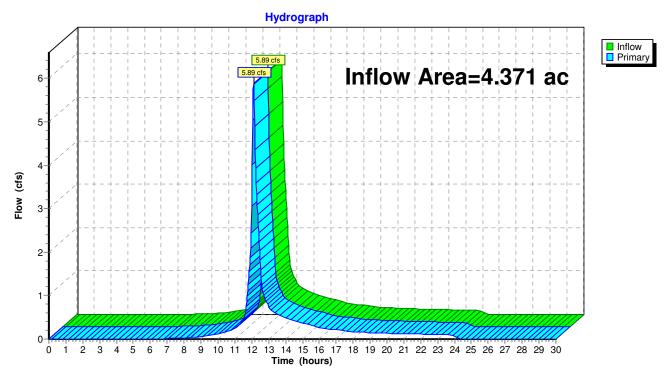
Inflow Area = 4.371 ac, 37.13% Impervious, Inflow Depth = 1.36" for 2-Year event

Inflow = 5.89 cfs @ 12.11 hrs, Volume= 0.497 af

Primary = 5.89 cfs @ 12.11 hrs, Volume= 0.497 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-4: Duchaine Boulevard



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 14

Summary for Pond POND 1: Extended Detention/Infiltration Basin 1

Inflow Area = 4.821 ac, 81.60% Impervious, Inflow Depth = 2.54" for 2-Year event Inflow 13.98 cfs @ 12.09 hrs, Volume= 1.021 af Outflow 3.52 cfs @ 12.47 hrs, Volume= 1.021 af, Atten= 75%, Lag= 22.9 min Discarded = 0.32 cfs @ 12.47 hrs, Volume= 0.412 af 3.20 cfs @ 12.47 hrs, Volume= Primary 0.609 af 0.00 hrs, Volume= Secondary = 0.00 cfs @ 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Peak Elev= 86.57' @ 12.47 hrs Surf.Area= 13,694 sf Storage= 18,524 cf

Plug-Flow detention time= 139.4 min calculated for 1.021 af (100% of inflow)

Center-of-Mass det. time= 139.4 min (938.6 - 799.2)

Volume	Invert	Avail.Sto	rage Storage	Description	
#1	85.00'	42,12	29 cf Custom	n Stage Data (Pr	ismatic) Listed below (Recalc)
Elevation	on Su	ırf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
85.0	00	9,930	0	0	
86.0	00	12,307	11,119	11,119	
87.0	00	14,741	13,524	24,643	
88.0	00	20,231	17,486	42,129	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	85.25'	15.0" Round	Culvert X 2.00	
	•		L= 40.0' CP	P, end-section c	onforming to fill, Ke= 0.500
			Inlet / Outlet	Invert= 85.25' / 8	85.00' S= 0.0063 '/' Cc= 0.900
			n= 0.013 Co	rrugated PE, sm	ooth interior, Flow Area= 1.23 sf
#2	Device 1	85.25'	5.0" Vert. Or	ifice/Grate X 2.0	0 C= 0.600
#3	Device 1	86.40'	4.0' long Sha	arp-Crested Rec	tangular Weir X 2.00
			2 End Contra	ction(s)	
#4	Secondary	87.50'	15.0' long x	10.0' breadth Bi	road-Crested Rectangular Weir
			Head (feet) (0.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60
			Coef. (Englis	h) 2.49 2.56 2.	70 2.69 2.68 2.69 2.67 2.64
#5	Discarded	85.00'	1.020 in/hr E	xfiltration over	Surface area

Discarded OutFlow Max=0.32 cfs @ 12.47 hrs HW=86.57' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.32 cfs)

Primary OutFlow Max=3.19 cfs @ 12.47 hrs HW=86.57' (Free Discharge)

1=Culvert (Passes 3.19 cfs of 8.55 cfs potential flow)

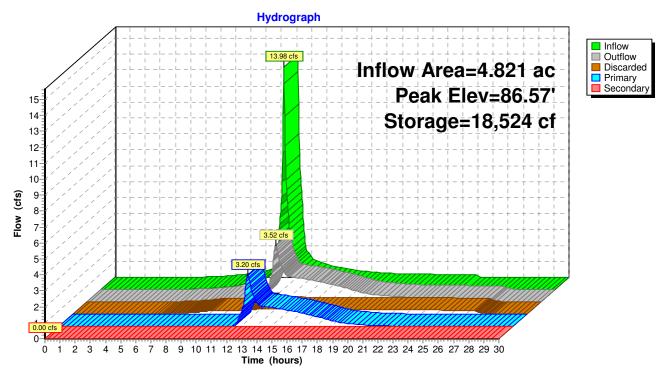
2=Orifice/Grate (Orifice Controls 1.38 cfs @ 5.08 fps)

—3=Sharp-Crested Rectangular Weir (Weir Controls 1.81 cfs @ 1.35 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=85.00' (Free Discharge)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 15

Pond POND 1: Extended Detention/Infiltration Basin 1



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 16

Summary for Pond POND 2: Extended Detention/Infiltration Basin 2

Inflow Area = 0.682 ac, 51.34% Impervious, Inflow Depth = 1.78" for 2-Year event Inflow 1.42 cfs @ 12.09 hrs, Volume= 0.101 af 0.17 cfs @ 12.86 hrs, Volume= Outflow 0.101 af, Atten= 88%, Lag= 45.9 min 0.17 cfs @ 12.86 hrs, Volume= Discarded = 0.101 af 0.00 cfs @ 0.00 hrs, Volume= Primary 0.000 af Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs / 4 Peak Elev= 85.74' @ 12.86 hrs Surf.Area= 7,155 sf Storage= 1,604 cf

Plug-Flow detention time= 82.6 min calculated for 0.101 af (100% of inflow) Center-of-Mass det. time= 82.5 min (914.5 - 832.0)

Volume	Invert A	vail.Storage	Storage	Description	
#1	85.50'	15,300 cf	Custon	n Stage Data (Pri	smatic) Listed below (Recalc)
Elevation (feet)	Surf.Are (sq-f		c.Store c-feet)	Cum.Store (cubic-feet)	
85.50	6,25	57	0	0	
86.00	8,13	34	3,598	3,598	
87.00	10,10	00	9,117	12,715	

15,300

Device	Routing	Invert	Outlet Devices
#1	Primary	85.50'	12.0" Round Culvert
	•		L= 21.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 85.50' / 85.00' S= 0.0238 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	85.85'	2.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	86.25'	2.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Secondary	86.75'	15.0' long x 10.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#5	Discarded	85.50'	1.020 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.17 cfs @ 12.86 hrs HW=85.74' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.17 cfs)

2,586

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=85.50' (Free Discharge)

-1=Culvert (Controls 0.00 cfs)

87.25

2=Orifice/Grate (Controls 0.00 cfs)

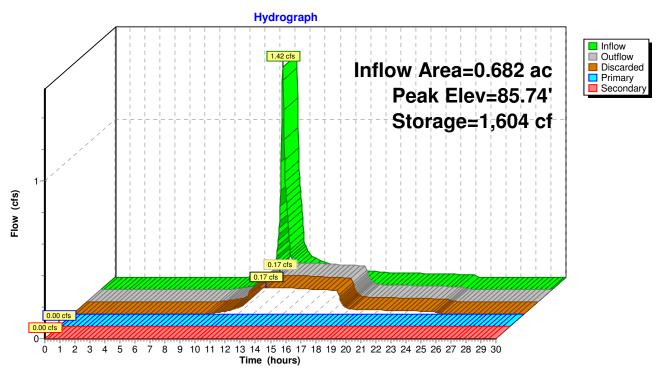
10,585

3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=85.50' (Free Discharge)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 17

Pond POND 2: Extended Detention/Infiltration Basin 2



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 18

Time span=0.00-30.00 hrs, dt=0.03 hrs, 1001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment POST-1: Post Runoff Area=210,010 sf 81.60% Impervious Runoff Depth=3.79"

Tc=6.0 min CN=91 Runoff=20.44 cfs 1.522 af

Subcatchment POST-2: Post Development Runoff Area=29,717 sf 51.34% Impervious Runoff Depth=2.90"

Tc=6.0 min CN=82 Runoff=2.30 cfs 0.165 af

Subcatchment POST-3: Post Development Runoff Area=150,851 sf 2.08% Impervious Runoff Depth=1.45"

Flow Length=390' Slope=0.0100 '/' Tc=13.8 min CN=64 Runoff=4.25 cfs 0.419 af

Subcatchment POST-5: Post Development Runoff Area=66,789 sf 54.70% Impervious Runoff Depth=2.81"

Flow Length=300' Slope=0.0100 '/' Tc=9.9 min CN=81 Runoff=4.40 cfs 0.359 af

Subcatchment POST-6: Post Development Runoff Area=43,316 sf 78.89% Impervious Runoff Depth=3.68"

Tc=6.0 min CN=90 Runoff=4.13 cfs 0.305 af

Subcatchment POST-7: Post Runoff Area=109,739 sf 43.74% Impervious Runoff Depth=2.37"

Flow Length=487' Tc=13.6 min CN=76 Runoff=5.47 cfs 0.498 af

Subcatchment POST-8: Post Development Runoff Area=80,316 sf 0.00% Impervious Runoff Depth=1.12"

Tc=6.0 min CN=59 Runoff=2.09 cfs 0.172 af

Pond AP-1: Easterly Wetland Inflow=13.71 cfs 1.477 af

Primary=13.71 cfs 1.477 af

Pond AP-2: City Drainage System Inflow=26.86 cfs 2.812 af

Primary=26.86 cfs 2.812 af

Pond AP-3: Abutting Parcel Inflow=2.09 cfs 0.172 af

Primary=2.09 cfs 0.172 af

Pond AP-4: Duchaine Boulevard Inflow=10.26 cfs 0.837 af

Primary=10.26 cfs 0.837 af

Pond POND 1: Extended Peak Elev=86.86' Storage=22,613 cf Inflow=20.44 cfs 1.522 af

Discarded=0.34 cfs 0.466 af Primary=9.55 cfs 1.056 af Secondary=0.00 cfs 0.000 af Outflow=9.89 cfs 1.522 af

Pond POND 2: Extended Detention/Infiltration Peak Elev=85.93' Storage=3,029 cf Inflow=2.30 cfs 0.165 af Discarded=0.19 cfs 0.163 af Primary=0.01 cfs 0.002 af Secondary=0.00 cfs 0.000 af Outflow=0.20 cfs 0.165 af

Total Runoff Area = 15.857 ac Runoff Volume = 3.441 af Average Runoff Depth = 2.60" 55.34% Pervious = 8.776 ac 44.66% Impervious = 7.081 ac

Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 19

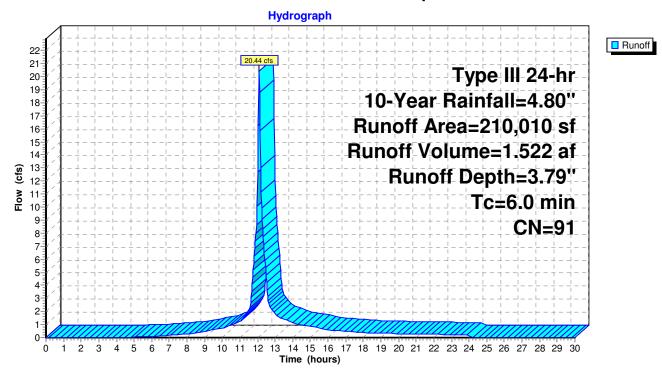
Summary for Subcatchment POST-1: Post Development Area 1

Runoff = 20.44 cfs @ 12.09 hrs, Volume= 1.522 af, Depth= 3.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Year Rainfall=4.80"

	Α	rea (sf)	CN	Description					
	1	17,140	98	Roofs, HSC	βB				
		54,222	98	Paved park	ing, HSG B	В			
		38,648	61	>75% Gras	s cover, Go	lood, HSG B			
	2	10,010	91	91 Weighted Average					
38,648 18.40% Pervious Area					a				
	171,362 81.60% Impervious Are				pervious Ar	rea			
	Tc	Length	Slope	•	Capacity	•			
(ı	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry.			

