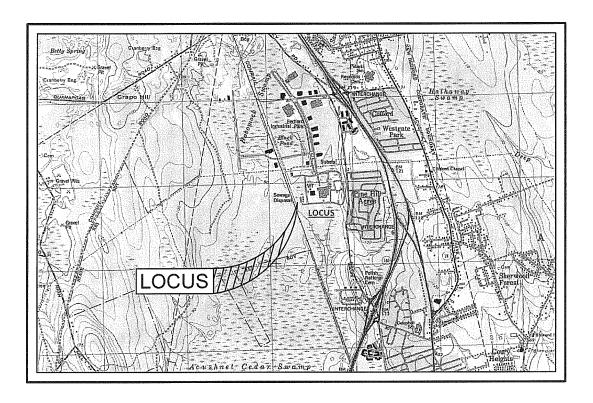


STORMWATER REPORT

SITE PLAN

ASSESSORS MAP 134 - LOT 5 100 DUCHAINE BLVD. NEW BEDFORD, MASSACHUSETTS



PREPARED FOR:

MIH1, LLC 401 COUNTY STREET NEW BEDFORD, MA 02740

•			

TABLE OF CONTENTS

- 1. STORMWATER MANAGEMENT REPORT
- 2. EXHIBIT "A" HYDROLOGIC CALCULATIONS & WATERSHED PLANS
- 3. EXHIBIT "B" NRCS SOIL MAP



STORMWATER MANAGEMENT REPORT AND HYDROLOGIC ANALYSIS

Proposed Site Clearing
100 Duchaine Blvd., New Bedford, Massachusetts

Project Summary

The subject property encompasses approximately 8.09 acres of forested land located on the westerly side of Duchaine Boulevard in New Bedford, Massachusetts. The site is currently undeveloped. It is bounded on the west by railroad tracks and on the east by commercial property. The topography has very minimal sloping, with the highest elevation 81 to an elevation of 77 near the rear of the lot.

The applicant is seeking permission to clear cut the parcel for temporary stockpile of material. Stormwater associated with the development will be controlled by a detention basin.

Methodology

Drainage computations were performed using the Natural Resources Conservation Services (NRCS) TR-20 method and HydroCAD[®] Drainage Calculation Software. Sketches of the existing and proposed watershed areas, HydroCAD[®] Report, and copies of the calculation sheets are included as appendices to this report.

Existing Conditions

The soils underlying the site are identified in the Soil Survey of Bristol County (see Appendix G). The Soils underlying the site consist of Scarboro mucky fine sandy loam, 0 to 3 percent slopes and Sudbury fine sandy loam, 0 to 3 percent slopes. Soil information was taken from United States Department of Agriculture, Natural Resources Conservation Service, Web Soil Survey of Bristol County, Massachusetts.

The Scarboro soil has the following properties:

- <u>Permeability:</u> Very Poorly drained
- Available water capacity: Low
- Depth to groundwater: 0-2 inches
- Hydrologic Soil Group: HSG-D

The Scarboro soil has the following properties:

- <u>Permeability:</u> Moderately well drained
- Available water capacity: Low

Depth to groundwater: 18-36 inches

Hydrologic Soil Group: HSG-B

Stormwater Management Overview

Existing Conditions:

Currently, the project area consists of a wooded undeveloped area in which stormwater sheet flows in the bordering vegetated wetland area.

Proposed Conditions:

Under proposed conditions, the stormwater will sheet flow to a detention basin. This basin will handle all storms up to a 100 year storm event. During a 100 year storm event stormwater will overflow the two broad crested weirs.

Stormwater Management Standards

Standard #1: Untreated Discharges

Under proposed conditions, there will be no new untreated discharges or erosion in wetland areas.

To improve existing conditions, we have proposed a rip rap pad at the pipe outlet to help control velocity and future erosion at the outlet. All stormwater discharges have been held below erodible velocities.

