



**Nitsch Engineering**

April 6, 2022

**STORMWATER  
REPORT**

For

**EAST BEACH PARKING LOT  
GREEN INFRASTRUCTURE RERTOFIT**  
New Bedford, Massachusetts

Prepared for:

**CITY OF NEW BEDFORD  
PARKS RECREATION & BEACHES**  
181 Hillman Street, Building 3  
New Bedford, MA 02740

Prepared by:

**NITSCH ENGINEERING, INC.**  
2 Center Plaza, Suite 430  
Boston, MA 02108

NITSCH PROJECT #14850



Nitsch Project #14850

**TABLE OF CONTENTS**

---

<b>1.0 INTRODUCTION .....</b>	<b>1</b>
<b>2.0 EXISTING CONDITIONS.....</b>	<b>2</b>
2.1 NRSC Soil Designations .....	3
2.2 On-Site Soil Investigations .....	3
2.3 Wetland Resource Areas .....	4
2.4 Total Maximum Daily Load (TMDL).....	5
<b>3.0 NEW BEDFORD STORMWATER MANAGEMENT STANDARDS .....</b>	<b>5</b>
<b>4.0 PROPOSED CONDITIONS .....</b>	<b>6</b>
4.1 Project Description.....	6
4.2 Proposed Green Infrastructure Improvements .....	7
4.3 Stormwater Management During Construction.....	9
<b>5.0 STORMWATER MANAGEMENT ANALYSIS .....</b>	<b>10</b>
5.1 Methodology .....	10
5.2 HydroCAD Version 10.00.....	10
<b>6.0 WATER QUALITY VOLUME CALCULATIONS .....</b>	<b>10</b>
<b>7.0 MASSDEP STORMWATER MANAGEMENT STANDARDS .....</b>	<b>11</b>
Standard 1: No New Untreated Discharges.....	11
Standard 2: Peak Rate Attenuation .....	12
Standard 3: Groundwater Recharge.....	12
Standard 4: Water Quality Treatment.....	12
Standard 5: Land Uses with Higher Potential Pollutant Loads (LUHPPL).....	13
Standard 6: Critical Areas .....	13
Standard 7: Redevelopments.....	13

Standard 8: Construction Period Pollution Prevention and Sedimentation Control ..... 13

Standard 9: Operation and Maintenance Plan..... 14

Standard 10: Prohibition of Illicit Discharges ..... 14

**8.0 CONCLUSION..... 14**

**FIGURES**

- DR-1A Existing Watershed Areas (30 Scale)
- DR-1B Existing Watershed Areas (100 Scale)
- DR-2A Proposed Watershed Areas (30 Scale)
- DR-2B Proposed Watershed Areas (100 Scale)

**APPENDICES**

**Appendix A Stormwater Management Standards Documentation**

- MassDEP Checklist for Stormwater Report
- Standard 4: TSS Removal Calculations
- Phosphorous Removal Calculations
- Standard 4: Isolator Row Sizing Calculations
- Standard 10: Illicit Discharge Compliance Statement

**Appendix B HydroCAD Calculations – Water Quality Storm (1.7-inch)**

**Appendix C HydroCAD – Existing Peak Runoff Rates**

**Appendix D HydroCAD – Proposed Peak Runoff Rates**

**Appendix E Long-Term Pollution Prevention and Stormwater Operation and Maintenance Plan**

**Appendix F Soil Investigations**

- NRCS Soil Maps and Descriptions
- Test Pit Information

## 1.0 INTRODUCTION

Nitsch Engineering has prepared this Stormwater Report to support the Notice of Intent (NOI) to the New Bedford Conservation Commission for the proposed East Beach Parking Lot Green Infrastructure Retrofit project within the East Beach parking lots, which are located within jurisdictional wetland resource areas (subsequently referred to as the “Project”).

The Site consists of three (3) separate parking lots located along Rodney French Boulevard East. The improvements include reconstructing and greening the existing parking lot and providing water quality treatment through bioretention basins and subsurface infiltration systems.

The Project limit of work includes the following parcels:

- Parking Lot A: Map 4, Lot 100
- Parking Lot B: Map 4, Lot 101
- Parking Lot C: Map 4, Lot 102



Figure 1. Site Locus

The proposed stormwater improvements have been designed to comply with the requirements of the City of New Bedford Stormwater Management Rules & Regulations (New Bedford Stormwater Regulations) and the Massachusetts Department of Environmental Protection (DEP) Stormwater Management Standards. The Project has also been designed to meet the City of New Bedford Green Infrastructure Plan water quality volume goal, which exceeds the requirements put forth in the New Bedford Stormwater Regulations, per the direction of the Department of Public Infrastructure (DPI).

## **2.0 EXISTING CONDITIONS**

---

The Project site consists of three (3) existing parking lots along Rodney French Boulevard East. The existing parking lots are entirely paved and are in poor surface condition. (Figure 1 – USGS Locus Map and Figure 2 – Aerial Locus Map).

- Parking Lot A:
  - 0.74 Acres
  - Bound by Rodney French Boulevard South to the south, Rodney French Boulevard East to the east, and Seymour Street to the north.
  - Flows to existing catch basins within Rodney French Boulevard South and Seymour Street.
- Parking Lot B:
  - 0.95 Acres
  - Bound by Seymour Street to the south, Rodney French Boulevard East to the east, and Hudson Street to the north.
  - Flows to existing catch basins within Seymour Street and Hudson Street.
- Parking Lot C:
  - 1.05 Acres
  - Bound by Hudson Street to the south, Rodney French Boulevard East to the east, and Portland Street to the north.
  - Flows to existing catch basin within Hudson Street and Portland Street.

There is no existing stormwater collection or treatment systems within the parking lots. There is an existing drain pipe and sewer pipe that run through Parking Lot A; however the parking lot does not appear to discharge to either pipe. These existing pipes will be maintained as part of the Project.

Stormwater runoff in Seymour Street, Rodney French Boulevard South, and the adjacent portion of East Rodney French Boulevard East is collected by a series of catch basins that flow west in East Rodney French Boulevard and connect to an existing 14-inch drain main which flows and outlets to the harbor via an outfall.

Stormwater runoff from Hudson Street, Portland Street, and the northern portion of East Rodney French Boulevard is collected by a closed drainage system which flows north and connects to a 15-inch storm drain main which outfalls to the harbor.



Figure 2: Existing Drainage Infrastructure

## 2.1 NRCS Soil Designations

The Soil Classification Summary (Table 1) outlines the Natural Resources Conservation Services (NRCS) designation of the soil series at the Site. The Site is located within urban land, which does not have a hydrologic soil rating (refer to the NRCS Soil Maps and Descriptions in Appendix E).

Table 1. NRCS Soil Classification Summary

Soil Unit	Soil Series	Hydrologic Soil Group
602	Urban Land	-

## 2.2 On-Site Soil Investigations

Nitsch Engineering, Inc. performed nine (9) test pits at the Site on 2/24/2022 and 3/3/2022.

Three (3) test pits were performed within each parking lot. The eastern test pits, along Rodney French Boulevard East, encountered evidence of groundwater at depths ranging from 58-inches to 83-inches below grade. The western test pits on the uphill side of the parking lots encountered groundwater

depths ranging from 78-inches to 90-inches. Based on the test pits, Nitsch Engineering determined that infiltration is feasible at the up-hill portions of the parking lots (western), where a minimum of 2-ft groundwater separation can be achieved.

Based on the sandy loam conditions encountered at the approximately bottom of BMP elevations, the Project assumed a Rawls Rate of 1.02 inches/hour, associated with the texture class Sandy Loam.

Table 2. Test Pit Summary

Test Pit #	Soil Type	Groundwater Elevation
1 [Lot A]	Sandy Loam	58"
2 [Lot A]	Sandy Loam	72"
3 [Lot B]	Sandy Loam	64"
4 [Lot B]	Sandy Loam	64"
5 [Lot C]	Sandy Loam	56"
6 [Lot C]	Sandy Loam	83"
7 [Lot A]	Sandy Loam	90"
8 [Lot B]	Sandy Loam	85"
9 [Lot C]	Sandy Loam	78"



Figure 3: Test Pit Locations

### 2.3 Wetland Resource Areas

The Project site is bordered by residential parcels and public roadways and is located within Land Subject to Coastal Storm Flowage (LSCSF). The Project site is also located within the 100-foot Buffer

Zone associated with the nearby coastal resource areas (i.e. the coastal beach). Refer to the Notice of Intent for additional information regarding the resource areas.

## 2.4 Total Maximum Daily Load (TMDL)

The sub-watershed has been identified as impaired/threatened per the Outer New Bedford Harbor, Category 5 (Impaired or threatened for one or more uses and requiring a TMDL). See Table 3 for a list of impairments.

**Table 3. List of Impairments for Outer New Bedford Harbor**

<b>Water Body</b>	<b>Classification</b>	<b>Impairment(s)</b>
<b>Outer New Bedford Harbor  (MA95-63_2008)</b>	Category 5: Impaired or threatened for one or more uses and requiring a TMDL.	Metals Nonpriority organics Nutrients Organic enrichment/Low DO Pathogens Priority organics

The Site does not have stormwater treatment prior to discharge in the existing conditions. The proposed green infrastructure improvements are designed specifically to improve stormwater quality. The Project has been designed to reduce stormwater discharge and associated pathogen pollutants through filtration and infiltration to meet the intent of the TMDL.

## 3.0 NEW BEDFORD STORMWATER MANAGEMENT STANDARDS

The City of New Bedford requires stormwater management systems on redevelopment sites be designed to meet an average annual pollutant removal equivalent to:

- 80% of the average annual postconstruction load of Total Suspended Solids (TSS) related to the total postconstruction impervious area on the site, and
- 50% of the average annual load of Total Phosphorus (TP) related to the total post-construction impervious surface area on the site.

This treatment shall be achieved by retaining the volume of runoff equivalent to, or greater than, 0.8 inch multiplied by the total post-construction impervious surface area on the redeveloped site or meeting a combination of retention and treatment that achieves the above standards.

### City of New Bedford Green Infrastructure Plan:

Nitsch Engineering understands that the City is in the process of developing and adopting a new city-wide green infrastructure strategy (the Green Infrastructure Master Strategy and Implementation Roadmap). Per coordination with the DPI, DPI indicated that the Project should align with the goal of this new plan, which will have a water quality treatment goal of 1.7-inches over the impervious area. This goal significantly exceeds state and local stormwater standards.



## 4.0 PROPOSED CONDITIONS

### 4.1 Project Description

The Project consists of the construction of new Best Management Practices (BMPs) that have been designed in accordance with the MassDEP Stormwater Management Standards and the City of New Bedford Stormwater Management Regulations. The systems are designed to provide water quality treatment prior to discharge to the existing drainage system, improving the water quality discharging to the Outer New Bedford Harbor. A summary of these improvements is provided below.

The Project will result in a net reduction in impervious area of 0.29 acres of impervious area (Table 4).

**Table 4. Existing and proposed land cover**

Land Use	Existing (acres)	Proposed (acres)	Change
<b>Impervious Area</b>	2.74	2.45	- 0.29
<b>Landscape/Gravel Area</b>	- -	0.29	+ 0.29
<b>Total</b>	2.74	2.74	- -

The Project includes the construction of bioretention basins designed to treat contributing impervious areas within the parking lot and the upstream residential parcels which flow overland into the parking lots. Where the bioretention basins cannot be sized to meet the full 1.7-inch water quality volume goal from the City of New Bedford Green Infrastructure Plan, the remaining runoff from the water quality volume will be directed to subgrade isolator rows to provide the remaining treatment prior to discharging to the City storm drain mains.

In addition, the Project will divert runoff from the adjacent Seymour Street, Hudson Street and Portland Street and direct runoff into the subsurface treatment and infiltration system located below the parking lots. The infiltration systems are sized to infiltrate the 1.7-inch water quality storm and bypass/overflow the larger storms.

The off-site contributing drainage areas are summarized below. See Figure DA-1 for a summary of the off-site contributing drainage areas.

**Table 5. Contributing Watershed Areas (acres)**

Land Use	Area (acres)
<b>Roadways (Hudson, Seymour, Portland St)</b>	0.86
<b>Residential Parcels (38% Impervious) <sup>A</sup></b>	4.94
<b>Total Contributing Watershed Area</b>	5.80 <sup>B</sup>

A) The % imperviousness of the residential portions is calculated based on the Curve Number associated with 1/4 acre residential lots - see HydroCAD methodology for more information on the Curve Number calculations.

B) Does not include the Parking Lots

Overflow from the new stormwater management systems will discharge to the new closed drainage systems within the parking lots, which will connect to the existing city storm drain mains in Rodney French Boulevard East, which are described in Section 2.0.

#### 4.2 Proposed Green Infrastructure Improvements

The Project includes the installation of green infrastructure stormwater improvements that have been designed to meet the MassDEP Stormwater Management Standards and City of New Bedford Stormwater Management Rules & Regulations. The Project is focused on improving the existing stormwater system by implementing environmentally-sensitive site design and LID techniques. This design prevents the generation of stormwater and non-point source pollution by reducing impervious surfaces with landscaping and permeable materials, disconnecting flow paths, treating and infiltrating stormwater at its source, and protecting natural processes. Stormwater systems have been designed to model natural hydrologic features, including promoting infiltration throughout the sites.

The proposed stormwater management system for the Project includes bioretention basins, isolator rows and subsurface infiltration systems. These system components are described below. Overflow from the proposed improvements will reconnect to the existing stormwater system and associated outfalls.