Subcatchment POST-1: Post Development Area 1



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 20

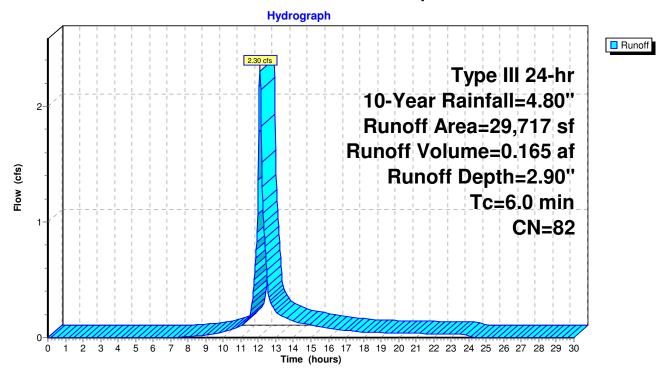
Summary for Subcatchment POST-2: Post Development Area 2

Runoff = 2.30 cfs @ 12.09 hrs, Volume= 0.165 af, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Year Rainfall=4.80"

	A	rea (sf)	CN I	Description					
		15,256	98	Paved park	ing, HSG B	3			
		10,651	61 :	>75% Ġras	s cover, Go	ood, HSG B			
		3,810	80 :	>75% Gras	s cover, Go	ood, HSG D			
		29,717	82 \	Weighted Average					
		14,461	4	48.66% Pervious Area					
		15,256	į	51.34% lmp	pervious Ar	ea			
	Tc	Length	Slope	•	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment POST-2: Post Development Area 2



Page 21

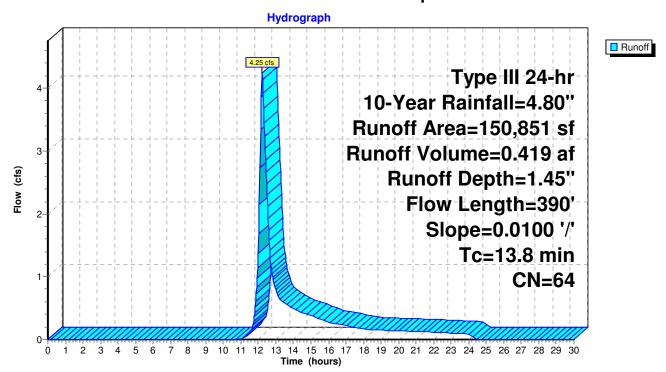
Summary for Subcatchment POST-3: Post Development Area 3

Runoff = 4.25 cfs @ 12.21 hrs, Volume= 0.419 af, Depth= 1.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Year Rainfall=4.80"

Aı	rea (sf)	CN I	Description		
	3,139	98 I	Paved park	ing, HSG B	}
	15,504	61 :	>75% Ġras	s cover, Go	ood, HSG B
	11,430	80 :	>75% Gras	s cover, Go	ood, HSG D
	81,786	55 \	Noods, Go	od, HSG B	
	38,992	77 \	Noods, Go	od, HSG D	
1	50,851	64 \	Neighted A	verage	
1	47,712	Ś	97.92% Pei	rvious Area	
	3,139 2.08% Impervious Area			ervious Area	a
Tc	Length	Slope		Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.3	50	0.0100	0.08		Sheet Flow, A-B
					Grass: Dense n= 0.240 P2= 3.50"
3.5	340	0.0100	1.61		Shallow Concentrated Flow, B-C
					Unpaved Kv= 16.1 fps
13.8	390	Total			

Subcatchment POST-3: Post Development Area 3



Page 22

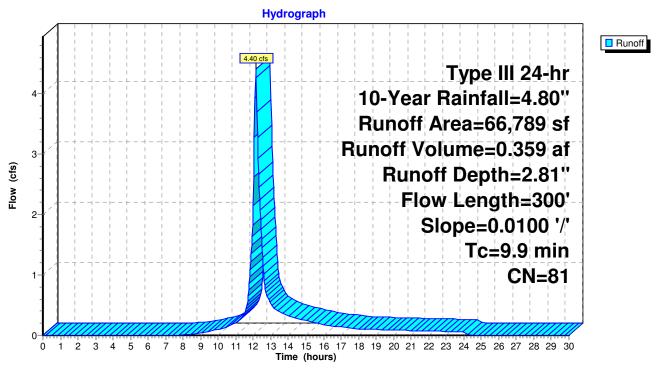
Summary for Subcatchment POST-5: Post Development Area 5

Runoff = 4.40 cfs @ 12.14 hrs, Volume= 0.359 af, Depth= 2.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Year Rainfall=4.80"

	Α	rea (sf)	CN D	Description		
		36,536	98 F	aved park	ing, HSG B	
		30,253	61 >	75% Gras	s cover, Go	ood, HSG B
		66,789	81 V	Veighted A	verage	
		30,253	4	5.30% Per	vious Area	
		36,536	5	4.70% lmp	pervious Ar	ea
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.1	50	0.0100	0.12		Sheet Flow, A-B
						Grass: Short n= 0.150 P2= 3.50"
	2.8	250	0.0100	1.50		Shallow Concentrated Flow, B-C
_						Grassed Waterway Kv= 15.0 fps
	99	300	Total			

Subcatchment POST-5: Post Development Area 5



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 23

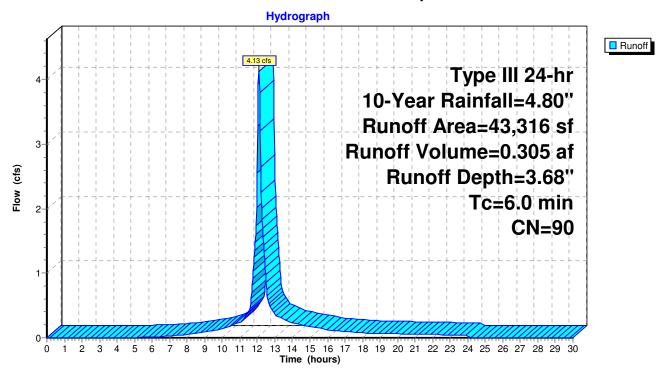
Summary for Subcatchment POST-6: Post Development Area 6

Runoff = 4.13 cfs @ 12.09 hrs, Volume= 0.305 af, Depth= 3.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Year Rainfall=4.80"

A	rea (sf)	CN	Description					
	34,171	98	Paved park	ing, HSG B	3			
	9,145	61 :	>75% Ġras	s cover, Go	ood, HSG B			
	43,316	3,316 90 Weighted Average						
	9,145 21.11% Pervious Area				a e e e e e e e e e e e e e e e e e e e			
	34,171		78.89% Imp	pervious Ar	rea			
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

Subcatchment POST-6: Post Development Area 6



Page 24

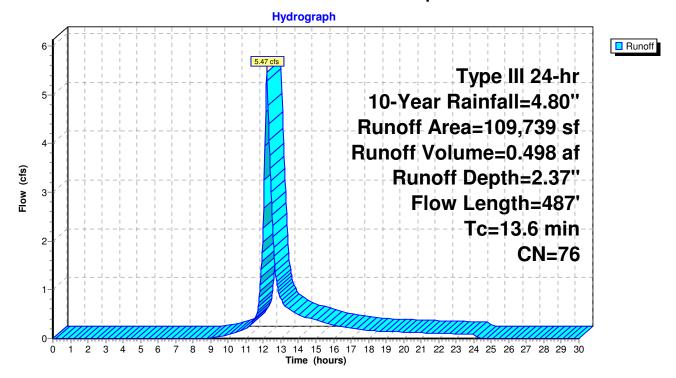
Summary for Subcatchment POST-7: Post Development Area 7

Runoff = 5.47 cfs @ 12.19 hrs, Volume= 0.498 af, Depth= 2.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Year Rainfall=4.80"

	Aı	rea (sf)	CN E	Description							
		47,999	98 F	98 Paved parking, HSG B							
		7,096	39 >	75% Gras	s cover, Go	ood, HSG A					
		54,644	61 >	75% Gras	s cover, Go	ood, HSG B					
	1	09,739	76 V	Veighted A	verage						
		61,740	5	6.26% Per	vious Area						
		47,999	4	3.74% Imp	pervious Ar	ea					
	Тс	Length	Slope	Velocity	Capacity	Description					
((min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	7.1	50	0.0100	0.12		Sheet Flow, A-B					
						Grass: Short n= 0.150 P2= 3.50"					
	0.2	37	0.0300	2.79		Shallow Concentrated Flow, B-C					
						Unpaved Kv= 16.1 fps					
	6.3	400	0.0050	1.06		Shallow Concentrated Flow, B-C					
						Grassed Waterway Kv= 15.0 fps					
	13.6	487	Total								

Subcatchment POST-7: Post Development Area 7



Page 25

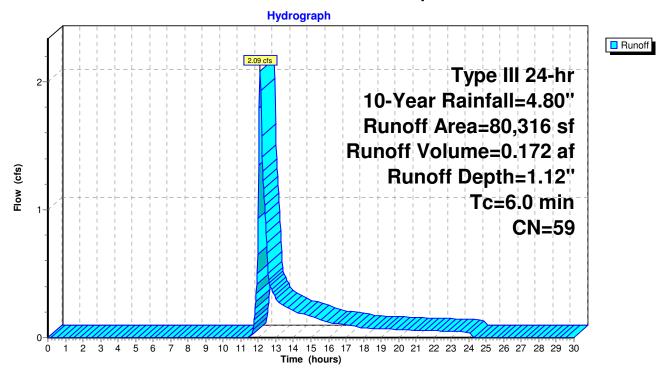
Summary for Subcatchment POST-8: Post Development Area 8

Runoff = 2.09 cfs @ 12.10 hrs, Volume= 0.172 af, Depth= 1.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Year Rainfall=4.80"