Standard #2: Peak Rate Control

The design of the stormwater system was designed for the post-development conditions to handle all storms' peak discharges and runoff volume to include the 2, 10 and 100-year storm events. The site drainage system was designed in consideration of the structural standards and techniques of the Best Management Practices (BMP) and Low Impact Development (LID) outlined in the "Stormwater Management Handbook".

The results of site drainage calculations are presented in the following Tables. The results are based upon evaluation of Pre-development conditions and the design of proposed surface and subsurface drainage systems for the Post-development condition. These results show the Post-Development offsite volume and runoff rates are reduced to less than the Pre-development conditions, thus meeting the BMP guidelines for this site development.

Tab	le 1 - Compari	son of	
Pre- versus Post-Do	evelopment Of	ffsite Runoff F	late, cfs
Frequency Storm	2-Year	10-Year	100-Year
Pre-Development	0.46	2.11	5.92
Post-Development	0.28	0.91	5.79

	ole 2 - Compari							
Pre- versus Post-Development Offsite Runoff Volume, af								
Frequency Storm	2-Year	10-Year	100-Year					
Pre-Development	0.080	0.219	0.517					
Post-Development	0.030	0.074	0.410					

Groundwater recharge is a factor in the design of the subsurface drainage system. Table-3 below presents the minimum recharge required and the proposed recharge of stormwater based upon the BMP methods of the "Stormwater Management Handbook". The proposed recharge quantities meet or exceed the required minimum recharges.

Standard #3: Recharge to Groundwater

Recharge facilities are proposed and sized according to the Stormwater Management Standards.

Table 3 - Drainage R (Required Recharge = 0.35" Tot	echarge Calculation tal Site Runoff from Impervious							
Areas for Cl	Areas for Class-B Soils)							
Required Recharge	Proposed Recharge							
0 sf x 0.35"	10.946.CE							
= 0 CF	10,846 CF							

Standard #4: Total Suspended Solids Removal/ Water Quality

In accordance with the guidelines of the Stormwater Management Policy, the Total Suspended Solids (TSS) Removal was calculated to be 80% thus meeting the minimum 80% requirement.

Standard #5: Higher Potential Pollutant Loads

The uses associated with this project are not classified to be land uses with higher potential pollutant loads; therefore, Standard #5 is not applicable to this project.

Standard #6: Critical Areas

This project is not located in a critical area.

Standard #7: Re-development Standards

This project is considered new construction.

Standard #8: Construction Period Pollution Prevention Plan

We will prepare a Construction Period Pollution Prevention Plan in accordance with the regulations prior to start of construction.

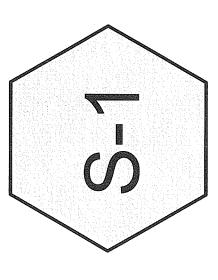
Standard #9: Operation and Maintenance

A long-term operation and maintenance plan will be prepared to ensure that stormwater management systems function as designed.

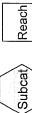
Standard #10: Illicit Discharge Statement

Under this proposal, we are not proposing any illicit discharges as defined in the Stormwater Management Regulations.

HYDROLOGIC CALCULATIONS & WATERSHED PLANS



Tributary to BVW







Drainage Diagram for 16357PRE
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Page 1

Summary for Subcatchment S-1: Tributary to BVW

Runoff = 0.46 cfs @ 12.34 hrs, Volume=

0.080 af, Depth= 0.35"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"

_	A	rea (sf)	CN I	Description		
	4	4,500		Gravel road	•	
_		17,300			od, HSG B	
	1	21,800	56 \	Neighted A	verage	
	121,800		800 Pervious Area		ea	
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
	0.2	8	0.0100	0.65		Sheet Flow, Sheet Flow
	10.0	315	0.0110	0.52		Smooth surfaces n= 0.011 P2= 3.40" Shallow Concentrated Flow, Shallow Concentrated Flow Woodland Kv= 5.0 fps
_	10.2	323	Total			