<p style="text-align: center;"><b><u>Treatment Train A</u></b> <b><u>Infiltrating Bioretention Basin</u></b> <b>[BB#3A, BB#3B, BB#6B]</b> Parking Lot Runoff + Contributing Residential Parcels → Sediment Forebay → Bioretention Basin</p> <p style="text-align: center;"><b><u>Treatment Train B</u></b> <b><u>Lined Biofiltration Basin</u></b> <b>[BB#2A, BB#4A, BB#4B, BB#5A, BB#6A, BB#7A, BB#7B, BB#8A, BB#8B]</b> Parking Lot Runoff + Contributing Residential Parcels → Biofiltration Basin</p> <p style="text-align: center;"><b><u>Treatment Train C</u></b> <b><u>Bioretention Basin, overflow to Isolator Rows</u></b> <b>[BB#1, BB#2B, BB#5B, BB#9]</b> Parking Lot Runoff + Contributing Residential Parcels → Bioretention Basin → Overflow to Isolator Row</p> <p style="text-align: center;"><b><u>Treatment Train D</u></b> <b><u>Upstream Watershed to Infiltration System</u></b> <b>[CB1, CB3, CB4, CB5]</b> Roadway and Residential Runoff → Isolator Row → Infiltration System</p>
---

#### Deep Sump and Hooded Catch Basins

Deep sump and hooded catch basins are proposed to provide pretreatment of the runoff from the roadways prior to discharging to the infiltration systems. Stormwater captured in the catch basins will be directed to additional pretreatment and then to the subsurface infiltration systems prior to discharge.

#### Subsurface Infiltration/Detention Systems

Stormwater runoff from the adjacent roadways will be collected and infiltrated using a subsurface infiltration system in each parking lot. The upstream diversion manhole will direct stormwater runoff from approximately the 1.7-inch storm event to the subsurface infiltration systems and bypass the larger storm events to the maximum extent practicable. The Isolator Rows will treat for phosphorous

and TSS. The systems are designed to, at minimum, infiltrate the 1.7-inch storm event water quality volume, and because of the added storage volume will reduce peak runoff rates in the larger storms. The larger storm events will overflow to the new closed drainage systems within the parking lots.

Roadway runoff that is tributary to these systems will be pretreated using deep sump and hooded catch basins and isolator rows to meet the 44% TSS removal prior to infiltration.

Subsurface Infiltration System #1 is proposed to collect runoff from the southern portion of Seymour Street. The system consists of StormTech SC-310 chambers enveloped by crushed stone.

Subsurface Infiltration System #2 is proposed to collect runoff from Hudson Street. The system consists of StormTech SC-370 chambers enveloped by crushed stone.

Subsurface Infiltration System #3 is proposed to collect runoff from the southern portion of Portland Street. The system consists of StormTech SC-310 chambers enveloped by crushed stone.

#### Infiltrating Bioretention Basins with Sediment Forebay

Bioretention basins are proposed to treat stormwater runoff from the parking lots and the residential areas from which runoff overland flows directly to the parking lots. Where groundwater separation can be achieved, the bioretention basins will be infiltrating (Bioretention Basins: BB#3A, BB#3B, BB#6B). Stormwater runoff will be treated as it flows through the bioretention soil media and the filter course layers, before reaching the crushed stone reservoir which will promote storage and infiltration into the subgrade.

The bioretention basins are sized to store and infiltrate the stormwater volume of 1.7-inches over the contributing impervious area. Overflow from larger storm events will be collected via overflow area drains within the basins that discharge to the new closed drainage system within the parking lots.

Pretreatment for the bioretention basin will be provided sediment forebays. The sediment forebays are designed in accordance with the MassDEP Stormwater Management Handbook to provide a water quality volume (WQV) equivalent to 0.1 inches per impervious acre.

#### Lined Bioretention Basins

Where groundwater separation **cannot** be achieved, the bioretention basins will be designed to treat stormwater runoff as it flows through the bioretention soil media and the filter course layers, before reaching the underdrain within the crushed stone reservoir which will connect to the new closed drainage system within the parking lots.

The bioretention basins are sized to store and filter the stormwater volume of 1.7-inches over the contributing impervious area through the bioretention basin section. Overflow from larger storm events will be collected via overflow area drains within the basins that discharge to the new closed drainage system within the parking lots.

Isolator Rows

The Isolator Rows consist of plastic chambers, wrapped in a geotextile fabric within a crushed stone reservoir. Runoff slowly passes from the chambers through a woven geotextile fabric and into the crushed stone reservoir below the system. The runoff passes through the fabric, leaving behind sediments and associated contaminants through the physical unit operations of filtration and sedimentation. As an organic filter cake develops over the fabric, phosphorus is also removed via the chemical process or sorption.

*Subsurface Infiltration System Pretreatment:*

Isolator Rows are proposed to provide pretreatment of the stormwater runoff from roadway areas prior to discharging to the adjacent chambers within the subsurface infiltration systems. The use of Isolator Rows is preferred over other proprietary structures because they provide a higher level of pollutant removal based on research at the University of New Hampshire Stormwater Center.

*Non-Infiltrative Water Quality Treatment*

Overflow from the 1.7-inch water quality volume storm within Bioretention Basins #1, #2B, #5B and #9 will be directed to a subsurface isolator row system wrapped in an impermeable liner with an underdrain. These systems are sized to treat the remainder of the water quality flow rate that could not be managed in these upstream bioretention basins due to space constraints. Stormwater will be treated by the isolator rows and then collected by the underdrains below the chambers, which will discharge to the new closed drainage system within the parking lots.

**4.3 Peak Flow Rates**

The proposed stormwater management system is expected to reduce the proposed peak runoff rates to at or below the existing rates for the follow design points:

- DP-1: Rodney French Boulevard South 14-inch Outfall
- DP-2: Portland Street 15-inch Outfall

Table 6 below summarize the existing and proposed hydrologic analyses for the site at each design point.

**Table 6 – Peak Rates of Runoff in Cubic Feet per Second (cfs)**

	Storm Event	1.7-Inch	2-year	10-year	25-year	100-year
<b>DP-1</b>	Existing	2.23	5.86	9.38	11.57	14.90
	Proposed	0.83	4.53	7.60	9.67	13.97
<b>DP-2</b>	Existing	6.88	18.76	30.51	37.90	49.10
	Proposed	3.50	16.44	27.81	34.44	44.68

#### **4.4 Stormwater Management During Construction**

The Site Contractor will be responsible for stormwater management of the active construction site and is required to provide construction period erosion and sediment controls as provided in the Project plans and specifications.

### **5.0 STORMWATER MANAGEMENT ANALYSIS**

---

#### **5.1 Methodology**

Nitsch Engineering completed a hydrologic analysis of the existing project site utilizing Soil Conservation Service (SCS) Runoff Curve Number (CN) methodology. The SCS method calculates the rate at which the runoff reaches the design point considering several factors: the slope and flow lengths of the subcatchment area, the soil type of the subcatchment area, and the type of surface cover in the subcatchment area. HydroCAD Version 10.00 computer modeling software was used in conjunction with the SCS method to determine the peak runoff rates and runoff volumes for the 2-, 10-, 25-, and 100-year, 24-hour storm events. The proposed project site is being analyzed with the same methodology.

The Site was divided into multiple drainage areas, or subcatchments, which drain to the design points along the property boundary and within the site. For each subcatchment area, SCS Runoff Curve Numbers (CNs) were selected by using the cover type and hydrologic soil group of each area. The peak runoff rates and runoff volumes for the 2-, 10-, 25- and 100-year 24-hour storm events were then determined by inputting the drainage areas, CNs, and time of concentration ( $T_c$ ) paths into the HydroCAD model.

The National Oceanic and Atmospheric Administration Atlas 14 precipitation frequency estimates were used to calculate the 2-, 10-, 25-, and 100- year 24-hour storm events in HydroCAD. Refer to the HydroCAD calculations in Appendix B and C for rainfall information.

#### **5.2 HydroCAD Version 10.00**

The HydroCAD computer program uses SCS and TR-20 methods to model drainage systems. TR-20 (Technical Release 20) was developed by the Soil Conservation Service to estimate runoff and peak discharges in small watersheds. TR-20 is generally accepted by engineers and reviewing authorities as the standard method for estimating runoff and peak discharges.

HydroCAD Version 10.00 uses up to four types of components to analyze the hydrology of a given site: subcatchments, reaches, basins, and links. Subcatchments are areas of land that produce surface runoff. The area, weighted CN, and  $T_c$  characterize each individual subcatchment area. Reaches are generally uniform streams, channels, or pipes that convey water from one point to another. A basin is any impoundment that fills with water from one or more sources and empties via an outlet structure. Links are used to introduce hydrographs into a project from another source or to provide a junction for more than one hydrograph within a project. The time span for the model was set for 0-72 hours in order to prevent truncation of the hydrograph.

### **6.0 WATER QUALITY VOLUME CALCULATIONS**

---

As described, the Project is designed to treat the first 1.7-inches of runoff from the parking lots and the adjacent roadway sections. The Project provides water quality treatment via infiltration of the 1.7-inch water quality volume where separation from groundwater is feasible. Where infiltration is not

feasible, the bioretention basins and the isolator rows are designed to treat the water quality volume via filtration.

The BMPs are designed to treat the 1.7-inch water quality storm to remove on average:

- 80% of the average annual postconstruction load of Total Suspended Solids (TSS) related to the total postconstruction impervious area of the contributing runoff;
- 50% of the average annual load of Total Phosphorus (TP) related to the total post-construction impervious area of the contributing runoff.

**Treatment Train A: Bioretention Basins**

TSS Removal	TP Removal
90% <sup>A</sup>	60% <sup>B</sup>

**Treatment Train B: Lined Bioretention Basin**

TSS Removal	TP Removal
90% <sup>A</sup>	60 % <sup>B</sup>

**Treatment Train C: Bioretention Basin to Isolator Row**

TSS Removal	TP Removal
81% <sup>A</sup>	60% <sup>B</sup>

**Treatment Train D: Subsurface Infiltration System with Isolator Row**

TSS Removal	TP Removal
80% <sup>A</sup>	99% <sup>B</sup>

A) See TSS Removal Worksheet in Appendix A

B) See Phosphorous Removal Charts in Appendix A

See *Appendix B: HydroCAD Calculations – Water Quality Storm (1.7-inches)* for the analysis of the BMPs during the 1.7-inch rainfall event. As shown, each system is designed so that it fully infiltrates or treats the 1.7-inch rainfall event.

Bioretention Basins BB#1, BB#2B, BB#5B, BB#9 cannot feasibly treat the full 1.7-inch water quality volume due to space constraints; therefore, the systems include overflow grates that direct the remainder of the water quality volume to subsurface isolator row systems.

**7.0 MassDEP Stormwater Management Standards**

---

The Project is considered a **redevelopment** under the DEP Stormwater Management Standards. As such, the Project is required to meet Standards 2, 3, and the pretreatment and structural best management practice requirements of Standards 4, 5, and 6 only to the maximum extent practicable. Existing stormwater discharges need to comply with Standard 1 only to the maximum extent practicable. The Project will comply with all other Standards. The Site will be designed to meet or meet to the maximum extent practicable the MassDEP Stormwater Management Standards as summarized below:

**Standard 1: No New Untreated Discharges**

The Project will not discharge any untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth. Stormwater from the Site will be collected and treated in accordance with the MassDEP Stormwater Management Standards and stormwater outfalls will be stabilized to prevent erosion.

**Standard 2: Peak Rate Attenuation**

The proposed stormwater management system will be designed so that the post-development peak discharge rates do not exceed pre-development peak discharge rates. To prevent storm damage and downstream flooding, the proposed stormwater management practices will mitigate peak runoff rates for the 2-, 10-, 25- and 100-year, 24-hour storm events. Refer to Table 6 for a pre- and post-development peak runoff rate comparison.

**Standard 3: Groundwater Recharge**

The Site was designed using environmentally sensitive site design, low impact development techniques, and green infrastructure stormwater treatment trains to increase annual recharge to groundwater. The annual recharge from the post-development site will approximate the annual recharge from pre-development conditions based on soil type using the guidelines provided in the MassDEP Stormwater Management Handbook.

Impervious Area in HSG A = 2.4 Acres (Parking Lots)  
 Rv (Recharge Volume) = 2.4 ac x 43,560 sf/ac x 0.6 in. / (12 inches/ft)  
 = 5,227 cubic feet

The infiltration BMPs are sized to exceed the recharge volume required under the MassDEP Stormwater Management Standards (Table 5). The recharge volume shown in Table 7 represents the static storage below the outlet as provided in HydroCAD.

**Table 7 – Proposed Recharge Volumes for Stormwater BMPs**

Infiltration BMP	Recharge Volume (cf) <sup>A</sup>
Subsurface Infiltration System #1	1,626
Subsurface Infiltration System #2	2,512
Subsurface Infiltration System #3	897
Bioretention Basin #1	270
Bioretention Basin #3A	281
Bioretention Basin #3B	141
Bioretention Basin #6B	223
<b>TOTAL</b>	<b>5,950</b>

a) The recharge volume is based on the static method analysis, which does not consider volume recharged via infiltration. The volume reflected is the storage volume below the outlet.

The HydroCAD reports provided in Appendix C indicate that all proposed infiltration BMPs will drain within 48 hours for the 2-, 10-, 25-, and 100-year storm events, meeting the 72-hour MassDEP drawdown requirement.

**Standard 4: Water Quality Treatment**

As detailed in Section 3.3., the proposed green infrastructure stormwater improvements have been designed to remove greater than 80% of the average annual post-construction load of Total Suspended Solids (TSS). Structural stormwater BMPs including deep sump and hooded catch basins, Isolator Rows, bioretention areas, and subsurface infiltration systems are sized to capture the required water quality volume (1 inch over the project site) and remove a minimum of 80% of total suspended solids.