	Aı	rea (sf)	CN	Description					
		43,702	55	Woods, Go	od, HSG B				
		28,772	61	>75% Gras	s cover, Go	ood, HSG B			
		1,347	80	>75% Gras	s cover, Go	ood, HSG D			
		6,495	77	Woods, Go	Woods, Good, HSG D				
		80,316	59	59 Weighted Average					
		80,316		100.00% P	ervious Are	a			
	Tc	Length	Slop	,	Capacity	Description			
(n	nin)	(feet)	(ft/f	t) (ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment POST-8: Post Development Area 8



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 26

Summary for Pond AP-1: Easterly Wetland

Analysis Point 1 is taken at the Boundary of the Easterly Wetland System

[40] Hint: Not Described (Outflow=Inflow)

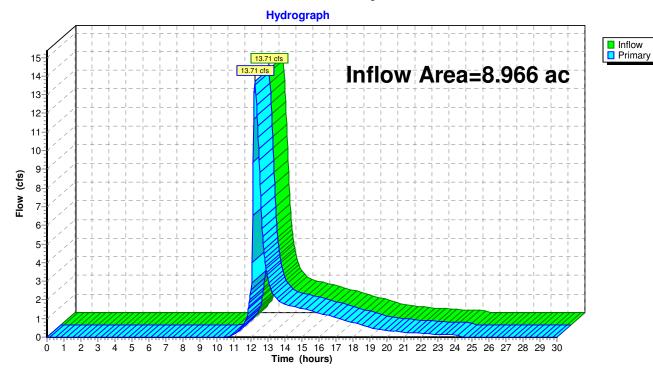
8.966 ac, 48.58% Impervious, Inflow Depth = 1.98" Inflow Area = for 10-Year event

Inflow

13.71 cfs @ 12.23 hrs, Volume= 1.477 af 13.71 cfs @ 12.23 hrs, Volume= 1.477 af, Atten= 0%, Lag= 0.0 min Primary

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-1: Easterly Wetland



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 27

Summary for Pond AP-2: City Drainage System

[40] Hint: Not Described (Outflow=Inflow)

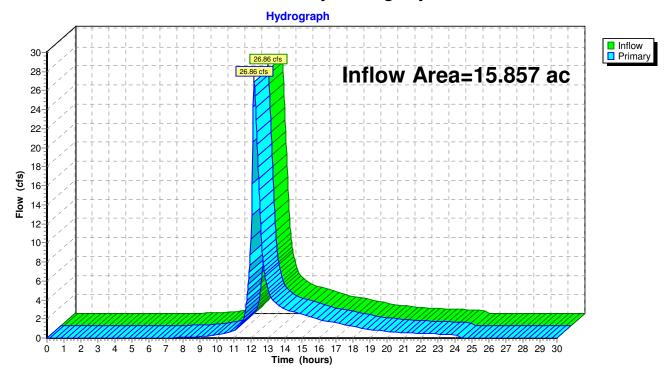
Inflow Area = 15.857 ac, 44.66% Impervious, Inflow Depth = 2.13" for 10-Year event

Inflow = 26.86 cfs @ 12.18 hrs, Volume= 2.812 af

Primary = 26.86 cfs @ 12.18 hrs, Volume= 2.812 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-2: City Drainage System



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 28

Summary for Pond AP-3: Abutting Parcel

[40] Hint: Not Described (Outflow=Inflow)

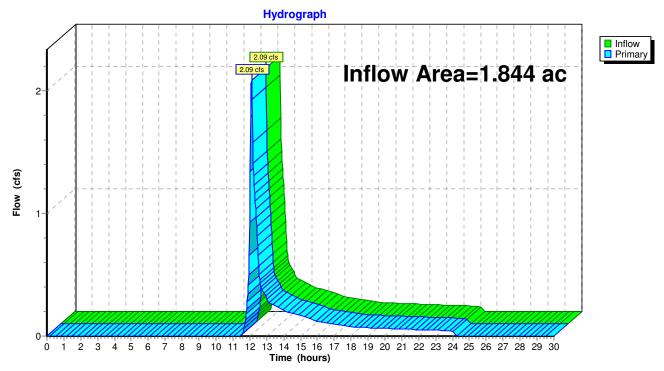
Inflow Area = 1.844 ac, 0.00% Impervious, Inflow Depth = 1.12" for 10-Year event

Inflow = 2.09 cfs @ 12.10 hrs, Volume= 0.172 af

Primary = 2.09 cfs @ 12.10 hrs, Volume= 0.172 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-3: Abutting Parcel



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 29

Summary for Pond AP-4: Duchaine Boulevard

[40] Hint: Not Described (Outflow=Inflow)

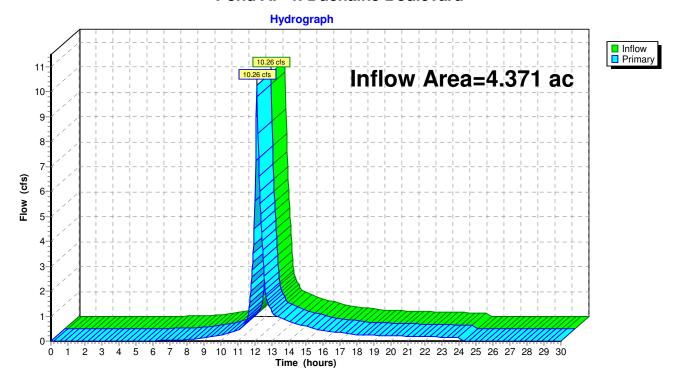
Inflow Area = 4.371 ac, 37.13% Impervious, Inflow Depth = 2.30" for 10-Year event

Inflow = 10.26 cfs @ 12.11 hrs, Volume= 0.837 af

Primary = 10.26 cfs @ 12.11 hrs, Volume= 0.837 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-4: Duchaine Boulevard



Page 30

Summary for Pond POND 1: Extended Detention/Infiltration Basin 1

Inflow Area = 4.821 ac, 81.60% Impervious, Inflow Depth = 3.79" for 10-Year event Inflow 20.44 cfs @ 12.09 hrs, Volume= 1.522 af Outflow 9.89 cfs @ 12.24 hrs, Volume= 1.522 af, Atten= 52%, Lag= 9.5 min Discarded = 0.34 cfs @ 12.24 hrs, Volume= 0.466 af 9.55 cfs @ 12.24 hrs, Volume= Primary 1.056 af 0.00 hrs, Volume= Secondary = 0.00 cfs @ 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Peak Elev= 86.86' @ 12.24 hrs Surf.Area= 14,402 sf Storage= 22,613 cf

Plug-Flow detention time= 117.6 min calculated for 1.522 af (100% of inflow)

Center-of-Mass det. time= 117.6 min (905.8 - 788.2)

Volume	Inver	t Avail.Sto	rage Storage	Description	
#1	85.00	42,12	29 cf Custon	n Stage Data (Pr	ismatic) Listed below (Recalc)
	_				
Elevation	on S	urf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
85.0	00	9,930	0	0	
86.0	00	12,307	11,119	11,119	
87.0	00	14,741	13,524	24,643	
88.0	00	20,231	17,486	42,129	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	85.25'	15.0" Round	Culvert X 2.00	
	,		L= 40.0' CP	P. end-section c	onforming to fill, Ke= 0.500
					35.00' S= 0.0063 '/' Cc= 0.900
			n= 0.013 Co	rrugated PE. sm	ooth interior, Flow Area= 1.23 sf
#2	Device 1	85.25'		ifice/Grate X 2.0	
#3	Device 1	86.40'	4.0' long Sha	arp-Crested Rec	tangular Weir X 2.00
			2 End Contra	•	3
#4	Secondary	87.50'		` '	road-Crested Rectangular Weir
			•		0.80 1.00 1.20 1.40 1.60
			` ,		70 2.69 2.68 2.69 2.67 2.64
#5	Discarded	85.00'	, ,	xfiltration over	

Discarded OutFlow Max=0.34 cfs @ 12.24 hrs HW=86.86' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.34 cfs)

Primary OutFlow Max=9.53 cfs @ 12.24 hrs HW=86.86' (Free Discharge)

1=Culvert (Passes 9.53 cfs of 10.38 cfs potential flow)

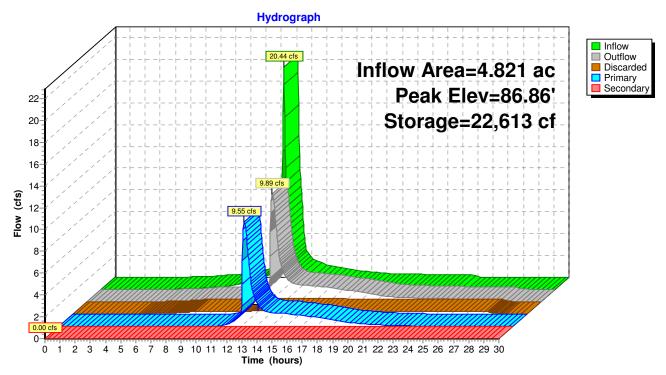
2=Orifice/Grate (Orifice Controls 1.55 cfs @ 5.70 fps)

—3=Sharp-Crested Rectangular Weir (Weir Controls 7.98 cfs @ 2.22 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=85.00' (Free Discharge)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 31

Pond POND 1: Extended Detention/Infiltration Basin 1



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 32

Summary for Pond POND 2: Extended Detention/Infiltration Basin 2

Inflow Area = 0.682 ac, 51.34% Impervious, Inflow Depth = 2.90" for 10-Year event Inflow 2.30 cfs @ 12.09 hrs, Volume= 0.165 af Outflow 0.20 cfs @ 13.18 hrs, Volume= 0.165 af, Atten= 92%, Lag= 65.5 min Discarded = 0.19 cfs @ 13.18 hrs, Volume= 0.163 af 0.01 cfs @ 13.18 hrs, Volume= Primary 0.002 af Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs / 4 Peak Elev= 85.93' @ 13.18 hrs Surf.Area= 7,867 sf Storage= 3,029 cf

Plug-Flow detention time= 152.8 min calculated for 0.165 af (100% of inflow)

Center-of-Mass det. time= 152.6 min (970.6 - 818.0)

Volume	Invert	Avail.Sto	rage Storage	Description				
#1	85.50'	15,30	00 cf Custom	0 cf Custom Stage Data (Prismatic) Listed below (Recalc)				
Elevation	on Su	ırf.Area	Inc.Store	Cum.Store				
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)				
85.5	50	6,257	0	0				
86.0	00	8,134	3,598	3,598				
87.0	00	10,100	9,117	12,715				
87.2	25	10,585	2,586	15,300				
Device	Routing	Invert	Outlet Device	es				
#1	Primary	85.50'	12.0" Round	d Culvert				
	•		L= 21.0' CP	P, square edge h	neadwall, Ke= 0.500			
			Inlet / Outlet	Invert= 85.50' / 8	5.00' S= 0.0238 '/' Cc= 0.900			
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf					
#2	Device 1	85.85'	2.0" Vert. Or	2.0" Vert. Orifice/Grate C= 0.600				
#3	Device 1	86.25'	2.0' long Sha	arp-Crested Rect	tangular Weir 2 End Contraction(s)			
#4	Secondary	86.75'	15.0' long x	10.0' breadth Br	oad-Crested Rectangular Weir			
	-		Head (feet) (0.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60			
			Coef. (Englis	h) 2.49 2.56 2.1	70 2.69 2.68 2.69 2.67 2.64			
#5	Discarded	85.50'	1.020 in/hr E	xfiltration over S	Surface area			