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Page 2

Summary for Subcatchment S-1: Tributary to BVW

Runoff = 2.11 cfs @ 12.17 hrs, Volume=

0.219 af, Depth= 0.94"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.80"

	Α	rea (sf)	CN	Description				· .	
	1	4,500 17,300		Gravel road Woods, Go	•				
-		21,800 21,800		Weighted A Pervious Ar	-				
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
_	0.2	8	0.0100	0.65		Sheet Flow, Sheet Flow	_		
	10.0	315	0.0110	0.52		Smooth surfaces n= 0.011 P2= 3.40" Shallow Concentrated Flow, Shallow Concentrated Flowodland Kv= 5.0 fps			centrated Flow
_	10.2	323	Total						

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Page 3

Summary for Subcatchment S-1: Tributary to BVW

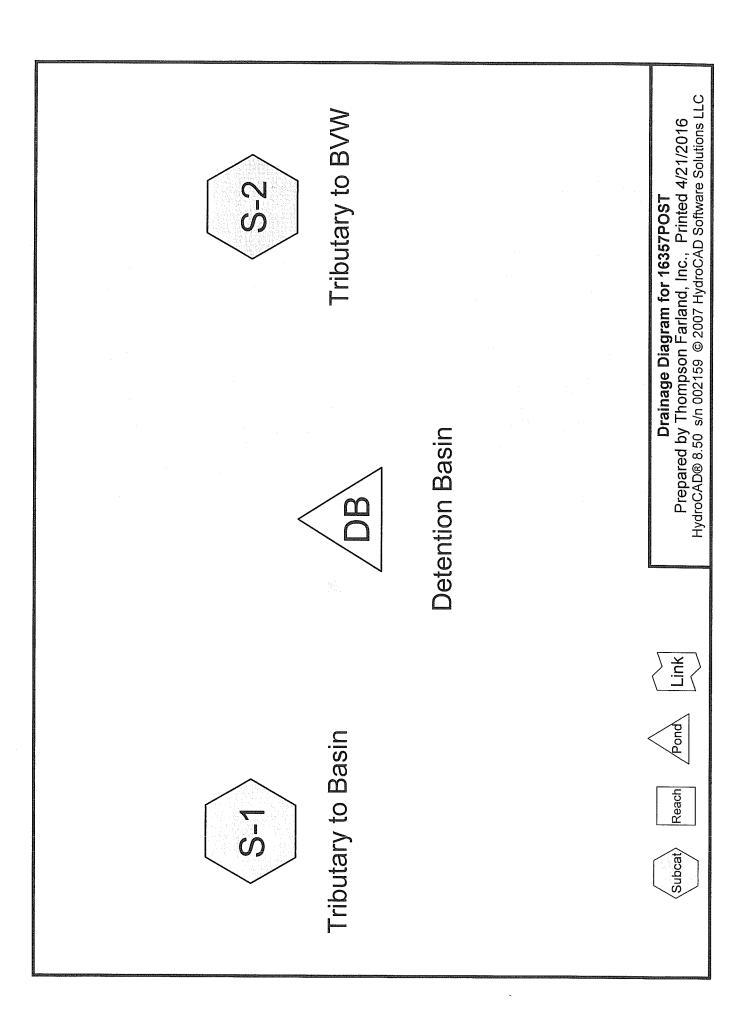
Runoff

5.92 cfs @ 12.15 hrs, Volume=

0.517 af, Depth= 2.22"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"

	ΑΑ	rea (sf)	CN [Description		
		4,500	85 (Gravel roac	ls, HSG B	
_	1	17,300	55 \	<u> Voods, Go</u>	od, HSG B	
	121,800 121,800		,			
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	0.2	8	0.0100	0.65		Sheet Flow, Sheet Flow Smooth surfaces n= 0.011 P2= 3.40"
	10.0	315	0.0110	0.52		Shallow Concentrated Flow, Shallow Concentrated Flow Woodland Kv= 5.0 fps
	10.2	323	Total			