The proposed water quality treatment BMPs are subject to the 44% TSS removal pretreatment requirement and the 1-inch rule for calculating water quality volumes to the maximum extent practicable because the site discharges to a bathing beach. Pretreatment prior to infiltration will meet the 44% TSS removal requirement to the maximum extent practicable using deep sump and hooded catch basins, Isolator Rows, sediment forebays, and the filtration course of the bioretention basins.

TSS removal calculation spreadsheets are provided in Appendix A.

Source control and pollution prevention measures, such as vacuum cleaning, street sweeping, proper snow management, and stabilization of eroded surfaces, are included in the Long-Term Pollution Prevention Plan and Operation and Maintenance Plan (Appendix E).

#### **Standard 5: Land Uses with Higher Potential Pollutant Loads (LUHPPL)**

The Project is not considered a LUHPPL and therefore, this standard is not applicable.

#### **Standard 6: Critical Areas**

The Project is subject to Standard #6 as it discharges to a bathing beach. The proposed water quality treatment BMPs are subject to the 44% TSS removal pretreatment requirement and the 1-inch water quality volume.

The Project meets these standards to the maximum extent practicable.

#### **Standard 7: Redevelopments**

Due to the overall reduction in impervious area, the Project is considered a redevelopment under the MassDEP Stormwater Management Standards. Therefore, the Project is required to meet Standard 2, Standard 3, and the pretreatment and structural stormwater BMP requirements of Standards 4, 5, and 6 to the maximum extent practicable. The Project should comply with all other requirements of the Stormwater Management Standards and improve existing conditions.

The Project meets this standard.

#### **Standard 8: Construction Period Pollution Prevention and Sedimentation Control**

A plan to control construction-related impacts, including erosion, sedimentation, and other pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) will be developed and implemented during the Notice of Intent permitting process. The contractor will be required to implement construction-period erosion and sediment controls as documented in the Project plan and specifications. These requirements include erosion and sediment controls (stabilization practices and structural practices), temporary and permanent stormwater management measures, materials management, waste disposal, off-site vehicle tracking, and spill prevention and response.



### **Standard 9: Operation and Maintenance Plan**

A post-construction operation and maintenance plan has been prepared and will be implemented to ensure that stormwater management systems function as designed. Source control and stormwater BMP operation requirements for the site are summarized in the Long-Term Pollution Prevention Plan and Operation and Maintenance Plan provided in Appendix E.

### **Standard 10: Prohibition of Illicit Discharges**

There will be no illicit discharges to the stormwater management system associated with the Project. An Illicit Discharge Compliance Statement is provided in Appendix A.

## **8.0 CONCLUSION**

---

In conclusion, the Project is providing significant improvements to stormwater being discharged to East Beach through the implementation of green infrastructure and water quality treatment BMPs including bioretention basins, increased vegetation, and infiltration. The proposed stormwater improvements will reduce peak runoff rates and volumes and improve the water quality of stormwater being discharged from the Site. The Project has been designed to meet and exceed the MassDEP Stormwater Management Standards and the City of New Bedford Stormwater Management Rules and Regulations. The Project was also designed to align the with the City of New Bedford Green Infrastructure Plan stormwater quality volume goals.

## **FIGURES**

---

DR-1A	Existing Watershed Areas (30 Scale)
DR-1B	Existing Watershed Areas (100 Scale)
DR-2A	Proposed Watershed Areas (30 Scale)
DR-2B	Proposed Watershed Areas (100 Scale)

## **APPENDIX A**

---

### **Stormwater Management Standards Documentation**

MassDEP Checklist for Stormwater Report

Standard 4: TSS Removal Calculations

Phosphorous Removal Calculations

Standard 4: Isolator Row Sizing Calculations

Standard 10: Illicit Discharge Compliance Statement



# Checklist for Stormwater Report

## B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

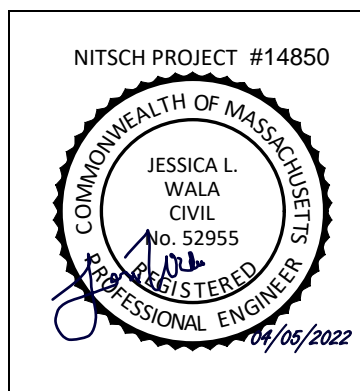
*Note:* Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

### Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



*Jessica Wala* 4/5/22  
Signature and Date

### Checklist

**Project Type:** Is the application for new development, redevelopment, or a mix of new and redevelopment?

- New development
- Redevelopment
- Mix of New Development and Redevelopment



# Checklist for Stormwater Report

---

## Checklist (continued)

**LID Measures:** Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- No disturbance to any Wetland Resource Areas
  - Site Design Practices (e.g. clustered development, reduced frontage setbacks)
  - Reduced Impervious Area (Redevelopment Only)
  - Minimizing disturbance to existing trees and shrubs
  - LID Site Design Credit Requested:
    - Credit 1
    - Credit 2
    - Credit 3
  - Use of “country drainage” versus curb and gutter conveyance and pipe
  - Bioretention Cells (includes Rain Gardens)
  - Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
  - Treebox Filter
  - Water Quality Swale
  - Grass Channel
  - Green Roof
  - Other (describe):      Subsurface Infiltration System
- 

### Standard 1: No New Untreated Discharges

- No new untreated discharges
- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



# Checklist for Stormwater Report

---

## Checklist (continued)

### Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

### Standard 3: Recharge

- Soil Analysis provided.
- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.
  - Static
  - Simple Dynamic
  - Dynamic Field<sup>1</sup>
- Runoff from all impervious areas at the site discharging to the infiltration BMP.
- Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
  - Site is comprised solely of C and D soils and/or bedrock at the land surface
  - M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
  - Solid Waste Landfill pursuant to 310 CMR 19.000
  - Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

---

<sup>1</sup> 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



# Checklist for Stormwater Report

---

## Checklist (continued)

### Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

### Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
  - Provisions for storing materials and waste products inside or under cover;
  - Vehicle washing controls;
  - Requirements for routine inspections and maintenance of stormwater BMPs;
  - Spill prevention and response plans;
  - Provisions for maintenance of lawns, gardens, and other landscaped areas;
  - Requirements for storage and use of fertilizers, herbicides, and pesticides;
  - Pet waste management provisions;
  - Provisions for operation and management of septic systems;
  - Provisions for solid waste management;
  - Snow disposal and plowing plans relative to Wetland Resource Areas;
  - Winter Road Salt and/or Sand Use and Storage restrictions;
  - Street sweeping schedules;
  - Provisions for prevention of illicit discharges to the stormwater management system;
  - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
  - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
  - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
  - Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
    - is within the Zone II or Interim Wellhead Protection Area
    - is near or to other critical areas
    - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
    - involves runoff from land uses with higher potential pollutant loads.
  - The Required Water Quality Volume is reduced through use of the LID site Design Credits.
  - Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



# Checklist for Stormwater Report

---

## Checklist (continued)

### Standard 4: Water Quality (continued)

- The BMP is sized (and calculations provided) based on:
  - The ½" or 1" Water Quality Volume or
  - The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

### Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- The NPDES Multi-Sector General Permit does **not** cover the land use.
- LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- All exposure has been eliminated.
- All exposure has **not** been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

### Standard 6: Critical Areas

- The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- Critical areas and BMPs are identified in the Stormwater Report.





# Checklist for Stormwater Report

---

## Checklist (continued)

### Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
- Limited Project
  - Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
  - Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
  - Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
  - Bike Path and/or Foot Path
  - Redevelopment Project
  - Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

### Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
  - Construction Period Operation and Maintenance Plan;
  - Names of Persons or Entity Responsible for Plan Compliance;
  - Construction Period Pollution Prevention Measures;
  - Erosion and Sedimentation Control Plan Drawings;
  - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
  - Vegetation Planning;
  - Site Development Plan;
  - Construction Sequencing Plan;
  - Sequencing of Erosion and Sedimentation Controls;
  - Operation and Maintenance of Erosion and Sedimentation Controls;
  - Inspection Schedule;
  - Maintenance Schedule;
  - Inspection and Maintenance Log Form.
- A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



# Checklist for Stormwater Report

---

## Checklist (continued)

### Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- The project is **not** covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

### Standard 9: Operation and Maintenance Plan

- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
  - Name of the stormwater management system owners;
  - Party responsible for operation and maintenance;
  - Schedule for implementation of routine and non-routine maintenance tasks;
  - Plan showing the location of all stormwater BMPs maintenance access areas;
  - Description and delineation of public safety features;
  - Estimated operation and maintenance budget; and
  - Operation and Maintenance Log Form.
- The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
  - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
  - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

### Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.

**EAST BEACH, NEW BEDFORD, MA  
TSS REMOVAL – WATER QUALITY TREATMENT SUMMARY**

---

Nitsch Engineering has prepared this Water Quality Treatment Summary for the proposed East Beach in New Bedford, MA. In compliance with MassDEP Stormwater Management Standard #4, the proposed stormwater management system is designed to remove at least 80% of the average annual post-construction load of TSS prior to discharge. The stormwater management system is designed to remove at least 44% of the average annual post-construction TSS load prior to discharge to the infiltration systems to the maximum extent practicable.

A summary of treatment trains proposed to provide water quantity control and water quality improvement at the proposed project site is provided below.

**Treatment Train A**  
**Infiltration Bioretention Basin**  
**[BB#3A, BB#3B, BB#6B]**

Parking Lot Runoff + Contributing Residential Parcels → Sediment Forebay → Bioretention Basin

**Treatment Train B**  
**Lined Biofiltration Basin**  
**[BB#2A, BB#4A, BB#4B, BB#5A, BB#6A, BB#7A, BB#7B, BB#8A, BB#8B]**  
Parking Lot Runoff + Contributing Residential Parcels → Biofiltration Basin

**Treatment Train C**  
**Bioretention Basin, overflow to Isolator Rows**  
**[BB#1, BB#2B, BB#5B, BB#9]**  
Parking Lot Runoff + Contributing Residential Parcels → Overflow to Isolator Row

**Treatment Train D**  
**Upstream Watershed to Infiltration System**  
**[CB1, CB3, CB4, CB5]**  
Roadway and Residential Runoff → Isolator Row → Infiltration System



**Treatment Train A**

Parking Lot Runoff + Contributing Residential Parcels → Sediment Forebay → Bioretention Basin

**Pretreatment Spreadsheet**

B	C	D	E	F
BMP	TSS Removal Rate	Starting TSS Load	Amount Removed (C*D)	Remaining Load (D-E)
Sediment Forebay	0.25	1.00	0.25	0.75

Total TSS Removal =

25%
-----

**Project is a  
 Redevelopment –  
 Meets standard to  
 maximum extent  
 practicable**

**Treatment Spreadsheet**

B	C	D	E	F
BMP	TSS Removal Rate	Starting TSS Load	Amount Removed (C*D)	Remaining Load (D-E)
Bioretention Basin	0.90	1.00	0.90	0.10

Total TSS Removal =

90%
-----

**Meets 80% TSS  
 removal requirement**

**Treatment Train B**

Parking Lot Runoff + Contributing Residential Parcels → Lined Biofiltration Basin

**Treatment Spreadsheet**

<b>B</b> BMP	<b>C</b> TSS Removal Rate	<b>D</b> Starting TSS Load	<b>E</b> Amount Removed (C*D)	<b>F</b> Remaining Load (D-E)
Bioretention Basin	0.90	1.00	0.90	0.10

Total TSS Removal =

90%
-----

**Meets 80% TSS  
removal requirement**



**Treatment Train C**

Parking Lot Runoff + Contributing Residential Parcels → Overflow to Isolator Row

**Treatment Spreadsheet**

<b>B</b> BMP	<b>C</b> TSS Removal Rate	<b>D</b> Starting TSS Load	<b>E</b> Amount Removed (C*D)	<b>F</b> Remaining Load (D-E)
Isolator Row	0.81	1.00	0.82	0.19

Total TSS Removal = 81% **Meets 80% TSS  
removal requirement**



**Treatment Train D**

Parking Lot Runoff + Contributing Residential Parcels → Sediment Forebay → Bioretention Basin

**Pretreatment Spreadsheet**

B BMP	C TSS Removal Rate	D Starting TSS Load	E Amount Removed (C*D)	F Remaining Load (D-E)
Deep Sump & Hooded Manhole	0.25	1.00	0.25	0.75
Isolator Row	0.81	0.75	0.61	0.14

Total TSS Removal =

86%

Meets 44% TSS removal pretreatment requirement

**Treatment Spreadsheet**

B BMP	C TSS Removal Rate	D Starting TSS Load	E Amount Removed (C*D)	F Remaining Load (D-E)
Infiltration System	0.80	1.00	0.80	0.20

Total TSS Removal =

80%

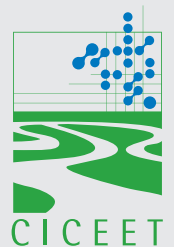
Meets 80% TSS removal requirement



University of New Hampshire  
Stormwater Center



2009 BIENNIAL REPORT





# Contents

- 3 Directors' Message
- 4 Highlights
- 8 Field Research Site
- 10 Applying UNHSC Research
- 12 Reading this Report
- 14–30 Stormwater Treatment System Data
  - 14 Porous Asphalt
  - 16 Pervious Concrete
  - 18 Subsurface Gravel Wetland
  - 20 Bioretention Systems
  - 22 Tree Box Filter
  - 24 StormTech Isolator Row
  - 26 Retention Ponds
  - 28 Deep Sump Catch Basins
  - 30 Filter Berm Swales
- 32–34 Targeted Research
- 35 Administration

Workshop at UNHSC field site



## About the Center

The University of New Hampshire Stormwater Center (UNHSC) is dedicated to the protection of water resources through effective stormwater management.