Discarded OutFlow Max=0.19 cfs @ 13.18 hrs HW=85.93' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.19 cfs)

Primary OutFlow Max=0.01 cfs @ 13.18 hrs HW=85.93' (Free Discharge)

1=Culvert (Passes 0.01 cfs of 0.72 cfs potential flow)

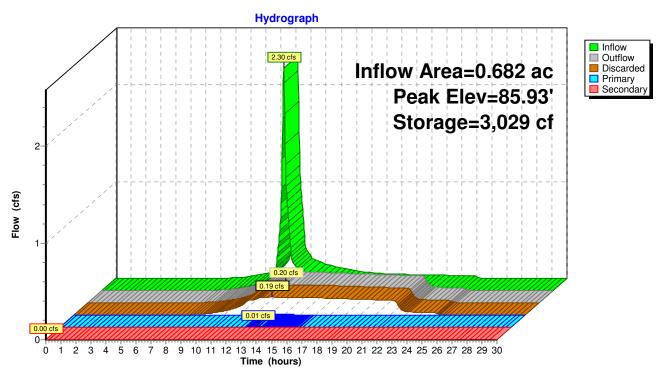
2=Orifice/Grate (Orifice Controls 0.01 cfs @ 0.96 fps)

3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=85.50' (Free Discharge)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 33

Pond POND 2: Extended Detention/Infiltration Basin 2



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 34

Time span=0.00-30.00 hrs, dt=0.03 hrs, 1001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment POST-1: Post Runoff Area=210,010 sf 81.60% Impervious Runoff Depth=4.57"

Tc=6.0 min CN=91 Runoff=24.38 cfs 1.835 af

Subcatchment POST-2: Post Development Runoff Area=29,717 sf 51.34% Impervious Runoff Depth=3.62"

Tc=6.0 min CN=82 Runoff=2.86 cfs 0.206 af

Subcatchment POST-3: Post Development Runoff Area=150,851 sf 2.08% Impervious Runoff Depth=1.98"

Flow Length=390' Slope=0.0100 '/' Tc=13.8 min CN=64 Runoff=6.00 cfs 0.572 af

Subcatchment POST-5: Post Development Runoff Area=66,789 sf 54.70% Impervious Runoff Depth=3.52"

Flow Length=300' Slope=0.0100'/' Tc=9.9 min CN=81 Runoff=5.51 cfs 0.450 af

Subcatchment POST-6: Post Development Runoff Area=43,316 sf 78.89% Impervious Runoff Depth=4.46"

Tc=6.0 min CN=90 Runoff=4.95 cfs 0.369 af

Subcatchment POST-7: Post Runoff Area=109,739 sf 43.74% Impervious Runoff Depth=3.04"

Flow Length=487' Tc=13.6 min CN=76 Runoff=7.03 cfs 0.638 af

Subcatchment POST-8: Post Development Runoff Area=80,316 sf 0.00% Impervious Runoff Depth=1.59"

Tc=6.0 min CN=59 Runoff=3.14 cfs 0.244 af

Pond AP-1: Easterly Wetland Inflow=17.43 cfs 1.925 af

Primary=17.43 cfs 1.925 af

Pond AP-2: City Drainage System Inflow=35.26 cfs 3.626 af

Primary=35.26 cfs 3.626 af

Pond AP-3: Abutting Parcel Inflow=3.14 cfs 0.244 af

Primary=3.14 cfs 0.244 af

Pond AP-4: Duchaine Boulevard Inflow=13.14 cfs 1.063 af

Primary=13.14 cfs 1.063 af

Pond POND 1: Extended Peak Elev=87.08' Storage=25,765 cf Inflow=24.38 cfs 1.835 af

Discarded=0.36 cfs 0.492 af Primary=11.47 cfs 1.343 af Secondary=0.00 cfs 0.000 af Outflow=11.83 cfs 1.835 af

Pond POND 2: Extended Detention/Infiltration Peak Elev=86.04' Storage=3,937 cf Inflow=2.86 cfs 0.206 af

Discarded=0.19 cfs 0.196 af Primary=0.03 cfs 0.010 af Secondary=0.00 cfs 0.000 af Outflow=0.23 cfs 0.206 af

Total Runoff Area = 15.857 ac Runoff Volume = 4.314 af Average Runoff Depth = 3.26" 55.34% Pervious = 8.776 ac 44.66% Impervious = 7.081 ac

Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 35

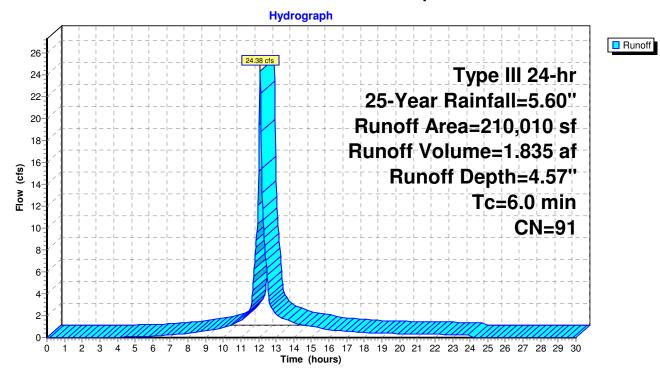
Summary for Subcatchment POST-1: Post Development Area 1

Runoff = 24.38 cfs @ 12.09 hrs, Volume= 1.835 af, Depth= 4.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Year Rainfall=5.60"

Area (st) CN	Description	Description					
117,14	0 98	Roofs, HSG B						
54,22	2 98	Paved park	Paved parking, HSG B					
38,64	8 61	>75% Grass cover, Good, HSG B						
210,010 91 Weighted Average								
38,64	8	18.40% Pei	vious Area	l				
171,36	2	81.60% lmp	ervious Ar	ea				
Tc Leng			Capacity	Description				
(min) (fee	et) (ft/	ft) (ft/sec)	(cfs)					
6.0				Direct Entry				

Subcatchment POST-1: Post Development Area 1



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 36

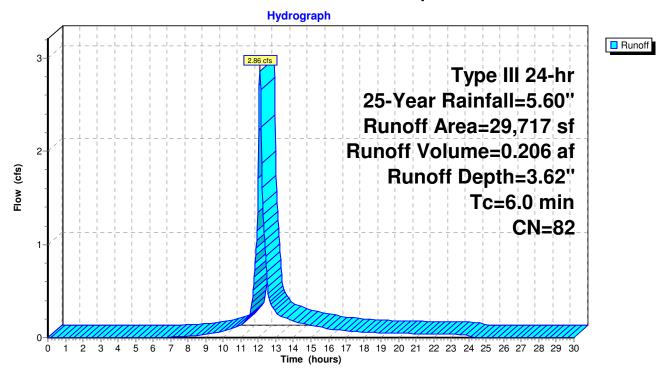
Summary for Subcatchment POST-2: Post Development Area 2

Runoff = 2.86 cfs @ 12.09 hrs, Volume= 0.206 af, Depth= 3.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Year Rainfall=5.60"

	Α	rea (sf)	CN	Description						
		15,256	98	Paved parking, HSG B						
		10,651	61	>75% Grass cover, Good, HSG B						
		3,810	80 :	>75% Grass cover, Good, HSG D						
		29,717	82	82 Weighted Average						
		14,461		48.66% Pervious Area						
		15,256		51.34% lmp	pervious Ar	ea				
	Tc	Length	Slope	•	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry				

Subcatchment POST-2: Post Development Area 2



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 37

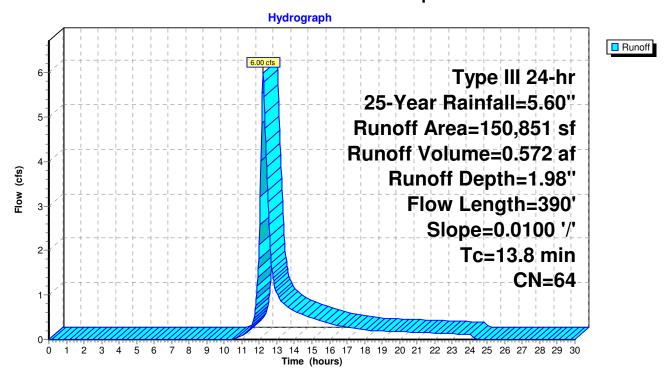
Summary for Subcatchment POST-3: Post Development Area 3

Runoff = 6.00 cfs @ 12.20 hrs, Volume= 0.572 af, Depth= 1.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Year Rainfall=5.60"

A	rea (sf)	CN [Description			
	3,139	98 F	aved park	ing, HSG B		
15,504 61 >75% Grass cover, Good, HSG B						
11,430 80 >75% Grass cover, Good, HSG D						
	81,786		,	od, HSG B		
	38,992	77 V	Voods, Go	od, HSG D		
150,851 64 Weighted Average						
1	47,712	S	7.92% Per	vious Area		
	3,139	2	2.08% Impe	ervious Area	a	
_						
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
10.3	50	0.0100	0.08		Sheet Flow, A-B	
					Grass: Dense n= 0.240 P2= 3.50"	
3.5	340	0.0100	1.61		Shallow Concentrated Flow, B-C	
					Unpaved Kv= 16.1 fps	
13.8	390	Total				

Subcatchment POST-3: Post Development Area 3



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 38

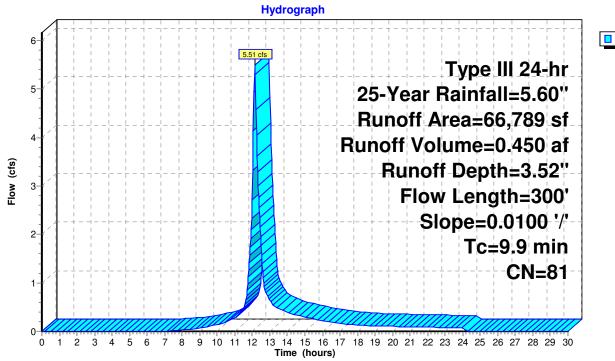
Summary for Subcatchment POST-5: Post Development Area 5

Runoff = 5.51 cfs @ 12.14 hrs, Volume= 0.450 af, Depth= 3.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Year Rainfall=5.60"

_	Α	rea (sf)	CN D	CN Description						
		36,536			ing, HSG B					
		30,253	61 >	75% Gras	s cover, Go	ood, HSG B				
	66,789 81 Weighted Average									
30,253 45.30% Pervious Area										
		36,536	5	4.70% Imp	ervious Ar	ea				
·										
	Тс	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	7.1	50	0.0100	0.12		Sheet Flow, A-B				
						Grass: Short n= 0.150 P2= 3.50"				
	2.8	250	0.0100	1.50		Shallow Concentrated Flow, B-C				
						Grassed Waterway Kv= 15.0 fps				
	9.9	300	Total	·	·					