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Page 1

Summary for Subcatchment S-1: Tributary to Basin

Runoff 4.18 cfs @ 12.10 hrs, Volume= 0.304 af, Depth= 1.77"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"

	Α	rea (sf)	CN I	Description			
		3,650	85 (Gravel road	ls, HSG B		
*		15,798	98 [Detention F	Pond		
		63,900	82 I	Dirt roads,	HSG B		
		6,052	55 \	Noods, Go	od, HSG B		
		89,400	83 \	Neighted A	verage		
		73,602	Pervious Area				
		15,798	Impervious Area				
				•		ϵ	en e
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	0.2	8	0.0100	0.65		Sheet Flow, Sheet Flow	
						Smooth surfaces n= 0.011 P2= 3.	40"
	6.4	200	0.0110	0.52		Shallow Concentrated Flow, Shall	
						Woodland Kv= 5.0 fps	
	6.6	208	Total				

Summary for Subcatchment S-2: Tributary to BVW

0.28 cfs @ 12.12 hrs, Volume= Runoff

0.030 af. Depth= 0.49"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"

Area (sf)	CN	Description			
850	85	Gravel road	ls, HSG B		
5,100	82	Dirt roads, l	HSG B		
26,450	55	Woods, Go	od, HSG B		
32,400	60	Weighted A	verage		
32,400		Pervious Ar	rea		
Tc Length (min) (feet)	Slop (ft/	•	Capacity (cfs)	Description	
6.0				Direct Entry, Minimum	

Summary for Pond DB: Detention Basin

Inflow Area =	2.052 ac, 17.67% Impervious, Inflow D	Depth = 1.77" for 2-yr event
Inflow =	4.18 cfs @ 12.10 hrs, Volume=	0.304 af
Outflow =	0.29 cfs @ 13.94 hrs, Volume=	0.304 af, Atten= 93%, Lag= 110.3 min
Discarded =	0.29 cfs @ 13.94 hrs, Volume=	0.304 af
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

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Page 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 76.51'@ 13.94 hrs Surf.Area= 12,322 sf Storage= 5,831 cf

Plug-Flow detention time=(not calculated: outflow precedes inflow) Center-of-Mass det. time=198.0 min (1,028.7 - 830.7)

Volume	Invert	Avail.Sto	rage Storage D	Description		
#1	76.00'	19,74	45 cf Custom	Stage Data (Prisma	atic)_isted below (Recalc)	
Elevation (feet)		f.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
76.00		10,538	0	0		
77.00	1	14,035	12,287	12,287		
77.50	•	15,798	7,458	19,745		
Device F	Routing	Invert	Outlet Devices			
#1 [Discarded	76.00'	1.020 in/hr Ex	filtration over Surf	ace area	
#2 F	Primary	77.00'	Head (feet) 0.	20 0.40 0.60 0.80	-Crested Rectangular Weir X 2.0 1.00 1.20 1.40 1.60 .69 2.68 2.69 2.67 2.64	00

Discarded OutFlow Max=0.29 cfs @ 13.94 hrs HW=76.51' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.29 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=76.00' (Free Discharge)

2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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Page 3

Summary for Subcatchment S-1: Tributary to Basin

Runoff = 7.03 cfs @ 12.09 hrs, Volume=

0.512 af, Depth= 2.99"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.80"

_	Α	rea (sf)	CN	Description		
		3,650	85	Gravel road	ds, HSG B	
*		15,798	98	Detention F	Pond	
		63,900	82	Dirt roads,	HSG B	
_		6,052	55	Woods, Go	od, HSG B	
		89,400	83	Weighted A	verage	
		73,602		Pervious A	rea	
		15,798		Impervious	Area	
	Тс	Length	Slope		Capacity	Description
	(min)	(feet)	(ft/ft)) (ft/sec)	(cfs)	
	0.2	8	0.0100	0.65		Sheet Flow, Sheet Flow
						Smooth surfaces n= 0.011 P2= 3.40"
	6.4	200	0.0110	0.52		Shallow Concentrated Flow, Shallow Concentrated Flow
_						Woodland Kv= 5.0 fps
	6.6	208	Total	-		