Center researchers evaluate and enhance the ability of stormwater treatment systems to treat the pollution in stormwater runoff and reduce the flooding that it can cause. The Center provides information on performance, cost, design, and maintenance to people who select, review, permit, design, install, and maintain stormwater management systems. The research is integrated with an evolving outreach

program that supports a wide range of stormwater managers and professionals who seek to build programs that protect water quality, preserve environmental values, and reduce the impact of stormwater runoff.

The Center receives its primary funding and program support from the Cooperative Institute for Coastal and Estuarine Environmental Technology (CICEET), a partnership of UNH and the National Oceanic and Atmospheric Administration (NOAA). It is housed within the University's Environmental Research Group, a division of the College of Engineering and Physical Sciences.

## Resources for Stormwater Managers

The Center's research has served as the foundation for a range of outreach products—from best management practice (BMP) workshops geared to support municipal decision makers and stormwater engineers to peer-reviewed publications that explore the frontiers of stormwater science. Learn more about these resources at [www.unh.edu/erg/cstev](http://www.unh.edu/erg/cstev).



- BMP Fact Sheets
- Data Reports
- Design Drawings
- Design Specs
- Journal Articles
- Web Resources

## Directors' Message



UNH Stormwater Center  
field site

These economic times challenge all of us to make difficult choices about what we can and cannot afford. For state and local governments facing budget shortfalls, the University of New Hampshire Stormwater Center has some welcome news: when it comes to effective stormwater management, you do not have to choose between affordability and healthy waters.

People often tell us that they think they do have to choose, that even if Low Impact Development (LID) stormwater techniques do a better job of protecting water quality, they are too costly to install and maintain. Yet our research is demonstrating that this is not the case. Since 2004 we have monitored the ability of 23 stormwater systems to treat pollution and reduce the volume of runoff. We have worked with hundreds of municipal officials, regulators, engineers, contractors, and educators on dozens of stormwater demonstration and education projects.

In the process, we have found that projects that use LID approaches to managing stormwater runoff can be both more effective in treating pollution and in some instances less expensive to install than those that rely on curbs, pipes, and ponds. LID systems do require maintenance to function properly, but so do all of the commonly used systems that are believed to require little or no attention. In particular, our research has demonstrated that when retention ponds are not adequately maintained, they not only fail to remove pollutants from runoff; they can magnify the negative impact of polluted stormwater on receiving waters.

Using LID approaches for stormwater management involves decentralizing runoff and maximizing infiltration, which ultimately reduces the stress on urban stormwater infrastructure. Metropolitan areas like Portland, OR., are already seeing the economic benefits of using LID to reduce the runoff flowing through their combined sewers. These savings extend to residential and commercial development and redevelopment projects. Homeowners that use techniques like rain barrels, drought resistant rain gardens, and porous pavements can save on water utility bills and help prevent flooded basements.

By allowing for these less familiar but more effective techniques in stormwater ordinances, municipalities can help insure these benefits at every level. In so doing, they anticipate the inevitable. Federal laws requiring LID-style approaches to stormwater are already in place as part of Phase II of the Clean Water Act. It is only a matter of time before all municipalities will have to comply with mandates to clean up impaired waters, and our research is showing that in many cases an LID approach to stormwater management is essential in meeting that goal.

A proactive response to federal regulations has the added benefit of preparing us for the impacts of climate change. Whether climate change has brought severe storms or drought to a community, LID stormwater techniques can help mitigate the flooding associated with impervious surfaces, can allow rainfall to replenish aquifers, and can be powerful tools for adaptive management.

This report is one of many tools we use to communicate our work in a way that we hope stormwater managers from many backgrounds will find useful. We welcome your comments and questions, about this report and all of the work we do.

Sincerely,

Handwritten signature of Robert Roseen in black ink.

**Robert Roseen**  
Director

Handwritten signature of Thomas Ballestero in black ink.

**Thomas Ballestero**  
Senior Scientist

Handwritten signature of Jamie Houle in black ink.

**Jamie Houle**  
Program Manager  
and Outreach Coordinator

# StormTech Isolator Row



The StormTech Isolator Row is an effective filtration/infiltration system best suited to locations where space is at a premium and the system's relatively expensive installation cost can be offset by increasing available space for development.

## About the StormTech Isolator Row

The StormTech Isolator Row is a manufactured system designed to provide subsurface water quality treatment and easy access for maintenance. It is typically used to remove pollution from runoff before it flows into unlined infiltration chambers designed for detention and water quantity control. The Isolator Row consists of a series of StormTech chambers installed over a layer of woven geotextile, which sits on a crushed stone infiltration bed surrounded with filter fabric. The bed is directly connected to an upstream manhole for maintenance access and large storm bypass. At UNHSC, the Isolator Row has met a TSS median annual removal standard of 80 percent, and exhibited an enhanced capacity to remove phosphorus. The Isolator Row is well suited for urban environments where space is at a premium.

## Implementation

The StormTech Isolator Row is part of a class of manufactured, subsurface filtration/infiltration systems that are being used more and more throughout the United States. In general, these systems are best suited to locations where above ground space is at a premium. They are often used in urban areas, where they are located beneath parking lots and other

infrastructure. As with any infiltration system, care must be taken when locating these systems near pollution hotspots, or where seasonal high groundwater levels may lead to groundwater contamination. In such cases, if installed, the systems should be lined to prevent infiltration into groundwater, and outfitted with subdrains that discharge to the surface. Designs for the StormTech Isolator Row are available from the manufacturer.

## System Performance

### Cost & Maintenance

While subsurface HDPE systems such as the Isolator Row tend to be more expensive than conventional stormwater treatments like retention ponds, the costs are ameliorated by the increase in available space for development. The cost to install a StormTech Isolator Row system large enough to treat runoff from one acre of impervious surface was \$34,000 in 2006.

In more than two years of operation, the system is at less than 50 percent of its recommended maintenance trigger point. Maintenance should be conducted when the sediment in the chambers reaches approximately three inches in depth according to recommendations from the manufacturer. Sediment accumulation can be monitored through inspection ports. When maintenance is needed, the entire row can be

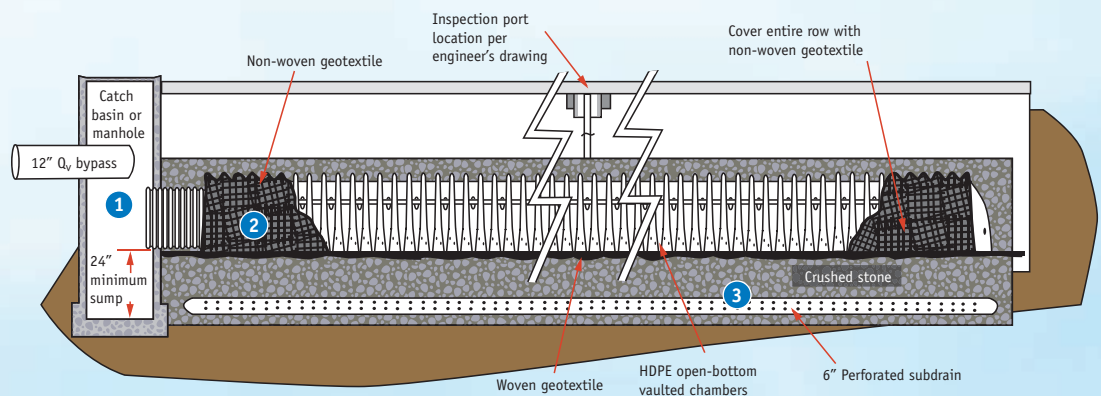
CATEGORY / BMP TYPE	Water Quality: Physical (Sedimentation, Filtration) & Chemical (Sorption)	SPECIFICATIONS	MAINTENANCE
Filtration, Infiltration, Manufactured Treatment Device		Catchment Area: 1 acre	Maintenance Sensitivity: Low
UNIT OPERATIONS & PROCESSES	DESIGN SOURCE: StormTech, LLC	Water Quality Flow: 1 cfs	Inspections: High
Hydrologic (Flow Alteration)	BASIC DIMENSIONS: Chamber: 51" wide X 30" high X 85.4" long	Water Quality Volume: 3,300 cf	Sediment Removal: Moderate
		INSTALLATION COST: \$34,000 per acre treated	

Fast Facts

## How the System Works

## WATER QUALITY TREATMENT PROCESS

1. Runoff flows into the Isolator Row chambers from a catchbasin or pipe.
2. Runoff slowly passes from the chambers through a woven geotextile fabric and into the crushed stone reservoir below the system. The runoff passes through the fabric, leaving behind sediments and associated contaminants through the physical unit operations of filtration and sedimentation. As an organic filter cake develops over the fabric, phosphorus is also removed via the chemical process of sorption.
3. Filtered runoff collects in a perforated subdrain and returns to a storm drain system, infiltrates into the subgrade, or is discharged to the surface.



washed clean through an access manhole and by a hydro-jet with sediment removed by vactoring (vacuuming). Entry into the system is considered a confined space entry and requires trained personnel and equipment.

During two years of evaluation at UNHSC, the Isolator Row has accumulated, at most, one and one half inches of sediment in its chambers. As a result, researchers have not performed maintenance on the system. The Isolator Row presents an interesting opportunity to study the relationship between maintenance and performance. Researchers have observed enhanced phosphorus removal as the system develops an organic filter cake between the chambers and the woven geotextile fabric that lies beneath them. This enhancement is tempered by the likelihood that, as the filter cake continues to grow, hydraulic efficiency will decline and more runoff will bypass the system untreated until maintenance is performed. Analyses are underway to develop maintenance recommendations that balance and optimize the water quality and water quantity management abilities of this system.

### Cold Climate

This system's water quality treatment and volume control capacity remained strong in all seasons, reinforcing the conclusion that filtration and infiltration systems perform well, even in cold climates.

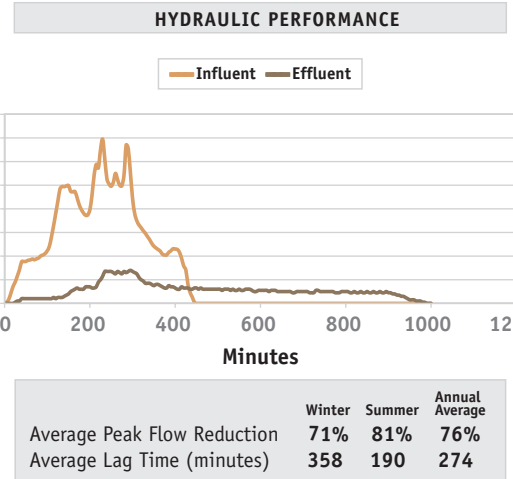
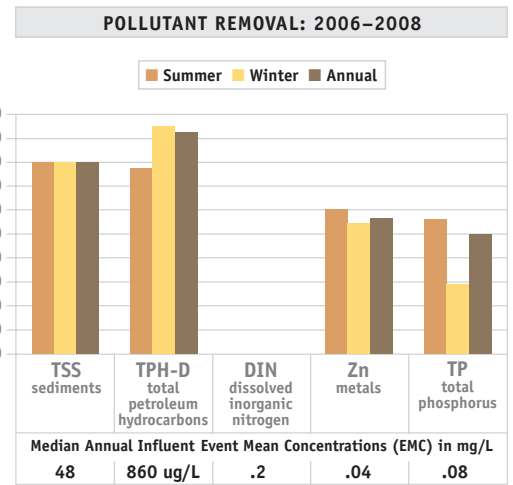
### Water Quality Treatment

The StormTech Isolator Row system does a good job of reducing the concentration of common pollutants associated with stormwater performance assessment with the exception of nitrogen. It generally meets EPA's recommended level of removal for total suspended solids, and meets regional ambient water quality criteria for heavy metals and petroleum hydrocarbons. The system has a capacity to achieve modest levels of total phosphorus removal, which may be enhanced over time. (See Cost & Maintenance Section.) The lack of nitrogen treatment is typical for non-vegetated aerobic systems. Nutrient load reduction would be further increased through volume reduction by infiltration. Like all other systems monitored at UNHSC, it does not provide chloride removal.

The chart at top right reflects the system's performance in removing total suspended solids, total petroleum hydrocarbons, dissolved inorganic nitrogen, total phosphorus, and zinc. Values represent results recorded over a two-year monitoring period, with the data further divided into summer and winter components.

### Water Quantity Control

Like other infiltration and filtration systems, the StormTech Isolator Row system exhibits the capacity to reduce peak flows and could be used to reduce runoff volume in appropriate soils, such as those belonging to groups "A" or "B." The figure at bottom right provides information on peak flow reduction and lag times for the system.



## SYSTEM DESIGN ▼

The StormTech Isolator Row is designed to provide subsurface water quality treatment for small storms. The manufacturer adapts the system's design in accordance with local watershed conditions and target treatment objectives.

Chamber units are made of high-density polyethylene (HDPE) pipe and are designed to bear loads consistent with those experienced by parking lots. The UNHSC chamber dimensions are 51 x 30 x 85.4 inches and can be linked together to form linear rows up to 200 feet long. The chambers are laid over woven geotextile, which rests on an infiltration base composed of one foot of three quarter inch crushed stone. The entire excavation is then wrapped in nonwoven geotextile to protect the system from the migration of fine particles from the surrounding soil.