Subcatchment POST-5: Post Development Area 5





Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 39

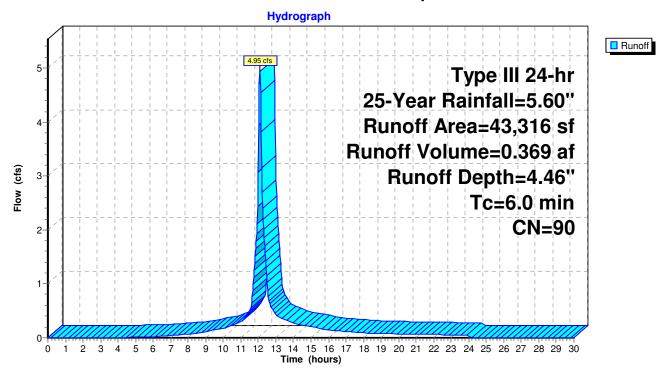
Summary for Subcatchment POST-6: Post Development Area 6

Runoff = 4.95 cfs @ 12.09 hrs, Volume= 0.369 af, Depth= 4.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Year Rainfall=5.60"

Area	(sf) CN	Description	Description						
34	171 98	Paved park	Paved parking, HSG B						
9,	145 61	>75% Gras	>75% Grass cover, Good, HSG B						
43.	43,316 90 Weighted Average								
9.	145	21.11% Per	rvious Area	a					
34	34,171 78.89% Impervious Are			rea					
	ength Slo	,	Capacity	•					
	(feet) (ft	/ft) (ft/sec)	(cfs)						
6.0				Direct Entry,					

Subcatchment POST-6: Post Development Area 6



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 40

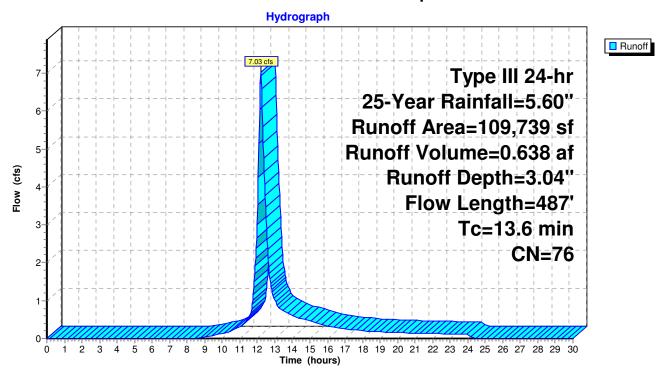
Summary for Subcatchment POST-7: Post Development Area 7

Runoff = 7.03 cfs @ 12.19 hrs, Volume= 0.638 af, Depth= 3.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Year Rainfall=5.60"

	Area (sf) CN Description						
47,999 98 Paved parking, HSG B						}	
		7,096	39 >	75% Gras	s cover, Go	ood, HSG A	
54,644 61 >75% Grass cover, God					s cover, Go	ood, HSG B	
	109,739 76 Weighted Average						
61,740			5	6.26% Per	vious Area		
	47,999 43.74% Impervious Are					ea	
	Tc	Length	Slope	Velocity	Capacity	Description	
((min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	7.1	50	0.0100	0.12		Sheet Flow, A-B	
						Grass: Short n= 0.150 P2= 3.50"	
	0.2	37	0.0300	2.79		Shallow Concentrated Flow, B-C	
						Unpaved Kv= 16.1 fps	
	6.3	400	0.0050	1.06		Shallow Concentrated Flow, B-C	
						Grassed Waterway Kv= 15.0 fps	
	13.6	487	Total				

Subcatchment POST-7: Post Development Area 7



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 41

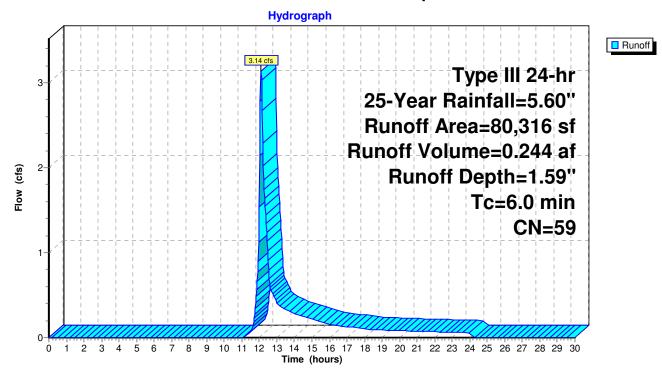
Summary for Subcatchment POST-8: Post Development Area 8

Runoff = 3.14 cfs @ 12.10 hrs, Volume= 0.244 af, Depth= 1.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Year Rainfall=5.60"

	Ar	ea (sf)	CN	Description					
		43,702	55	Woods, Good, HSG B					
		28,772	61	>75% Grass cover, Good, HSG B					
		1,347	80	>75% Grass cover, Good, HSG D					
		6,495	77	Woods, Good, HSG D					
		80,316	59	59 Weighted Average					
		80,316		100.00% Pe	ervious Are	a			
	Tc	Length	Slope	e Velocity	Capacity	Description			
(n	nin)	(feet)	(ft/ft) (ft/sec)	(cfs)				
	6.0					Direct Entry			

Subcatchment POST-8: Post Development Area 8



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 42

Summary for Pond AP-1: Easterly Wetland

Analysis Point 1 is taken at the Boundary of the Easterly Wetland System

[40] Hint: Not Described (Outflow=Inflow)

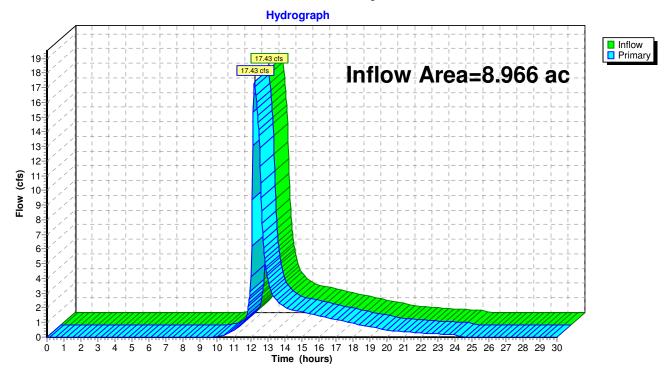
Inflow Area = 8.966 ac, 48.58% Impervious, Inflow Depth = 2.58" for 25-Year event

Inflow = 17.43 cfs @ 12.21 hrs, Volume= 1.925 af

Primary = 17.43 cfs @ 12.21 hrs, Volume= 1.925 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-1: Easterly Wetland



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 43

Summary for Pond AP-2: City Drainage System

[40] Hint: Not Described (Outflow=Inflow)

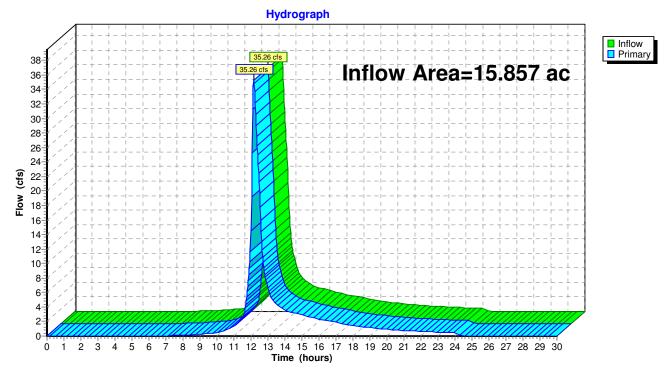
Inflow Area = 15.857 ac, 44.66% Impervious, Inflow Depth = 2.74" for 25-Year event

Inflow = 35.26 cfs @ 12.15 hrs, Volume= 3.626 af

Primary = 35.26 cfs @ 12.15 hrs, Volume= 3.626 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-2: City Drainage System



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 44

Summary for Pond AP-3: Abutting Parcel

[40] Hint: Not Described (Outflow=Inflow)

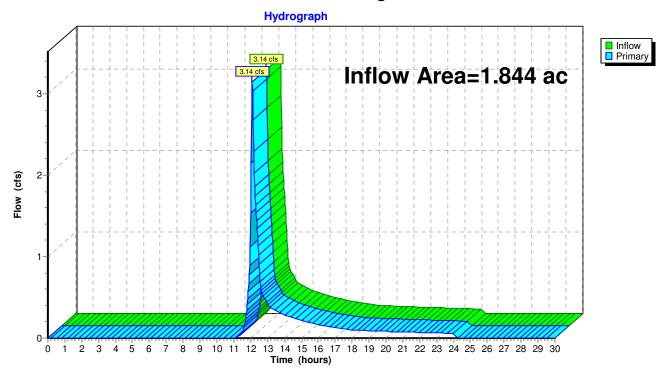
Inflow Area = 1.844 ac, 0.00% Impervious, Inflow Depth = 1.59" for 25-Year event

Inflow = 3.14 cfs @ 12.10 hrs, Volume= 0.244 af

Primary = 3.14 cfs @ 12.10 hrs, Volume= 0.244 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-3: Abutting Parcel



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 45

Summary for Pond AP-4: Duchaine Boulevard

[40] Hint: Not Described (Outflow=Inflow)

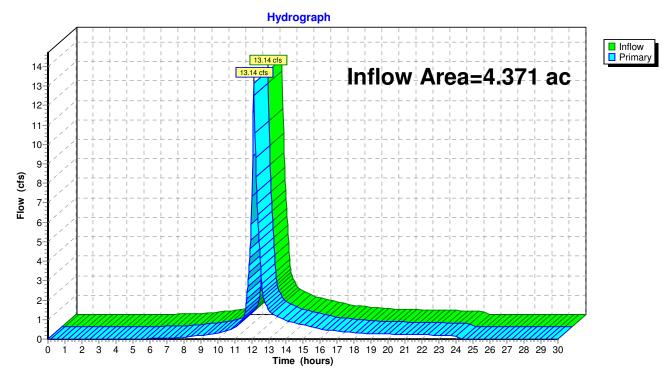
Inflow Area = 4.371 ac, 37.13% Impervious, Inflow Depth = 2.92" for 25-Year event

Inflow = 13.14 cfs @ 12.11 hrs, Volume= 1.063 af

Primary = 13.14 cfs @ 12.11 hrs, Volume= 1.063 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-4: Duchaine Boulevard



Page 46

Summary for Pond POND 1: Extended Detention/Infiltration Basin 1

Inflow Area = 4.821 ac, 81.60% Impervious, Inflow Depth = 4.57" for 25-Year event Inflow 24.38 cfs @ 12.09 hrs, Volume= 1.835 af Outflow 11.83 cfs @ 12.24 hrs, Volume= 1.835 af, Atten= 51%, Lag= 9.3 min Discarded = 0.36 cfs @ 12.24 hrs, Volume= 0.492 af 11.47 cfs @ 12.24 hrs, Volume= Primary 1.343 af 0.00 hrs, Volume= Secondary = 0.00 cfs @ 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Peak Elev= 87.08' @ 12.24 hrs Surf.Area= 15,153 sf Storage= 25,765 cf