Summary for Subcatchment S-2: Tributary to BVW

Runoff = 0.91 cfs @ 12.10 hrs, Volume=

0.074 af, Depth= 1.19"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.80"

Area (sf)	CN	Description		
850	85	Gravel road	s, HSG B	
5,100	82	Dirt roads, I	HSG B	
26,450	55	Woods, Go	od, HSG B	
32,400	32,400 60 Weighted Average			
32,400		Pervious Ar	ea	
Tc Length (min) (feet)	Slop (ft/	,	Capacity (cfs)	Description
6.0				Direct Entry, Minimum

Summary for Pond DB: Detention Basin

Inflow Area =	2.052 ac, 17.67% Impervious, Inflow Depth = 2.99" for 10-yr event	
Inflow =	7.03 cfs @ 12.09 hrs, Volume= 0.512 af	
Outflow =	0.33 cfs @ 15.07 hrs, Volume= 0.503 af, Atten= 95%, Lag= 178.6 min	
Discarded =	0.33 cfs @ 15.07 hrs, Volume= 0.503 af	
Primary =	0.00 cfs @ 0.00 hrs, Volume= 0.000 af	

Page 4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 76.96' @ 15.07 hrs Surf.Area= 13,895 sf Storage= 11,727 cf

Plug-Flow detention time=(not calculated: outflow precedes inflow) Center-of-Mass det. time=356.9 min (1,172.5 - 815.7)

Volume	Invert	Avail.Sto	rage Storage D	Description			_
#1	76.00	19,7	45 cf Custom	Stage Data (Pı	rismatic)_isted	below (Recalc)	
Elevatio		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
76.0	00	10,538	0	0			
77.0	00	14,035	12,287	12,287			
77.5	50	15,798	7,458	19,745			
Device	Routing	Invert	Outlet Devices				
#1	Discarded	76.00'	1.020 in/hr Ex	filtration over	Surface area		
#2	Primary	77.00'	10.0' long x 1	0.0' breadth B	Broad-Crested	Rectangular Weir X 2.00	0
	,		Head (feet) 0.	20 0.40 0.60	0.80 1.00 1.20	0 1.40 1.60	
			Coef. (English)	2.49 2.56 2.	70 2.69 2.68	2.69 2.67 2.64	

Discarded OutFlow Max=0.33 cfs @ 15.07 hrs HW=76.96' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.33 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=76.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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Page 5

Summary for Subcatchment S-1: Tributary to Basin

Runoff 11.62 cfs @ 12.09 hrs, Volume= 0.860 af, Depth= 5.03"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"

_	Α	rea (sf)	CN	Description				
		3,650		Gravel road				
*		15,798	98	Detention F	Pond			
		63,900	82	Dirt roads,	HSG B			
_		6,052	55	Woods, Go	od, HSG B			
		89,400	83	Weighted A	verage			
		73,602		Pervious A	rea			
		15,798		Impervious	Area			
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description		
	0.2	8	0.0100	0.65		Sheet Flow, Sheet Flow		****
*****	6.4	200	0.0110	0.52		Smooth surfaces n= 0.011 P2= 3.40" Shallow Concentrated Flow, Shallow Co Woodland Kv= 5.0 fps	oncentrate	ed Flow
	6.6	208	Total					

Summary for Subcatchment S-2: Tributary to BVW

Runoff 2.20 cfs @ 12.09 hrs, Volume=

0.161 af, Depth= 2.60"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"

	Α	rea (sf)	CN	Description			
		850	85	Gravel road	ls, HSG B		-
		5,100	82	Dirt roads, I	HSG B		
		26,450	55	Woods, Go	od, HSG B		
		32,400	60	Weighted A	verage		_
		32,400		Pervious Ar	ea		
	Tc (min)	Length (feet)	Slope (ft/ft)	•	Capacity (cfs)	Description	
•	6.0		, ,	,	, ,	Direct Entry Minimum	-