A three- to five-foot separation from seasonal high groundwater table (as designated by regulations) is necessary to minimize the potential for groundwater contamination. Stormwater flows of

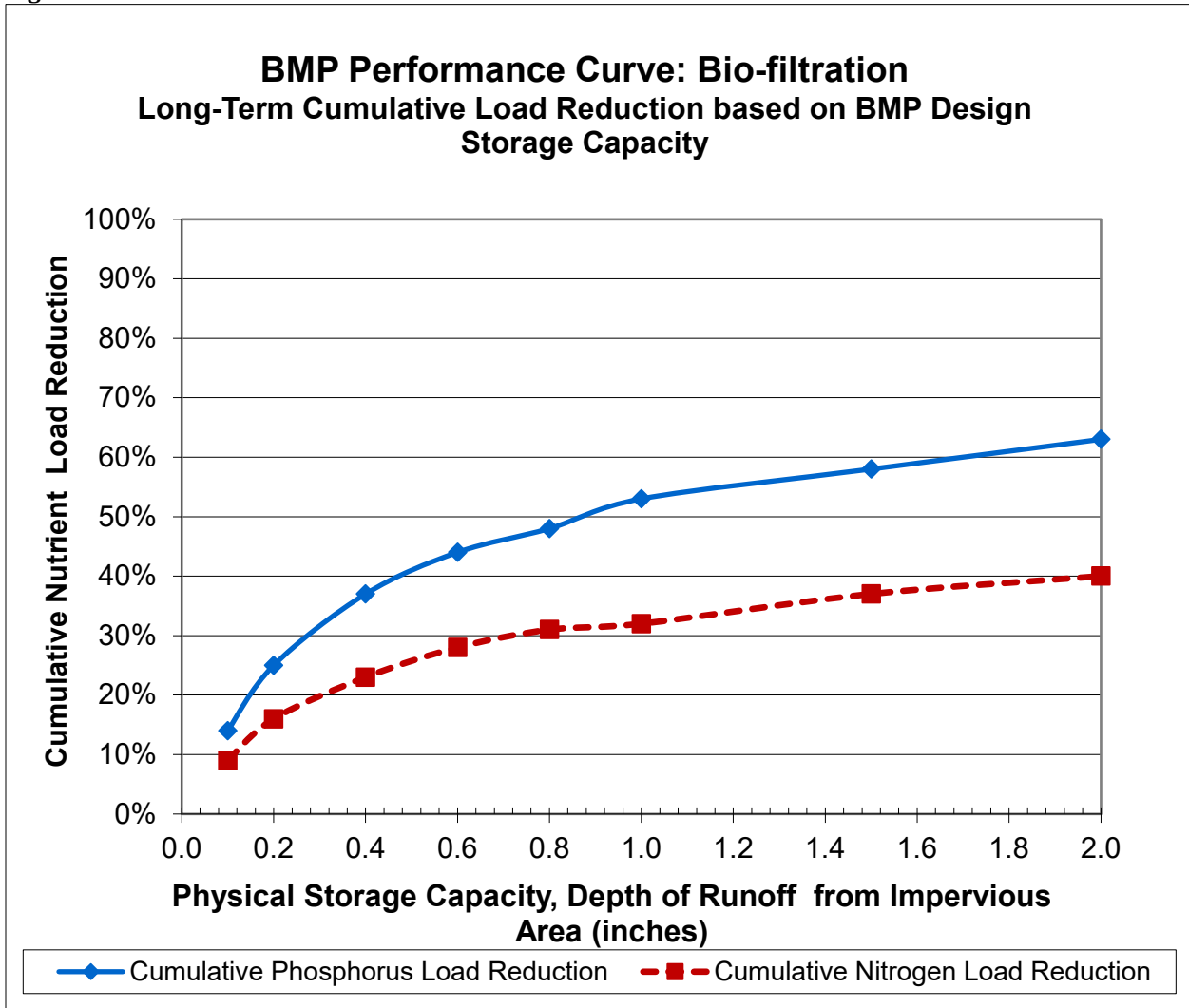
up to one cubic foot per second (cfs) enter the system through an upstream manhole or other flow diverter. This is representative of flow-based sizing of a BMP common for devices that have limited detention or storage. Such devices are often better described by a maximum treatable flow rate as opposed to a treatment volume.

A bypass is incorporated in the StormTech system where flows exceeding the design rate are bypassed around the device and flow directly into adjacent chambers that can be sized to treat the  $C_p$  and  $Q_p$ . Because of the bypass design, maintenance requirements are extremely important. A poorly maintained device would bypass prematurely into the unlined chamber systems and eventually clog subsurface soils resulting in system failure.

**Table 3-18: Bio-filtration BMP Performance Table**

Bio-filtration BMP Performance Table: Long-Term Phosphorus & Nitrogen Load Reduction								
BMP Capacity: Depth of Runoff from Impervious Area (inches)	0.1	0.2	0.4	0.6	0.8	1.0	1.5	2.0
Cumulative Phosphorus Load Reduction	14%	25%	37%	44%	48%	53%	58%	63%
Cumulative Nitrogen Load Reduction	9%	16%	23%	28%	31%	32%	37%	40%

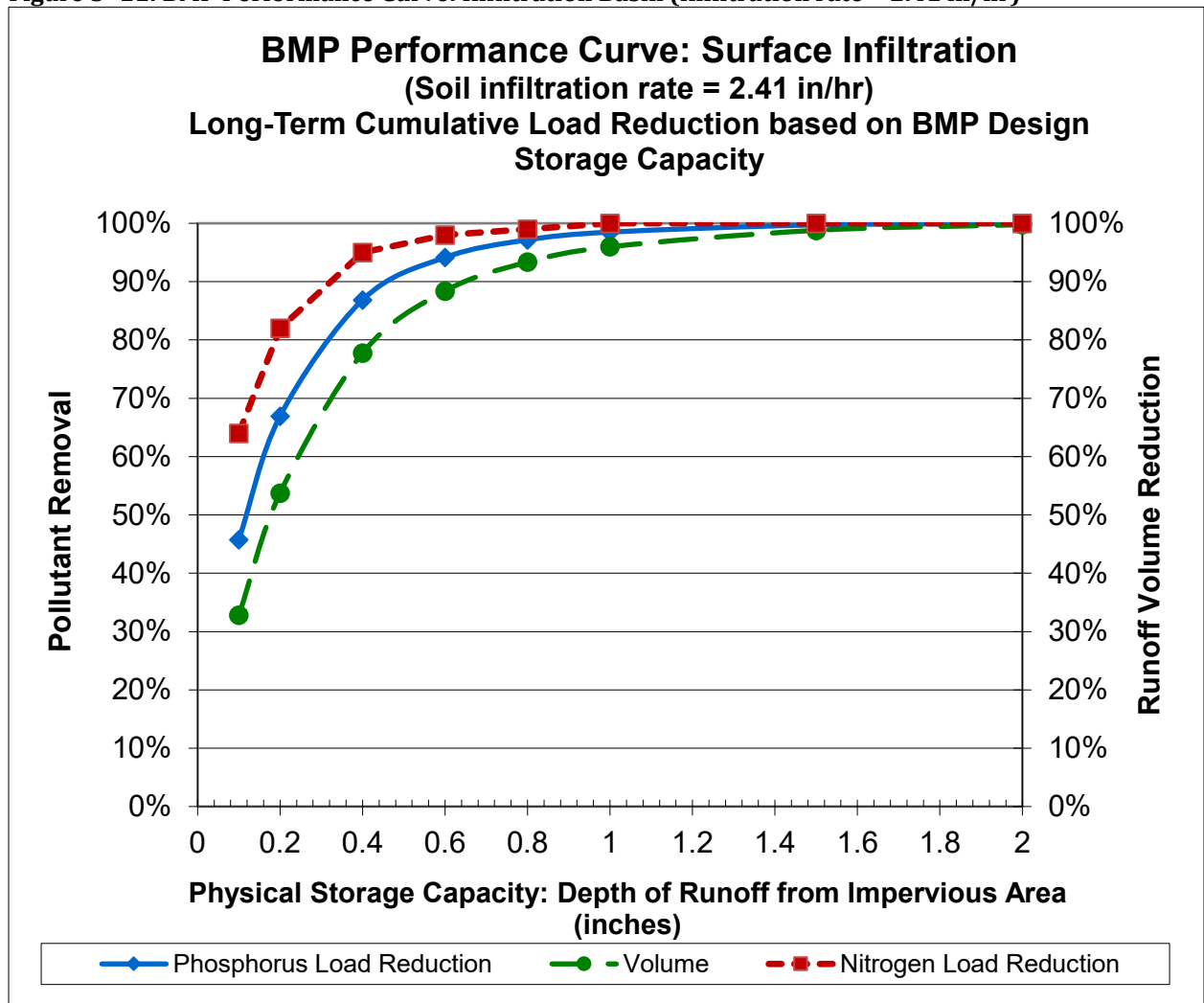
**Figure 3- 13: BMP Performance Curve: Bio-filtration**



**Table 3- 16: Surface Infiltration (2.41 in/hr) BMP Performance Table**

Surface Infiltration (2.41 in/hr) BMP Performance Table: Long-Term Phosphorus Load Reduction								
BMP Capacity: Depth of Runoff from Impervious Area (inches)	0.1	0.2	0.4	0.6	0.8	1.0	1.5	2.0
Runoff Volume Reduction	32.8%	53.8%	77.8%	88.4%	93.4%	96.0%	98.8%	99.8%
Cumulative Phosphorus Load Reduction	46%	67%	87%	94%	97%	98%	100%	100%
Cumulative Nitrogen Load Reduction	64%	82%	95%	98%	99%	100%	100%	100%

**Figure 3- 11: BMP Performance Curve: Infiltration Basin (infiltration rate = 2.41 in/hr)**





STORMTECH ISOLATOR ROW SIZING CHART					
	SC-310	SC-740	DC-780	MC-3500	MC-4500
Chamber Area (Sq.Ft.)	20	27.8	27.8	43.2	30.1
Treated Flow Rate per chamber (CFS)	0.11	0.15	0.15	0.24	0.17

**NOTE:** Testing of the Isolator Row completed by Tennessee Tech has been verified by NJCAT and it has shown to have a TSS removal efficiency of 84% for SIL-CO-SIL 250  
 NJCAT verified Treated Flow Rate (GPM / Sq.Ft.) 2.5

## NON-INFILTRATIVE ISOLATOR ROWS

### PARKING LOT A

BMP-1 OVERFLOW: 0.43 CFS  
 4 SC-310 CHAMBERS

BMP-2 OVERFLOW: 0.70 CFS  
 7 SC-310 CHAMBERS

TOTAL: 11 CHAMBERS

### PARKING LOT B

BMP-5b OVERFLOW: 0.79 CFS  
 8 SC-310 CHAMBERS

### PARKING LOT C

BMP-9 OVERFLOW: 1.00 CFS  
 10 SC-310 CHAMBERS

## SUBSURFACE INFILTRATION SYSTEM ISOLATOR ROWS

### SUBSURFACE INFILTRATION SYSTEM #1

1.7-inch WQF: 0.42 cfs  
 5 SC-310 Chambers

### SUBSURFACE INFILTRATION SYSTEM #2

1.7-inch WQF: 2.62  
 18 SC-740 Chambers

### SUBSURFACE INFILTRATION SYSTEM #3

1.7-inch WQF: 0.36 cfs  
 4 SC-310 Chambers

**STANDARD 10: Illicit Discharge Compliance Statement**

Project Name: East Beach Green Infrastructure Project	Nitsch Project #: 14850
Location: New Bedford, MA	Checked by: JJ
Prepared by: JW	Sheet No. 1 of 1
Date: 4/5/22	

**Standard 10 states: All illicit discharges to the stormwater management system are prohibited.**

This is to verify:

1. Based on the information available there are no known or suspected illicit discharges to the stormwater management system at East Beach site in New Bedford, MA as defined in the MassDEP Stormwater Handbook.
2. The design of the stormwater system includes no proposed illicit discharges.