Plug-Flow detention time= 109.0 min calculated for 1.833 af (100% of inflow)

Center-of-Mass det. time= 109.2 min (892.3 - 783.2)

Volume	Invert	Avail.Sto	rage Storage	Description			
#1	85.00'	42,12	29 cf Custom	n Stage Data (Pr	ismatic) Listed below (Recalc)		
Elevation	on Su	ırf.Area	Inc.Store	Cum.Store			
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)			
85.0	00	9,930	0				
86.0	00	12,307	11,119	11,119			
87.0	00	14,741	13,524	24,643			
88.0	00	20,231	17,486	42,129			
Device	Routing	Invert	Outlet Device	es			
#1	Primary	85.25'	15.0" Round	Culvert X 2.00			
	•		L= 40.0' CPP, end-section conforming to fill, Ke= 0.500				
			Inlet / Outlet Invert= 85.25' / 85.00' S= 0.0063 '/' Cc= 0.900				
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf				
#2	Device 1	85.25'	5.0" Vert. Or	ifice/Grate X 2.0	0 C= 0.600		
#3	Device 1	86.40'	4.0' long Sha	arp-Crested Rec	tangular Weir X 2.00		
			2 End Contra	iction(s)			
#4	Secondary	87.50'	15.0' long x	10.0' breadth Bi	road-Crested Rectangular Weir		
			Head (feet) (Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60			
			Coef. (Englis	h) 2.49 2.56 2.	70 2.69 2.68 2.69 2.67 2.64		
#5	Discarded	85.00'	1.020 in/hr Exfiltration over Surface area				

Discarded OutFlow Max=0.36 cfs @ 12.24 hrs HW=87.07' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.36 cfs)

Primary OutFlow Max=11.47 cfs @ 12.24 hrs HW=87.07' (Free Discharge)

-1=Culvert (Barrel Controls 11.47 cfs @ 4.67 fps)

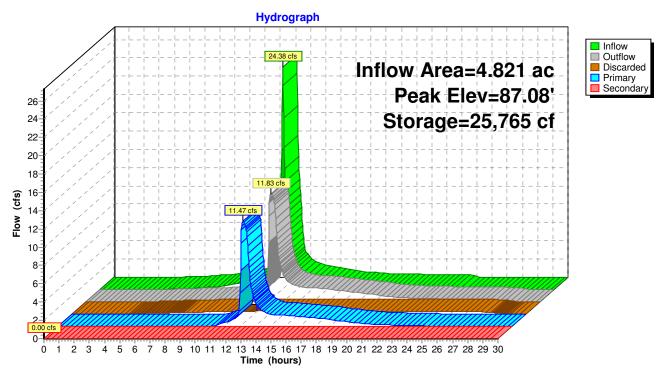
2=Orifice/Grate (Passes < 1.67 cfs potential flow)

-3=Sharp-Crested Rectangular Weir (Passes < 14.02 cfs potential flow)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=85.00' (Free Discharge)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 47

Pond POND 1: Extended Detention/Infiltration Basin 1



Page 48

Summary for Pond POND 2: Extended Detention/Infiltration Basin 2

Inflow Area = 0.682 ac, 51.34% Impervious, Inflow Depth = 3.62" for 25-Year event Inflow 2.86 cfs @ 12.09 hrs, Volume= 0.206 af Outflow 0.23 cfs @ 13.29 hrs, Volume= 0.206 af, Atten= 92%, Lag= 71.9 min Discarded = 0.19 cfs @ 13.29 hrs, Volume= 0.196 af 0.03 cfs @ 13.29 hrs, Volume= Primary 0.010 af 0.00 hrs, Volume= Secondary = 0.00 cfs @ 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs / 4 Peak Elev= 86.04' @ 13.29 hrs Surf.Area= 8,215 sf Storage= 3,937 cf

Plug-Flow detention time= 181.8 min calculated for 0.206 af (100% of inflow)

Center-of-Mass det. time= 181.6 min (993.3 - 811.7)

Volume	Invert	Avail.Sto	rage Storage	Description				
#1	85.50'	15,30	00 cf Custom	n Stage Data (Pri	ismatic) Listed below (Recalc)			
Elevation	on Su	ırf.Area	Inc.Store	Cum.Store				
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)				
85.5	50	6,257	0	0				
86.0	00	8,134	3,598	3,598				
87.0	00	10,100	9,117	12,715				
87.2	25	10,585	2,586	15,300				
Device	Routing	Invert	Outlet Device	es				
#1	Primary	85.50'	12.0" Round	d Culvert				
	•		L= 21.0' CPP, square edge headwall, Ke= 0.500					
			Inlet / Outlet	Inlet / Outlet Invert= 85.50' / 85.00' S= 0.0238 '/' Cc= 0.900				
			n= 0.013 Co	rrugated PE, sme	ooth interior, Flow Area= 0.79 sf			
#2	Device 1	85.85'	2.0" Vert. Or	ifice/Grate C=	0.600			
#3	Device 1	86.25'	2.0' long Sha	arp-Crested Rec	tangular Weir 2 End Contraction(s)			
#4	Secondary	86.75'	15.0' long x	10.0' breadth Br	oad-Crested Rectangular Weir			
	-		Head (feet) (0.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60			
					70 2.69 2.68 2.69 2.67 2.64			
#5	Discarded	85.50'	1.020 in/hr E	xfiltration over S	Surface area			

Discarded OutFlow Max=0.19 cfs @ 13.29 hrs HW=86.04' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.19 cfs)

Primary OutFlow Max=0.03 cfs @ 13.29 hrs HW=86.04' (Free Discharge)

-1=Culvert (Passes 0.03 cfs of 1.09 cfs potential flow)

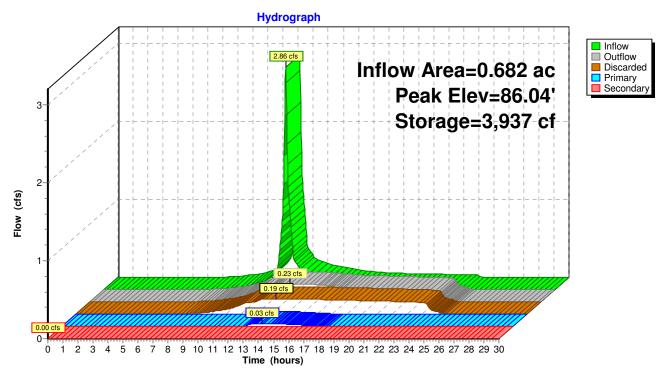
-2=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.58 fps)

3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=85.50' (Free Discharge) 4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 49

Pond POND 2: Extended Detention/Infiltration Basin 2



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 50

Time span=0.00-30.00 hrs, dt=0.03 hrs, 1001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment POST-1: Post Runoff Area=210,010 sf 81.60% Impervious Runoff Depth=5.94"

Tc=6.0 min CN=91 Runoff=31.23 cfs 2.386 af

Subcatchment POST-2: Post Development Runoff Area=29,717 sf 51.34% Impervious Runoff Depth=4.92"

Tc=6.0 min CN=82 Runoff=3.85 cfs 0.279 af

Subcatchment POST-3: Post Development Runoff Area=150,851 sf 2.08% Impervious Runoff Depth=3.00"

Flow Length=390' Slope=0.0100 '/' Tc=13.8 min CN=64 Runoff=9.33 cfs 0.866 af

Subcatchment POST-5: Post Development Runoff Area=66,789 sf 54.70% Impervious Runoff Depth=4.81"

Flow Length=300' Slope=0.0100 '/' Tc=9.9 min CN=81 Runoff=7.45 cfs 0.614 af

Subcatchment POST-6: Post Development Runoff Area=43,316 sf 78.89% Impervious Runoff Depth=5.82"

Tc=6.0 min CN=90 Runoff=6.37 cfs 0.483 af

Subcatchment POST-7: Post Runoff Area=109,739 sf 43.74% Impervious Runoff Depth=4.26"

Flow Length=487' Tc=13.6 min CN=76 Runoff=9.85 cfs 0.894 af

Subcatchment POST-8: Post Development Runoff Area=80,316 sf 0.00% Impervious Runoff Depth=2.51"

Tc=6.0 min CN=59 Runoff=5.20 cfs 0.385 af

Pond AP-1: Easterly Wetland Inflow=23.06 cfs 2.751 af

Primary=23.06 cfs 2.751 af

Pond AP-2: City Drainage System Inflow=48.13 cfs 5.126 af

Primary=48.13 cfs 5.126 af

Pond AP-3: Abutting Parcel Inflow=5.20 cfs 0.385 af

Primary=5.20 cfs 0.385 af

Pond AP-4: Duchaine Boulevard Inflow=18.47 cfs 1.482 af

Primary=18.47 cfs 1.482 af

Pond POND 1: Extended Peak Elev=87.44' Storage=31,726 cf Inflow=31.23 cfs 2.386 af

Discarded=0.41 cfs 0.530 af Primary=13.80 cfs 1.856 af Secondary=0.00 cfs 0.000 af Outflow=14.20 cfs 2.386 af

Pond POND 2: Extended Detention/Infiltration Peak Elev=86.25' Storage=5,723 cf Inflow=3.85 cfs 0.279 af

Discarded=0.20 cfs 0.250 af Primary=0.06 cfs 0.029 af Secondary=0.00 cfs 0.000 af Outflow=0.27 cfs 0.279 af

Total Runoff Area = 15.857 ac Runoff Volume = 5.907 af Average Runoff Depth = 4.47" 55.34% Pervious = 8.776 ac 44.66% Impervious = 7.081 ac

Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 51

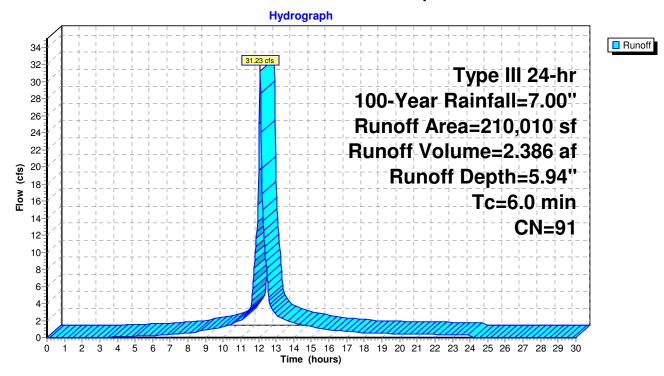
Summary for Subcatchment POST-1: Post Development Area 1

Runoff = 31.23 cfs @ 12.09 hrs, Volume= 2.386 af, Depth= 5.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Year Rainfall=7.00"