Direct Entry, Minimum

Summary for Pond DB: Detention Basin

Inflow Area =	2.052 ac, 17.67% Impervious, Inflow I	Depth = 5.03" for 100-yr event
Inflow =	11.62 cfs @ 12.09 hrs, Volume=	0.860 af
Outflow =	3.93 cfs @ 12.39 hrs, Volume=	0.803 af, Atten= 66%, Lag= 17.9 min
Discarded =	0.35 cfs @ 12.39 hrs, Volume=	0.553 af
Primary =	3.59 cfs @ 12.39 hrs, Volume=	0.249 af

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Page 6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 77.17' @ 12.39 hrs Surf.Area= 14,646 sf Storage= 14,770 cf

Plug-Flow detention time=(not calculated: outflow precedes inflow) Center-of-Mass det. time=230.1 min (1,031.2 - 801.0)

Volume	Invert	Avail.Sto	orage Storage D	Description	
#1	76.00	19,7	45 cf Custom	Stage Data (Pri	ismatic)_isted below (Recalc)
Elevatio		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
76.0	00	10,538	0	0	
77.0	00	14,035	12,287	12,287	
77.5	50	15,798	7,458	19,745	
Device	Routing	Invert	Outlet Devices		
#1	Discarded	76.00'	1.020 in/hr Ex	filtration over S	Surface area
#2 Primary 77.00' 10.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64					

Primary OutFlow Max=3.59 cfs @ 12.39 hrs HW=77.17' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 3.59 cfs @ 1.04 fps)

NRCS SOIL MAP



MAP LEGEND

Area of Ini	Area of Interest (AOI)	S	Spoil Area
	Area of Interest (AOI)		Stony Spot
Soils		-17	Very Stony Spot
	Soil Map Unit Polygons		Mot Spot
4	Soil Map Unit Lines	> }>	vet oput
	ctuind timin a con iso	0	Other
1	Soil Map Office Politics		Special Line Features
Special	Special Point Features		
*O#	Blowout	Water Features	Se
)	Security (Co.)		Streams and Canals
Z	Borrow Pit		
Ì		Transportation	=
×	Clay Spot		Rails
0	Closed Depression		Interstate Highways

tation	Rails	Interstate Highways	US Routes
Transportation	*	No. of the Control of	S123772
		ession	

Background

Major Roads Local Roads

Gravelly Spot

Gravel Pit

Aerial Photography

Marsh or swamp

* Ø 0 0

Lava Flow

Landfill

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

Saline Spot Sandy Spot

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting Enlargement of maps beyond the scale of mapping can cause Warning: Soil Map may not be valid at this scale.

Please rely on the bar scale on each map sheet for map measurements.

soils that could have been shown at a more detailed scale.

Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov Source of Map: Natural Resources Conservation Service Coordinate System: Web Mercator (EPSG:3857)

distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Bristol County, Massachusetts, Southern Part Version 9, Sep 28, 2015 Survey Area Data: Soil Survey Area:

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Mar 30, 2011—Oct 8,

imagery displayed on these maps. As a result, some minor shifting The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background of map unit boundaries may be evident.

Severely Eroded Spot

Slide or Slip Sodic Spot

Sinkhole

Chile

USDA

Map Unit Legend

Bristol County, Massachusetts, Southern Part (MA603)							
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI				
38A	Pipestone loamy sand, 0 to 3 percent slopes	0.0	0.0%				
39A	Scarboro mucky fine sandy loam, 0 to 3 percent slopes	6.2	58.1%				
260A	Sudbury fine sandy loam, 0 to 3 percent slopes	2.2	20.9%				
602	Urban land	2.2	20.9%				
Totals for Area of Interest		10.6	100.0%				