\_\_\_\_\_  
Jessica Wala, PE

4/5/22

\_\_\_\_\_  
Date

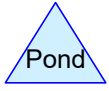
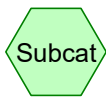
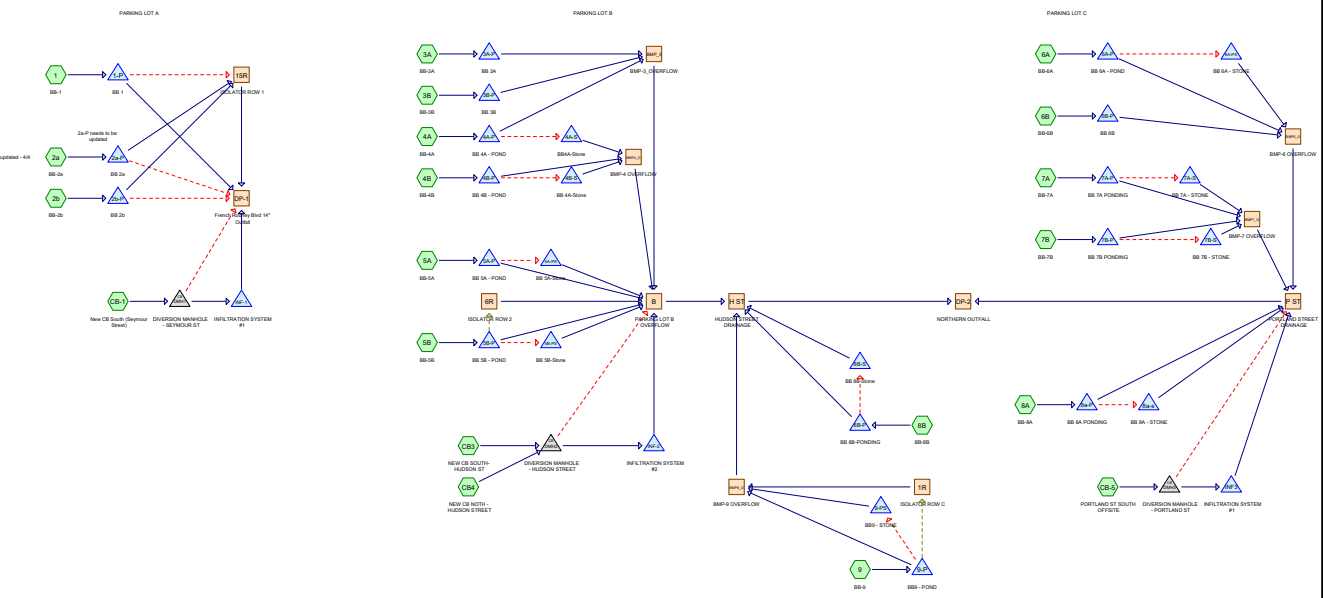


## **APPENDIX B**

---

### **1.7-inch Water Quality Volume Analysis – HydroCAD Calculations**

# 1.7-INCH WATER QUALITY VOLUME ANALYSIS



**Routing Diagram for 14850\_Proposed-Drainage-Areas**  
 Prepared by {enter your company name here}, Printed 4/5/2022  
 HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

## 14850\_Proposed-Drainage-Areas

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Printed 4/5/2022

Page 2

### Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
214,977	83	1/4 acre lots, 38% imp, HSG C (1, 2b, 3A, 5B, 6A, 6B, 9, CB-1, CB-5, CB3, CB4)
16,767	74	>75% Grass cover, Good, HSG C (1, 2a, 2b, 3A, 3B, 4A, 4B, 5A, 5B, 6A, 6B, 7A, 7B, 8A, 8B, 9)
107,061	98	Paved parking, HSG C (1, 2a, 2b, 3A, 3B, 4A, 4B, 5A, 5B, 6A, 6B, 7A, 7B, 8A, 8B, 9)
37,488	98	Roadway (CB-1, CB-5, CB3, CB4)
<b>376,293</b>	<b>88</b>	<b>TOTAL AREA</b>

# 14850\_Proposed-Drainage-Areas

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Printed 4/5/2022

Page 3

## Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
0	HSG B	
338,805	HSG C	1, 2a, 2b, 3A, 3B, 4A, 4B, 5A, 5B, 6A, 6B, 7A, 7B, 8A, 8B, 9, CB-1, CB-5, CB3, CB4
0	HSG D	
37,488	Other	CB-1, CB-5, CB3, CB4
<b>376,293</b>		<b>TOTAL AREA</b>

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 4

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: BB-1</b>	Runoff Area=38,826 sf 51.66% Impervious Runoff Depth=0.63" Tc=6.0 min CN=86 Runoff=0.71 cfs 2,036 cf
<b>Subcatchment2a: BB-2a</b>	Runoff Area=3,116 sf 92.62% Impervious Runoff Depth=1.29" Tc=6.0 min CN=96 Runoff=0.11 cfs 334 cf
<b>Subcatchment2b: BB-2b</b>	Runoff Area=21,490 sf 80.50% Impervious Runoff Depth=1.04" Tc=6.0 min CN=93 Runoff=0.65 cfs 1,868 cf
<b>Subcatchment3A: BB-3A</b>	Runoff Area=10,987 sf 58.16% Impervious Runoff Depth=0.73" Tc=6.0 min CN=88 Runoff=0.24 cfs 668 cf
<b>Subcatchment3B: BB-3B</b>	Runoff Area=4,545 sf 77.34% Impervious Runoff Depth=1.04" Tc=6.0 min CN=93 Runoff=0.14 cfs 395 cf
<b>Subcatchment4A: BB-4A</b>	Runoff Area=4,843 sf 86.37% Impervious Runoff Depth=1.20" Tc=6.0 min CN=95 Runoff=0.16 cfs 484 cf
<b>Subcatchment4B: BB-4B</b>	Runoff Area=3,048 sf 86.09% Impervious Runoff Depth=1.20" Tc=6.0 min CN=95 Runoff=0.10 cfs 305 cf
<b>Subcatchment5A: BB-5A</b>	Runoff Area=3,072 sf 73.44% Impervious Runoff Depth=0.97" Tc=6.0 min CN=92 Runoff=0.09 cfs 249 cf
<b>Subcatchment5B: BB-5B</b>	Runoff Area=34,755 sf 71.39% Impervious Runoff Depth=0.91" Tc=6.0 min CN=91 Runoff=0.92 cfs 2,623 cf
<b>Subcatchment6A: BB-6A</b>	Runoff Area=15,148 sf 46.97% Impervious Runoff Depth=0.58" Tc=6.0 min CN=85 Runoff=0.26 cfs 736 cf
<b>Subcatchment6B: BB-6B</b>	Runoff Area=6,495 sf 77.45% Impervious Runoff Depth=1.04" Tc=6.0 min CN=93 Runoff=0.20 cfs 564 cf
<b>Subcatchment7A: BB-7A</b>	Runoff Area=3,165 sf 87.74% Impervious Runoff Depth=1.20" Tc=6.0 min CN=95 Runoff=0.11 cfs 316 cf
<b>Subcatchment7B: BB-7B</b>	Runoff Area=4,942 sf 88.73% Impervious Runoff Depth=1.20" Tc=6.0 min CN=95 Runoff=0.17 cfs 494 cf
<b>Subcatchment8A: BB-8A</b>	Runoff Area=3,978 sf 79.99% Impervious Runoff Depth=1.04" Tc=6.0 min CN=93 Runoff=0.12 cfs 346 cf
<b>Subcatchment8B: BB-8B</b>	Runoff Area=5,598 sf 87.78% Impervious Runoff Depth=1.20" Tc=6.0 min CN=95 Runoff=0.19 cfs 559 cf
<b>Subcatchment9: BB-9</b>	Runoff Area=29,651 sf 74.77% Impervious Runoff Depth=0.97" Tc=6.0 min CN=92 Runoff=0.84 cfs 2,402 cf

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 5

<b>SubcatchmentCB-1: New CB South</b>	Runoff Area=19,582 sf 58.17% Impervious Runoff Depth=0.73" Flow Length=512' Tc=6.0 min CN=88 Runoff=0.42 cfs 1,191 cf
<b>SubcatchmentCB-5: PORTLANDST</b>	Runoff Area=19,743 sf 50.83% Impervious Runoff Depth=0.63" Flow Length=574' Tc=6.0 min CN=86 Runoff=0.36 cfs 1,035 cf
<b>SubcatchmentCB3: NEW CB SOUTH-</b>	Runoff Area=25,183 sf 51.84% Impervious Runoff Depth=0.63" Flow Length=635' Tc=6.0 min CN=86 Runoff=0.46 cfs 1,320 cf
<b>SubcatchmentCB4: NEW CB NOTH -</b>	Runoff Area=118,126 sf 49.24% Impervious Runoff Depth=0.63" Flow Length=822' Tc=6.0 min CN=86 Runoff=2.16 cfs 6,194 cf
<b>Reach 1R: ISOLATORROW C</b>	Inflow=0.77 cfs 1,195 cf Outflow=0.77 cfs 1,195 cf
<b>Reach 6R: ISOLATORROW 2</b>	Inflow=0.75 cfs 961 cf Outflow=0.75 cfs 961 cf
<b>Reach 15R: ISOLATORROW 1</b>	Inflow=0.72 cfs 2,650 cf Outflow=0.72 cfs 2,650 cf
<b>Reach B: PARKING LOT B OVERFLOW</b>	Inflow=2.54 cfs 4,951 cf Outflow=2.54 cfs 4,951 cf
<b>Reach BMP4_O: BMP-4 OVERFLOW</b>	Inflow=0.05 cfs 788 cf Outflow=0.05 cfs 788 cf
<b>Reach BMP6_O: BMP-6 OVERFLOW</b>	Inflow=0.04 cfs 736 cf Outflow=0.04 cfs 736 cf
<b>Reach BMP7_O: BMP-7 OVERFLOW</b>	Inflow=0.04 cfs 810 cf Outflow=0.04 cfs 810 cf
<b>Reach BMP9_O: BMP-9 OVERFLOW</b>	Inflow=0.81 cfs 2,402 cf Outflow=0.81 cfs 2,402 cf
<b>Reach BMP_3: BMP-3_OVERFLOW</b>	Inflow=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf
<b>Reach DP-1: French Rodney Blvd 14" Outfall</b>	Inflow=0.83 cfs 2,711 cf Outflow=0.83 cfs 2,711 cf
<b>Reach DP-2: NORTHERN OUTFALL</b>	Inflow=3.50 cfs 9,827 cf Outflow=3.50 cfs 9,827 cf
<b>Reach H ST: HUDSON STREET DRAINAGE</b>	Inflow=3.36 cfs 7,912 cf Outflow=3.36 cfs 7,912 cf
<b>Reach P ST: PORTLAND STREET DRAINAGE</b>	Inflow=0.14 cfs 1,915 cf Outflow=0.14 cfs 1,915 cf

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 6

<b>Pond 1-P: BB 1</b>	Peak Elev=9.90' Storage=630 cf Inflow=0.71 cfs 2,036 cf Discarded=0.04 cfs 1,515 cf Primary=0.00 cfs 0 cf Secondary=0.23 cfs 521 cf Outflow=0.27 cfs 2,036 cf
<b>Pond 2a-P: BB 2a</b>	Peak Elev=8.14' Storage=53 cf Inflow=0.11 cfs 334 cf Primary=0.10 cfs 298 cf Secondary=0.00 cfs 0 cf Outflow=0.10 cfs 298 cf
<b>Pond 2b-P: BB 2b</b>	Peak Elev=8.24' Storage=98 cf Inflow=0.65 cfs 1,868 cf Primary=0.62 cfs 1,832 cf Secondary=0.00 cfs 0 cf Outflow=0.62 cfs 1,832 cf
<b>Pond 3A-P: BB 3A</b>	Peak Elev=10.89' Storage=302 cf Inflow=0.24 cfs 668 cf Discarded=0.02 cfs 668 cf Primary=0.00 cfs 0 cf Outflow=0.02 cfs 668 cf
<b>Pond 3B-P: BB 3B</b>	Peak Elev=12.74' Storage=177 cf Inflow=0.14 cfs 395 cf Discarded=0.01 cfs 395 cf Primary=0.00 cfs 0 cf Outflow=0.01 cfs 395 cf
<b>Pond 4A-P: BB 4A - POND</b>	Peak Elev=9.91' Storage=151 cf Inflow=0.16 cfs 484 cf Primary=0.00 cfs 0 cf Secondary=0.03 cfs 484 cf Outflow=0.03 cfs 484 cf
<b>Pond 4A-S: BB4A-Stone</b>	Peak Elev=6.11' Storage=8 cf Inflow=0.03 cfs 484 cf Outflow=0.03 cfs 484 cf
<b>Pond 4B-P: BB 4B - POND</b>	Peak Elev=10.92' Storage=96 cf Inflow=0.10 cfs 305 cf Primary=0.00 cfs 0 cf Secondary=0.02 cfs 305 cf Outflow=0.02 cfs 305 cf
<b>Pond 4B-S: BB 4A-Stone</b>	Peak Elev=6.09' Storage=4 cf Inflow=0.02 cfs 305 cf Outflow=0.02 cfs 305 cf
<b>Pond 5A-P: BB 5A - POND</b>	Peak Elev=8.89' Storage=45 cf Inflow=0.09 cfs 249 cf Primary=0.00 cfs 0 cf Secondary=0.03 cfs 249 cf Outflow=0.03 cfs 249 cf
<b>Pond 5A-PS: BB 5A-Stone</b>	Peak Elev=6.11' Storage=16 cf Inflow=0.03 cfs 249 cf Outflow=0.03 cfs 249 cf
<b>Pond 5B-P: BB 5B - POND</b>	Peak Elev=8.86' Storage=457 cf Inflow=0.92 cfs 2,623 cf Primary=0.00 cfs 0 cf Secondary=0.07 cfs 1,663 cf Tertiary=0.75 cfs 961 cf Outflow=0.81 cfs 2,623 cf
<b>Pond 5B-PS: BB 5B-Stone</b>	Peak Elev=6.00' Storage=0 cf Inflow=0.07 cfs 1,663 cf Outflow=0.07 cfs 1,663 cf
<b>Pond 6A-P: BB 6A - POND</b>	Peak Elev=10.72' Storage=242 cf Inflow=0.26 cfs 736 cf Primary=0.00 cfs 0 cf Secondary=0.04 cfs 736 cf Outflow=0.04 cfs 736 cf
<b>Pond 6A-PS: BB 6A - STONE</b>	Peak Elev=6.13' Storage=11 cf Inflow=0.04 cfs 736 cf Outflow=0.04 cfs 736 cf
<b>Pond 6B-P: BB 6B</b>	Peak Elev=11.90' Storage=269 cf Inflow=0.20 cfs 564 cf Discarded=0.02 cfs 564 cf Primary=0.00 cfs 0 cf Outflow=0.02 cfs 564 cf
<b>Pond 7A-P: BB 7A PONDING</b>	Peak Elev=9.84' Storage=108 cf Inflow=0.11 cfs 316 cf Primary=0.00 cfs 0 cf Secondary=0.02 cfs 316 cf Outflow=0.02 cfs 316 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 7