	Α	rea (sf)	CN	Description						
	1	17,140	98	Roofs, HSG B						
		54,222	98	Paved parking, HSG B						
		38,648	61	>75% Grass cover, Good, HSG B						
	2	10,010	91 Weighted Average							
		38,648		18.40% Per	vious Area	a e e e e e e e e e e e e e e e e e e e				
	1	71,362		81.60% lmp	pervious Ar	rea				
	Tc	Length	Slope	,	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment POST-1: Post Development Area 1



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 52

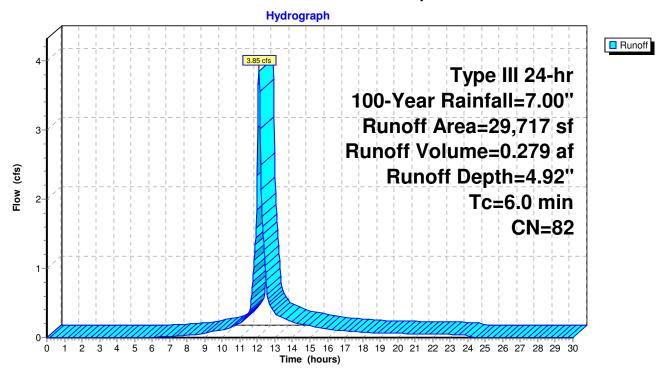
Summary for Subcatchment POST-2: Post Development Area 2

Runoff = 3.85 cfs @ 12.09 hrs, Volume= 0.279 af, Depth= 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Year Rainfall=7.00"

 Α	rea (sf)	CN	Description						
	15,256	98	Paved parking, HSG B						
	10,651	61	>75% Grass cover, Good, HSG B						
	3,810	80	>75% Grass cover, Good, HSG D						
	29,717	82	Weighted Average						
	14,461		48.66% Pervious Area						
	15,256		51.34% lmp	pervious Ar	ea				
Tc	Length	Slope	,	Capacity	Description				
(min)	(feet)	(ft/ft)	t) (ft/sec) (cfs)						
6.0					Direct Entry,				

Subcatchment POST-2: Post Development Area 2



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 53

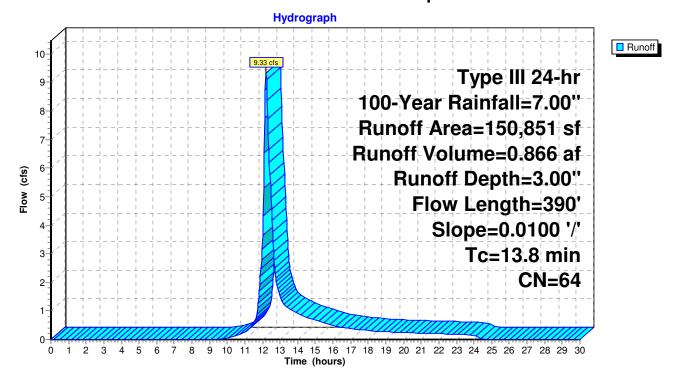
Summary for Subcatchment POST-3: Post Development Area 3

Runoff = 9.33 cfs @ 12.20 hrs, Volume= 0.866 af, Depth= 3.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Year Rainfall=7.00"

Aı	rea (sf)	CN I	Description					
	3,139 98 Paved parking, HSG B							
15,504 61 >75% Grass cover, Good, HSG B								
11,430 80 >75% Grass cover, Good, HSG D								
81,786 55 Woods, Good, HSG B								
	38,992	77 \	Noods, Go	od, HSG D				
150,851 64 Weighted Average								
147,712 97.92% Pervious Area								
3,139 2.08% Impervious Area					a			
Tc	Length	Slope		Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
10.3	50	0.0100	0.08		Sheet Flow, A-B			
					Grass: Dense n= 0.240 P2= 3.50"			
3.5	340	0.0100	1.61		Shallow Concentrated Flow, B-C			
					Unpaved Kv= 16.1 fps			
13.8	390	Total						

Subcatchment POST-3: Post Development Area 3



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 54

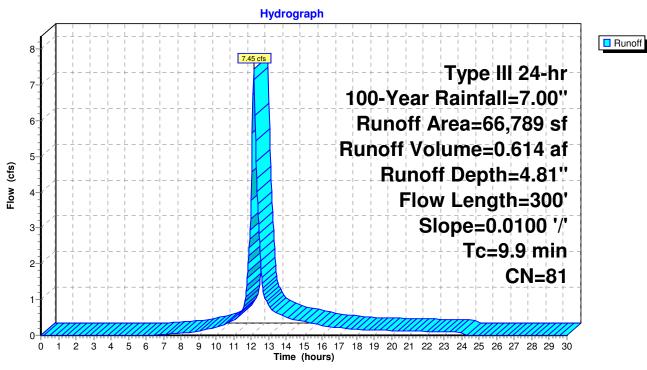
Summary for Subcatchment POST-5: Post Development Area 5

Runoff = 7.45 cfs @ 12.14 hrs, Volume= 0.614 af, Depth= 4.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Year Rainfall=7.00"

	Α	rea (sf)	CN D	escription			
	36,536 98 Paved parking, HSG B						
	30,253 61 >75% Grass cover, Good, HSG B						
66,789 81 Weighted Average					verage		
30,253 45.30% Pervious Area							
36,536 54.70% Impervious Area					pervious Ar	ea	
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	7.1	50	0.0100	0.12		Sheet Flow, A-B	
						Grass: Short n= 0.150 P2= 3.50"	
	2.8	250	0.0100	1.50		Shallow Concentrated Flow, B-C	
_						Grassed Waterway Kv= 15.0 fps	
	99	300	Total				

Subcatchment POST-5: Post Development Area 5



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 55

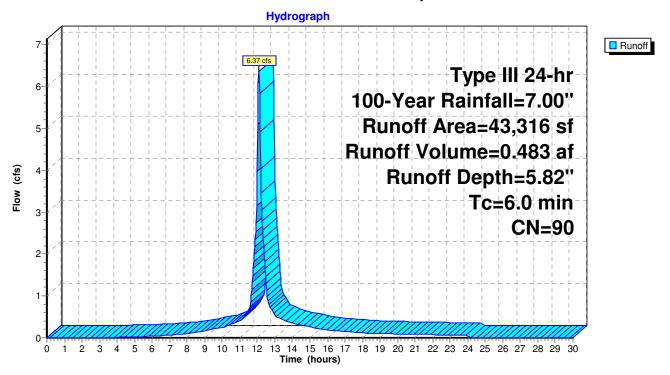
Summary for Subcatchment POST-6: Post Development Area 6

Runoff = 6.37 cfs @ 12.09 hrs, Volume= 0.483 af, Depth= 5.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Year Rainfall=7.00"

A	rea (sf)	CN	Description						
	34,171	98	Paved parking, HSG B						
	9,145	61	>75% Grass cover, Good, HSG B						
	43,316 90 Weighted Average								
	9,145		21.11% Per	vious Area	a e e e e e e e e e e e e e e e e e e e				
34,171		78.89% Impervious Are			rea				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
6.0					Direct Entry,				

Subcatchment POST-6: Post Development Area 6



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 56

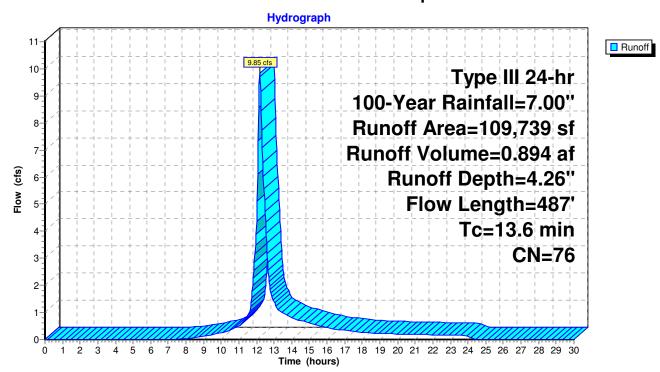
Summary for Subcatchment POST-7: Post Development Area 7

Runoff = 9.85 cfs @ 12.19 hrs, Volume= 0.894 af, Depth= 4.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Year Rainfall=7.00"

	Α	rea (sf)	CN D	Description						
		47,999	98 F	Paved parking, HSG B						
	7,096 39 >75% Grass cover, Good, HSG A									
		54,644 61 >75% Grass cover, Good, HSG B								
109,739 76 Weighted Average					verage					
		61,740	5	6.26% Per	vious Area					
		47,999	4	3.74% Imp	ervious Ar	ea				
	Тс	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	7.1	50	0.0100	0.12		Sheet Flow, A-B				
						Grass: Short n= 0.150 P2= 3.50"				
	0.2 37 0.0300 2.79			Shallow Concentrated Flow, B-C						
				Unpaved Kv= 16.1 fps						
	6.3	400	0.0050	1.06		Shallow Concentrated Flow, B-C				
Grassed Waterway Kv= 1				Grassed Waterway Kv= 15.0 fps						
	13.6	487	Total							

Subcatchment POST-7: Post Development Area 7



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 57

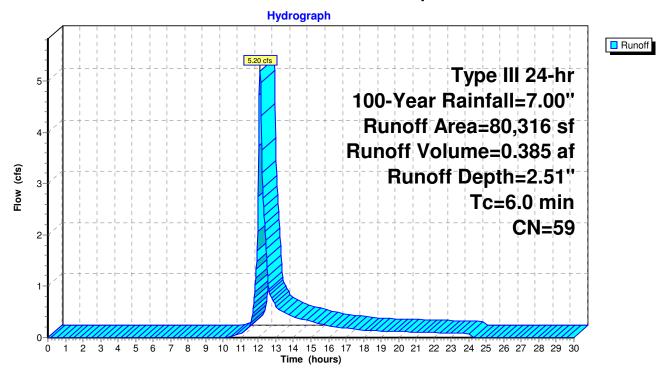
Summary for Subcatchment POST-8: Post Development Area 8

Runoff = 5.20 cfs @ 12.10 hrs, Volume= 0.385 af, Depth= 2.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Year Rainfall=7.00"

Ar	rea (sf)	CN	Description					
-	43,702	55	Woods, Good, HSG B					
	28,772	61	>75% Grass cover, Good, HSG B					
	1,347	80	>75% Grass cover, Good, HSG D					
	6,495	77	Woods, Good, HSG D					
	80,316	59 Weighted Average						
	80,316		100.00% Pe	ervious Are	a			
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)) (ft/sec)	(cfs)				
6.0					Direct Entry.			

Subcatchment POST-8: Post Development Area 8



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 58

Summary for Pond AP-1: Easterly Wetland

Analysis Point 1 is taken at the Boundary of the Easterly Wetland System

[40] Hint: Not Described (Outflow=Inflow)

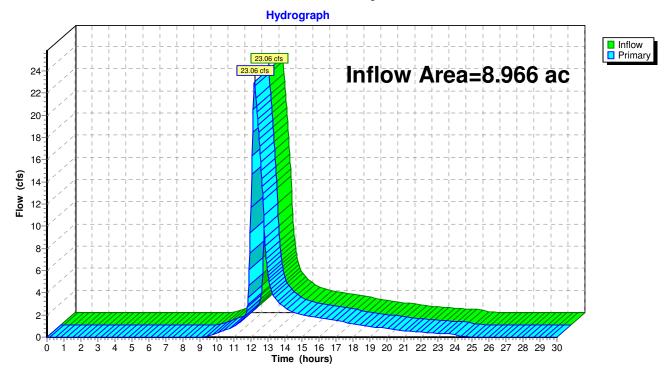
Inflow Area = 8.966 ac, 48.58% Impervious, Inflow Depth = 3.68" for 100-Year event

Inflow = 23.06 cfs @ 12.21 hrs, Volume= 2.751 af

Primary = 23.06 cfs @ 12.21 hrs, Volume= 2.751 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-1: Easterly Wetland



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 59

Summary for Pond AP-2: City Drainage System

[40] Hint: Not Described (Outflow=Inflow)

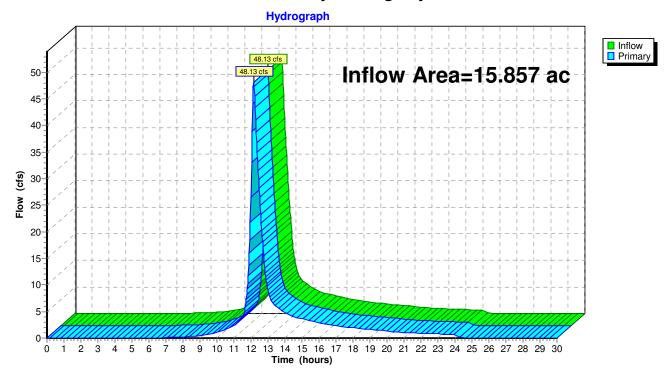
Inflow Area = 15.857 ac, 44.66% Impervious, Inflow Depth = 3.88" for 100-Year event

Inflow = 48.13 cfs @ 12.15 hrs, Volume= 5.126 af

Primary = 48.13 cfs @ 12.15 hrs, Volume= 5.126 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-2: City Drainage System



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 60

Summary for Pond AP-3: Abutting Parcel

[40] Hint: Not Described (Outflow=Inflow)

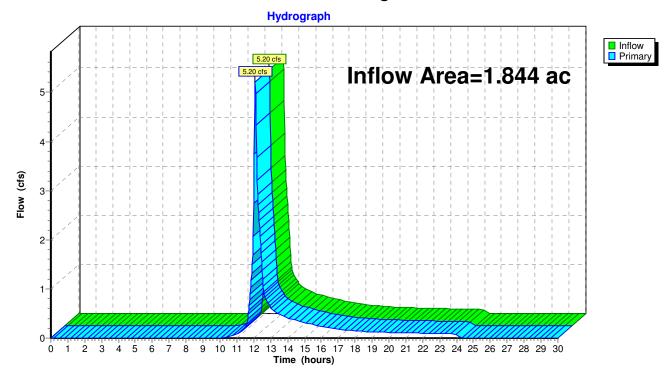
Inflow Area = 1.844 ac, 0.00% Impervious, Inflow Depth = 2.51" for 100-Year event

Inflow = 5.20 cfs @ 12.10 hrs, Volume= 0.385 af

Primary = 5.20 cfs @ 12.10 hrs, Volume= 0.385 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-3: Abutting Parcel



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 61

Summary for Pond AP-4: Duchaine Boulevard

[40] Hint: Not Described (Outflow=Inflow)

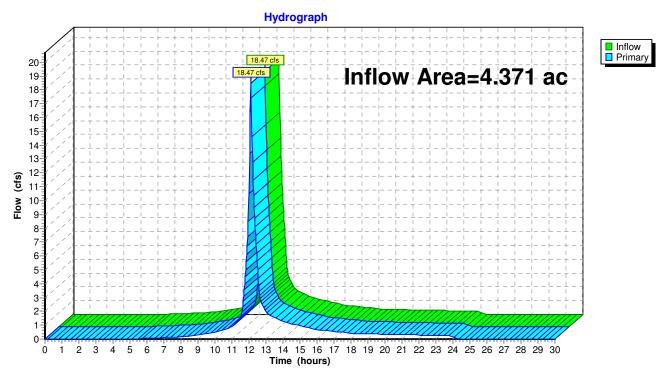
Inflow Area = 4.371 ac, 37.13% Impervious, Inflow Depth = 4.07" for 100-Year event

Inflow = 18.47 cfs @ 12.10 hrs, Volume= 1.482 af

Primary = 18.47 cfs @ 12.10 hrs, Volume= 1.482 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs

Pond AP-4: Duchaine Boulevard



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 62

Summary for Pond POND 1: Extended Detention/Infiltration Basin 1

Inflow Area = 4.821 ac, 81.60% Impervious, Inflow Depth = 5.94" for 100-Year event Inflow 31.23 cfs @ 12.09 hrs, Volume= 2.386 af 14.20 cfs @ 12.26 hrs, Volume= Outflow 2.386 af, Atten= 55%, Lag= 10.4 min 0.41 cfs @ 12.26 hrs, Volume= Discarded = 0.530 af 13.80 cfs @ 12.26 hrs, Volume= Primary 1.856 af Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs Peak Elev= 87.44' @ 12.26 hrs Surf.Area= 17,178 sf Storage= 31,726 cf

Plug-Flow detention time= 99.2 min calculated for 2.384 af (100% of inflow)

Center-of-Mass det. time= 99.4 min (875.8 - 776.4)

Volume	Invert	Avail.Stor	age Storage	e Description			
#1	85.00'	42,12	9 cf Custon	n Stage Data (Pr	ismatic) Listed below (Recalc)		
				•	•		
Elevation	on Su	ırf.Area	Inc.Store	Cum.Store			
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)			
85.0	00	9,930	0	0			
86.0	00	12,307	11,119	11,119			
87.0	00	14,741	13,524	24,643			
88.0	00	20,231	17,486	42,129			
Device	Routing	Invert	Outlet Device	es			
#1	Primary	85.25'	15.0" Round	d Culvert X 2.00			
	•		L= 40.0' CP	P, end-section c	onforming to fill, Ke= 0.500		
			Inlet / Outlet	Invert= 85.25' / 8	35.00' S= 0.0063 '/' Cc= 0.900		
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf				
#2	Device 1	85.25'					
#3	Device 1	86.40'	4.0' long Sharp-Crested Rectangular Weir X 2.00				
			2 End Contra	\ /			
#4	Secondary	87.50'			oad-Crested Rectangular Weir		
			, ,		0.80 1.00 1.20 1.40 1.60		
			, ,	,	70 2.69 2.68 2.69 2.67 2.64		
#5	Discarded	85.00'	1.020 in/hr E	xfiltration over	Surface area		

Discarded OutFlow Max=0.41 cfs @ 12.26 hrs HW=87.44' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.41 cfs)

Primary OutFlow Max=13.79 cfs @ 12.26 hrs HW=87.44' (Free Discharge)

-1=Culvert (Barrel Controls 13.79 cfs @ 5.62 fps)

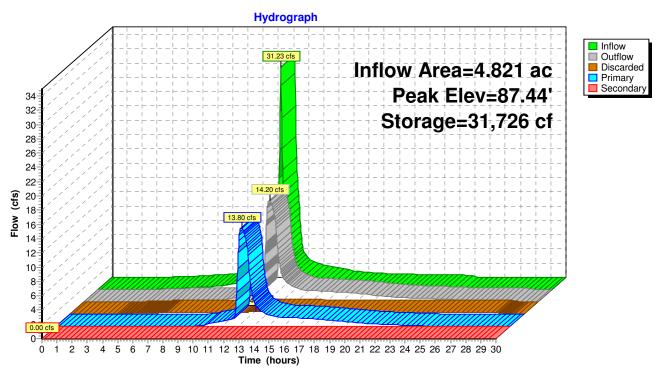
2=Orifice/Grate (Passes < 1.85 cfs potential flow)

☐3=Sharp-Crested Rectangular Weir (Passes < 26.41 cfs potential flow)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=85.00' (Free Discharge)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 63

Pond POND 1: Extended Detention/Infiltration Basin 1



Prepared by Field Engineering Co. Inc.

HydroCAD® 10.00-12 s/n 01897 © 2014 HydroCAD Software Solutions LLC

Page 64

Summary for Pond POND 2: Extended Detention/Infiltration Basin 2

Inflow Area = 0.682 ac, 51.34% Impervious, Inflow Depth = 4.92" for 100-Year event Inflow 3.85 cfs @ 12.09 hrs, Volume= 0.279 af 0.27 cfs @ 13.61 hrs, Volume= Outflow 0.279 af, Atten= 93%, Lag= 91.3 min 0.20 cfs @ 13.61 hrs, Volume= Discarded = 0.250 af 0.06 cfs @ 13.61 hrs, Volume= Primary 0.029 af Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.03 hrs / 4 Peak Elev= 86.25' @ 13.61 hrs Surf.Area= 8,632 sf Storage= 5,723 cf

Plug-Flow detention time= 233.8 min calculated for 0.279 af (100% of inflow)

Center-of-Mass det. time= 233.7 min (1,036.7 - 803.0)

Volume	Invert	Avail.Sto	rage Storage	Description			
#1	85.50'	15,30	00 cf Custom	n Stage Data (Pris	smatic) Listed below (Recalc)		
Elevation	an Ci	ırf.Area	Inc.Store	Cum.Store			
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)			
85.5	50	6,257	0	0			
86.0	00	8,134	3,598	3,598			
87.0	00	10,100	9,117	12,715			
87.2		10,585	2,586	15,300			
ъ .	ъ .:		0 11 1 5 1				
Device	Routing	Invert	Outlet Device	es			
#1	Primary	85.50'	12.0" Round	l Culvert			
	_		L= 21.0' CP	P, square edge he	eadwall, Ke= 0.500		
			Inlet / Outlet	Invert= 85.50' / 85	5.00' S= 0.0238 '/' Cc= 0.900		
					oth interior, Flow Area= 0.79 sf		
#2	Device 1	85.85'	2.0" Vert. Orifice/Grate C= 0.600				
#3	Device 1	86.25'			angular Weir 2 End Contraction(s)		
#4	Secondary	86.75'	•	•	pad-Crested Rectangular Weir		
" '	Coochdary	00.70	_		0.80 1.00 1.20 1.40 1.60		
					0 2.69 2.68 2.69 2.67 2.64		
#5	Discorded	0E E0!	, ,	,			
#5	Discarded	85.50'		xfiltration over S	uriace area		

Discarded OutFlow Max=0.20 cfs @ 13.61 hrs HW=86.25' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.20 cfs)

Primary OutFlow Max=0.06 cfs @ 13.61 hrs HW=86.25' (Free Discharge)

1=Culvert (Passes 0.06 cfs of 1.88 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.06 cfs @ 2.72 fps)

3=Sharp-Crested Rectangular Weir (Weir Controls 0.00 cfs @ 0.19 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=85.50' (Free Discharge)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 65

Pond POND 2: Extended Detention/Infiltration Basin 2