**Pond 7A-S: BB 7A - STONE** Peak Elev=5.18' Storage=4 cf Inflow=0.02 cfs 316 cf  
Outflow=0.02 cfs 316 cf

**Pond 7B-P: BB 7B PONDING** Peak Elev=10.47' Storage=162 cf Inflow=0.17 cfs 494 cf  
Primary=0.00 cfs 0 cf Secondary=0.03 cfs 494 cf Outflow=0.03 cfs 494 cf

**Pond 7B-S: BB 7B - STONE** Peak Elev=5.21' Storage=5 cf Inflow=0.03 cfs 494 cf  
Outflow=0.03 cfs 494 cf

**Pond 8a-P: BB 8A PONDING** Peak Elev=8.71' Storage=87 cf Inflow=0.12 cfs 346 cf  
Primary=0.00 cfs 0 cf Secondary=0.03 cfs 346 cf Outflow=0.03 cfs 346 cf

**Pond 8a-s: BB 8A - STONE** Peak Elev=4.51' Storage=10 cf Inflow=0.03 cfs 346 cf  
Outflow=0.03 cfs 346 cf

**Pond 8B-P: BB 8B-PONDING** Peak Elev=9.60' Storage=186 cf Inflow=0.19 cfs 559 cf  
Primary=0.00 cfs 0 cf Secondary=0.03 cfs 559 cf Outflow=0.03 cfs 559 cf

**Pond 8B-S: BB 8B-Stone** Peak Elev=4.52' Storage=11 cf Inflow=0.03 cfs 559 cf  
Outflow=0.03 cfs 559 cf

**Pond 9-P: BB9 - POND** Peak Elev=8.66' Storage=253 cf Inflow=0.84 cfs 2,402 cf  
Primary=0.00 cfs 0 cf Secondary=0.04 cfs 1,207 cf Tertiary=0.77 cfs 1,195 cf Outflow=0.81 cfs 2,402 cf

**Pond 9-PS: BB9 - STONE** Peak Elev=6.13' Storage=7 cf Inflow=0.04 cfs 1,207 cf  
Outflow=0.04 cfs 1,207 cf

**Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST** Peak Elev=9.36' Inflow=0.42 cfs 1,191 cf  
Primary=0.31 cfs 1,130 cf Secondary=0.11 cfs 61 cf Outflow=0.42 cfs 1,191 cf

**Pond DMH2: DIVERSION MANHOLE - HUDSON STREET** Peak Elev=11.81' Inflow=2.62 cfs 7,514 cf  
Primary=0.93 cfs 6,224 cf Secondary=1.70 cfs 1,290 cf Outflow=2.62 cfs 7,514 cf

**Pond DMH3: DIVERSION MANHOLE - PORTLANDST** Peak Elev=11.31' Inflow=0.36 cfs 1,035 cf  
Primary=0.31 cfs 1,012 cf Secondary=0.05 cfs 23 cf Outflow=0.36 cfs 1,035 cf

**Pond INF-1: INFILTRATIONSYSTEM #1** Peak Elev=8.33' Storage=364 cf Inflow=0.31 cfs 1,130 cf  
Discarded=0.05 cfs 1,130 cf Primary=0.00 cfs 0 cf Outflow=0.05 cfs 1,130 cf

**Pond INF-2: INFILTRATIONSYSTEM #2** Peak Elev=9.46' Storage=2,921 cf Inflow=0.93 cfs 6,224 cf  
Discarded=0.11 cfs 6,224 cf Primary=0.00 cfs 0 cf Outflow=0.11 cfs 6,224 cf

**Pond INF3: INFILTRATIONSYSTEM #1** Peak Elev=8.82' Storage=385 cf Inflow=0.31 cfs 1,012 cf  
Discarded=0.04 cfs 1,012 cf Primary=0.00 cfs 0 cf Outflow=0.04 cfs 1,012 cf

**Total Runoff Area = 376,293 sf Runoff Volume = 24,119 cf Average Runoff Depth = 0.77"**  
**39.88% Pervious = 150,053 sf 60.12% Impervious = 226,240 sf**



## 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 37

### Summary for Reach DP-1: French Rodney Blvd 14" Outfall

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =	83,014 sf, 62.20% Impervious,	Inflow Depth = 0.39"	for 1.7-in event
Inflow =	0.83 cfs @ 12.15 hrs,	Volume=	2,711 cf
Outflow =	0.83 cfs @ 12.15 hrs,	Volume=	2,711 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 38

### Summary for Reach DP-2: NORTHERN OUTFALL

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =	293,279 sf, 59.54% Impervious,	Inflow Depth = 0.40"	for 1.7-in event
Inflow =	3.50 cfs @ 12.15 hrs,	Volume=	9,827 cf
Outflow =	3.50 cfs @ 12.15 hrs,	Volume=	9,827 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 41

**Summary for Pond 1-P: BB 1**

Inflow Area = 38,826 sf, 51.66% Impervious, Inflow Depth = 0.63" for 1.7-in event  
 Inflow = 0.71 cfs @ 12.14 hrs, Volume= 2,036 cf  
 Outflow = 0.27 cfs @ 12.33 hrs, Volume= 2,036 cf, Atten= 63%, Lag= 11.6 min  
 Discarded = 0.04 cfs @ 12.33 hrs, Volume= 1,515 cf  
**Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf**  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall  
 Secondary = 0.23 cfs @ 12.33 hrs, Volume= 521 cf  
 Routed to Reach 15R : ISOLATOR ROW 1

**NO OVERFLOW**

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.90' @ 12.33 hrs Surf.Area= 1,388 sf Storage= 630 cf

Plug-Flow detention time= 155.8 min calculated for 2,034 cf (100% of inflow)  
 Center-of-Mass det. time= 155.8 min ( 1,013.2 - 857.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.20'	1,114 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.20	490	0	0
9.50	800	194	194
10.20	1,830	920	1,114

Device	Routing	Invert	Outlet Devices
#1	Primary	8.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Discarded	9.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.00'
#3	Device 1	10.00'	<b>24inch-Dome Grate Capacity X 2.00</b>
#4	Secondary	9.83'	<b>15inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.04 cfs @ 12.33 hrs HW=9.90' (Free Discharge)  
 ↑ **2=Exfiltration** ( Controls 0.04 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=9.20' (Free Discharge)  
 ↑ **1=Culvert** (Passes 0.00 cfs of 2.86 cfs potential flow)  
 ↑ **3=24inch-Dome Grate Capacity** ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.23 cfs @ 12.33 hrs HW=9.90' (Free Discharge)  
 ↑ **4=15inch-Dome Grate Capacity**(Custom Controls 0.23 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 42

**Summary for Pond 2a-P: BB 2a**

Inflow Area = 3,116 sf, 92.62% Impervious, Inflow Depth = 1.29" for 1.7-in event  
 Inflow = 0.11 cfs @ 12.13 hrs, Volume= 334 cf  
 Outflow = 0.10 cfs @ 12.16 hrs, Volume= 298 cf, Atten= 11%, Lag= 2.1 min  
 Primary = 0.10 cfs @ 12.16 hrs, Volume= 298 cf

Routed to Reach 15R : ISOLATOR ROW 1

Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM

ALL OVERFLOW TO  
ISOLATOR ROW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.14' @ 12.16 hrs Surf.Area= 430 sf Storage= 53 cf

Plug-Flow detention time= 84.7 min calculated for 298 cf (89% of inflow)  
 Center-of-Mass det. time= 31.0 min ( 827.7 - 796.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	710 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	320	0	0
9.00	1,100	710	710

Device	Routing	Invert	Outlet Devices
#1	Secondary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	8.50'	<b>24inch-Dome Grate Capacity X 2.00</b>
#3	Primary	8.10'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.10 cfs @ 12.16 hrs HW=8.14' (Free Discharge)  
 ↑3=15inch-Dome Grate Capacity (Custom Controls 0.10 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.00' (Free Discharge)  
 ↑1=Culvert (Passes 0.00 cfs of 2.27 cfs potential flow)  
 ↑2=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 43

**Summary for Pond 2b-P: BB 2b**

Inflow Area = 21,490 sf, 80.50% Impervious, Inflow Depth = 1.04" for 1.7-in event  
 Inflow = 0.65 cfs @ 12.13 hrs, Volume= 1,868 cf  
 Outflow = 0.62 cfs @ 12.15 hrs, Volume= 1,832 cf, Atten= 4%, Lag= 1.2 min  
 Primary = 0.62 cfs @ 12.15 hrs, Volume= 1,832 cf  
     Routed to Reach 15R : ISOLATOR ROW 1  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
     Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM  
  
ALL OVERFLOW TO  
ISOLATOR ROW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.24' @ 12.15 hrs Surf.Area= 505 sf Storage= 98 cf

Plug-Flow detention time= 19.6 min calculated for 1,830 cf (98% of inflow)  
 Center-of-Mass det. time= 8.2 min ( 827.9 - 819.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	710 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	320	0	0
9.00	1,100	710	710

Device	Routing	Invert	Outlet Devices
#1	Secondary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	8.50'	<b>24inch-Dome Grate Capacity X 2.00</b>
#3	Primary	8.10'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.62 cfs @ 12.15 hrs HW=8.24' (Free Discharge)  
 ↑**3=15inch-Dome Grate Capacity**(Custom Controls 0.62 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.00' (Free Discharge)  
 ↑**1=Culvert** (Passes 0.00 cfs of 2.27 cfs potential flow)  
 ↑**2=24inch-Dome Grate Capacity**( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 44

**Summary for Pond 3A-P: BB 3A**

Inflow Area = 10,987 sf, 58.16% Impervious, Inflow Depth = 0.73" for 1.7-in event  
 Inflow = 0.24 cfs @ 12.13 hrs, Volume= 668 cf  
 Outflow = 0.02 cfs @ 13.36 hrs, Volume= 668 cf, Atten= 92%, Lag= 73.4 min  
 Discarded = 0.02 cfs @ 13.36 hrs, Volume= 668 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW

NO OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.89' @ 13.36 hrs Surf.Area= 737 sf Storage= 302 cf

Plug-Flow detention time= 183.8 min calculated for 668 cf (100% of inflow)  
 Center-of-Mass det. time= 183.8 min ( 1,031.3 - 847.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.25'	622 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet) Cum.Store (cubic-feet)
	10.25	271	0 0
	10.45	350	62 62
	11.25	1,050	560 622

Device	Routing	Invert	Outlet Devices
#1	Primary	9.30'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.30' / 9.20' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.55 sf
#2	Discarded	10.25'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.30'
#3	Primary	11.15'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	10.90'	<b>24inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.02 cfs @ 13.36 hrs HW=10.89' (Free Discharge)  
 ↑ **2=Exfiltration** ( Controls 0.02 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=10.25' (Free Discharge)  
 ↑ **1=Culvert** (Passes 0.00 cfs of 1.72 cfs potential flow)  
 ↑ **4=24inch-Dome Grate Capacity** ( Controls 0.00 cfs)  
 ↑ **3=Sharp-Crested Rectangular Weir** ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 45

**Summary for Pond 3B-P: BB 3B**

Inflow Area = 4,545 sf, 77.34% Impervious, Inflow Depth = 1.04" for 1.7-in event  
 Inflow = 0.14 cfs @ 12.13 hrs, Volume= 395 cf  
 Outflow = 0.01 cfs @ 13.11 hrs, Volume= 395 cf, Atten= 91%, Lag= 59.0 min  
 Discarded = 0.01 cfs @ 13.11 hrs, Volume= 395 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routed to Reach BMP\_3 : BMP-3\_OVERFLOW

**NO OVERFLOW**

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 12.74' @ 13.11 hrs Surf.Area= 479 sf Storage= 177 cf

Plug-Flow detention time= 150.1 min calculated for 395 cf (100% of inflow)  
 Center-of-Mass det. time= 150.1 min ( 969.8 - 819.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	12.20'	263 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
12.20	180	0	0
12.90	570	263	263

Device	Routing	Invert	Outlet Devices
#1	Primary	10.70'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.70' / 10.60' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.55 sf
#2	Discarded	12.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 8.70'
#3	Device 1	12.80'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	12.85'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.01 cfs @ 13.11 hrs HW=12.74' (Free Discharge)  
 ↑ **2=Exfiltration** ( Controls 0.01 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=12.20' (Free Discharge)  
 ↑ **1=Culvert** (Passes 0.00 cfs of 2.73 cfs potential flow)  
 ↑ **3=24inch-Dome Grate Capacity** ( Controls 0.00 cfs)  
 ↑ **4=Sharp-Crested Rectangular Weir** ( Controls 0.00 cfs)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 46

## Summary for Pond 4A-P: BB 4A - POND

Inflow Area = 4,843 sf, 86.37% Impervious, Inflow Depth = 1.20" for 1.7-in event  
 Inflow = 0.16 cfs @ 12.13 hrs, Volume= 484 cf  
 Outflow = 0.03 cfs @ 12.58 hrs, Volume= 484 cf, Atten= 82%, Lag= 26.8 min  
**Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf**  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW  
 Secondary = 0.03 cfs @ 12.58 hrs, Volume= 484 cf  
 Routed to Pond 4A-S : BB4A-Stone

**NO OVERFLOW**

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.91' @ 12.58 hrs Surf.Area= 491 sf Storage= 151 cf

Plug-Flow detention time= 40.5 min calculated for 484 cf (100% of inflow)  
 Center-of-Mass det. time= 40.5 min ( 845.9 - 805.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.50'	320 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.50	250	0	0
10.20	664	320	320

Device	Routing	Invert	Outlet Devices
#1	Primary	8.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.00'
#3	Primary	10.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s) <b>24inchDome Grate Capacity</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Device 1	9.95'	

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=9.50' (Free Discharge)

- 1=Culvert (Passes 0.00 cfs of 3.62 cfs potential flow)
- 4=24inchDome Grate Capacity ( Controls 0.00 cfs)
- 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.03 cfs @ 12.58 hrs HW=9.91' (Free Discharge)

- 2=Exfiltration ( Controls 0.03 cfs)



# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 47

## Summary for Pond 4A-S: BB4A-Stone

Inflow = 0.03 cfs @ 12.58 hrs, Volume= 484 cf  
Outflow = 0.03 cfs @ 12.61 hrs, Volume= 484 cf, Atten= 0%, Lag= 2.2 min  
Primary = 0.03 cfs @ 12.61 hrs, Volume= 484 cf  
Routed to Reach BMP4\_O : BMP-4 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Peak Elev= 6.11' @ 12.61 hrs Surf.Area= 230 sf Storage= 8 cf

Plug-Flow detention time= 7.1 min calculated for 484 cf (100% of inflow)  
Center-of-Mass det. time= 7.1 min ( 853.0 - 845.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	138 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 460 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	230	0	0
8.00	230	460	460

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.03 cfs @ 12.61 hrs HW=6.11' (Free Discharge)  
↑1=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.14 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 48

**Summary for Pond 4B-P: BB 4B - POND**

Inflow Area = 3,048 sf, 86.09% Impervious, Inflow Depth = 1.20" for 1.7-in event  
 Inflow = 0.10 cfs @ 12.13 hrs, Volume= 305 cf  
 Outflow = 0.02 cfs @ 12.57 hrs, Volume= 305 cf, Atten= 82%, Lag= 26.6 min  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW  
 Secondary = 0.02 cfs @ 12.57 hrs, Volume= 305 cf  
 Routed to Pond 4B-S : BB 4A-Stone

NO OVERFLOW

FLOW TO STONE RESERVOIR BELOW POND

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.92' @ 12.57 hrs Surf.Area= 312 sf Storage= 96 cf

Plug-Flow detention time= 41.2 min calculated for 305 cf (100% of inflow)  
 Center-of-Mass det. time= 41.2 min ( 846.6 - 805.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.50'	199 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.50	144	0	0
11.20	424	199	199

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.00' / 8.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.00'
#3	Primary	11.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	10.95'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=10.50' (Free Discharge)

- 1=Culvert (Passes 0.00 cfs of 3.62 cfs potential flow)
- 4=24inch-Dome Grate Capacity ( Controls 0.00 cfs)
- 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.02 cfs @ 12.57 hrs HW=10.92' (Free Discharge)

- 2=Exfiltration ( Controls 0.02 cfs)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 49

## Summary for Pond 4B-S: BB 4A-Stone

Inflow = 0.02 cfs @ 12.57 hrs, Volume= 305 cf  
 Outflow = 0.02 cfs @ 12.60 hrs, Volume= 305 cf, Atten= 0%, Lag= 1.7 min  
 Primary = 0.02 cfs @ 12.60 hrs, Volume= 305 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.09' @ 12.60 hrs Surf.Area= 145 sf Storage= 4 cf

Plug-Flow detention time= 5.6 min calculated for 305 cf (100% of inflow)  
 Center-of-Mass det. time= 5.6 min ( 852.1 - 846.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	87 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 290 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	145	0	0
8.00	145	290	290

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.60 hrs HW=6.09' (Free Discharge)  
 ↑1=Orifice/Grate (Orifice Controls 0.02 cfs @ 1.01 fps)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 50

## Summary for Pond 5A-P: BB 5A - POND

Inflow Area = 3,072 sf, 73.44% Impervious, Inflow Depth = 0.97" for 1.7-in event  
 Inflow = 0.09 cfs @ 12.13 hrs, Volume= 249 cf  
 Outflow = 0.03 cfs @ 12.33 hrs, Volume= 249 cf, Atten= 66%, Lag= 12.1 min  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW  
 Secondary = 0.03 cfs @ 12.33 hrs, Volume= 249 cf  
 Routed to Pond 5A-PS : BB 5A-Stone

NO OVERFLOW

FLOW TO STONE RESERVOIR BELOW POND

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.89' @ 12.33 hrs Surf.Area= 510 sf Storage= 45 cf

Plug-Flow detention time= 10.7 min calculated for 249 cf (100% of inflow)  
 Center-of-Mass det. time= 10.7 min ( 836.7 - 826.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.80'	645 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.80	480	0	0
9.80	810	645	645

Device	Routing	Invert	Outlet Devices
#1	Primary	7.60'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.60' / 7.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.80'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.60'
#3	Device 1	9.50'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.80' (Free Discharge)

- ↑1=Culvert (Passes 0.00 cfs of 2.86 cfs potential flow)
- ↑3=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.03 cfs @ 12.33 hrs HW=8.89' (Free Discharge)

- ↑2=Exfiltration ( Controls 0.03 cfs)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 51

## Summary for Pond 5A-PS: BB 5A-Stone

Inflow = 0.03 cfs @ 12.33 hrs, Volume= 249 cf  
 Outflow = 0.03 cfs @ 12.46 hrs, Volume= 249 cf, Atten= 0%, Lag= 7.8 min  
 Primary = 0.03 cfs @ 12.46 hrs, Volume= 249 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.11' @ 12.46 hrs Surf.Area= 480 sf Storage= 16 cf

Plug-Flow detention time= 18.6 min calculated for 249 cf (100% of inflow)  
 Center-of-Mass det. time= 18.7 min ( 855.4 - 836.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	288 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 960 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	480	0	0
8.00	480	960	960

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.03 cfs @ 12.46 hrs HW=6.11' (Free Discharge)  
 ↑1=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.14 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 52

**Summary for Pond 5B-P: BB 5B - POND**

Inflow Area = 34,755 sf, 71.39% Impervious, Inflow Depth = 0.91" for 1.7-in event  
 Inflow = 0.92 cfs @ 12.13 hrs, Volume= 2,623 cf  
 Outflow = 0.81 cfs @ 12.17 hrs, Volume= 2,623 cf, Atten= 12%, Lag= 2.2 min  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW  
 Secondary = 0.07 cfs @ 12.17 hrs, Volume= 1,663 cf  
 Routed to Pond 5B-PS : BB 5B-Stone  
 Tertiary = 0.75 cfs @ 12.17 hrs, Volume= 961 cf  
 Routed to Reach 6R : ISOLATOR ROW 2

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.86' @ 12.17 hrs Surf.Area= 1,064 sf Storage= 457 cf

Plug-Flow detention time= 38.4 min calculated for 2,623 cf (100% of inflow)  
 Center-of-Mass det. time= 38.3 min ( 870.1 - 831.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.20'	889 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.20	327	0	0
9.20	1,450	889	889

Device	Routing	Invert	Outlet Devices
#1	Primary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.20'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.00'
#3	Device 1	8.90'	<b>24inchDome Grate Capacity X 2.00</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Tertiary	8.70'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.20' (Free Discharge)  
 ↑1=Culvert (Passes 0.00 cfs of 2.86 cfs potential flow)  
 ↑3=24inchDome Grate Capacity ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.07 cfs @ 12.17 hrs HW=8.85' (Free Discharge)  
 ↑2=Exfiltration ( Controls 0.07 cfs)

**Tertiary OutFlow** Max=0.72 cfs @ 12.17 hrs HW=8.85' (Free Discharge)  
 ↑4=15inch-Dome Grate Capacity (Custom Controls 0.72 cfs)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 53

## Summary for Pond 5B-PS: BB 5B-Stone

[44] Hint: Outlet device #1 is below defined storage

Inflow = 0.07 cfs @ 12.17 hrs, Volume= 1,663 cf  
 Outflow = 0.07 cfs @ 12.17 hrs, Volume= 1,663 cf, Atten= 0%, Lag= 0.1 min  
 Primary = 0.07 cfs @ 12.17 hrs, Volume= 1,663 cf

Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.00' @ 12.17 hrs Surf.Area= 690 sf Storage= 0 cf

Plug-Flow detention time= 0.1 min calculated for 1,662 cf (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 943.8 - 943.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	414 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 1,380 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	690	0	0
8.00	690	1,380	1,380

Device	Routing	Invert	Outlet Devices
#1	Primary	4.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.57 cfs @ 12.17 hrs HW=6.00' (Free Discharge)  
 ↑**1=Orifice/Grate** (Orifice Controls 0.57 cfs @ 6.52 fps)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 54

## Summary for Pond 6A-P: BB 6A - POND

Inflow Area = 15,148 sf, 46.97% Impervious, Inflow Depth = 0.58" for 1.7-in event  
 Inflow = 0.26 cfs @ 12.14 hrs, Volume= 736 cf  
 Outflow = 0.04 cfs @ 12.87 hrs, Volume= 736 cf, Atten= 86%, Lag= 44.1 min  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach BMP6 O : BMP-6 OVERFLOW  
 Secondary = 0.04 cfs @ 12.87 hrs, Volume= 736 cf  
 Routed to Pond 6A-PS : BB 6A - STONE

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.72' @ 12.87 hrs Surf.Area= 576 sf Storage= 242 cf

Plug-Flow detention time= 60.3 min calculated for 736 cf (100% of inflow)  
 Center-of-Mass det. time= 60.2 min ( 922.4 - 862.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.20'	491 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.20	350	0	0
11.10	740	491	491

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.00' / 8.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.20'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.00'
#3	Device 1	10.80'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	11.00'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=10.20' (Free Discharge)

- ↑ 1=Culvert (Passes 0.00 cfs of 2.86 cfs potential flow)
- ↑ 3=24inch-Dome Grate Capacity ( Controls 0.00 cfs)
- ↑ 4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.87 hrs HW=10.72' (Free Discharge)

- ↑ 2=Exfiltration ( Controls 0.04 cfs)



# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 55

## Summary for Pond 6A-PS: BB 6A - STONE

Inflow = 0.04 cfs @ 12.87 hrs, Volume= 736 cf  
 Outflow = 0.04 cfs @ 12.92 hrs, Volume= 736 cf, Atten= 0%, Lag= 2.7 min  
 Primary = 0.04 cfs @ 12.92 hrs, Volume= 736 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.13' @ 12.92 hrs Surf.Area= 290 sf Storage= 11 cf

Plug-Flow detention time= 6.9 min calculated for 736 cf (100% of inflow)  
 Center-of-Mass det. time= 7.0 min ( 929.4 - 922.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	174 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 580 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	290	0	0
8.00	290	580	580

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.92 hrs HW=6.13' (Free Discharge)  
 ↑1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.21 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 56

**Summary for Pond 6B-P: BB 6B**

Inflow Area = 6,495 sf, 77.45% Impervious, Inflow Depth = 1.04" for 1.7-in event  
 Inflow = 0.20 cfs @ 12.13 hrs, Volume= 564 cf  
 Outflow = 0.02 cfs @ 13.26 hrs, Volume= 564 cf, Atten= 92%, Lag= 68.0 min  
 Discarded = 0.02 cfs @ 13.26 hrs, Volume= 564 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.90' @ 13.26 hrs Surf.Area= 579 sf Storage= 269 cf

Plug-Flow detention time= 194.1 min calculated for 564 cf (100% of inflow)  
 Center-of-Mass det. time= 194.0 min ( 1,013.8 - 819.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	11.20'	394 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
11.20	185	0	0
12.10	690	394	394

Device	Routing	Invert	Outlet Devices
#1	Primary	10.10'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.10' / 10.00' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Discarded	11.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 8.10'
#3	Device 1	11.95'	<b>24inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.02 cfs @ 13.26 hrs HW=11.90' (Free Discharge)  
 ↑**2=Exfiltration** ( Controls 0.02 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=11.20' (Free Discharge)  
 ↑**1=Culvert** (Passes 0.00 cfs of 2.58 cfs potential flow)  
 ↑**3=24inch-Dome Grate Capacity** ( Controls 0.00 cfs)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 57

## Summary for Pond 7A-P: BB 7A PONDING

Inflow Area = 3,165 sf, 87.74% Impervious, Inflow Depth = 1.20" for 1.7-in event  
 Inflow = 0.11 cfs @ 12.13 hrs, Volume= 316 cf  
 Outflow = 0.02 cfs @ 12.61 hrs, Volume= 316 cf, Atten= 84%, Lag= 28.7 min  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach BMP7 O : BMP-7 OVERFLOW  
 Secondary = 0.02 cfs @ 12.61 hrs, Volume= 316 cf  
 Routed to Pond 7A-S : BB 7A - STONE

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.84' @ 12.61 hrs Surf.Area= 281 sf Storage= 108 cf

Plug-Flow detention time= 53.4 min calculated for 316 cf (100% of inflow)  
 Center-of-Mass det. time= 53.4 min ( 858.7 - 805.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.30'	227 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.30	115	0	0
10.20	390	227	227

Device	Routing	Invert	Outlet Devices
#1	Primary	8.10'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.10' / 8.00' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.30'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.10'
#3	Device 1	9.90'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	10.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=9.30' (Free Discharge)

- ↑1=Culvert (Passes 0.00 cfs of 2.86 cfs potential flow)
- ↑3=24inch-Dome Grate Capacity ( Controls 0.00 cfs)
- ↑4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.02 cfs @ 12.61 hrs HW=9.84' (Free Discharge)

- ↑2=Exfiltration ( Controls 0.02 cfs)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 58

## Summary for Pond 7A-S: BB 7A - STONE

Inflow = 0.02 cfs @ 12.61 hrs, Volume= 316 cf  
 Outflow = 0.02 cfs @ 12.64 hrs, Volume= 316 cf, Atten= 0%, Lag= 2.0 min  
 Primary = 0.02 cfs @ 12.64 hrs, Volume= 316 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5.18' @ 12.64 hrs Surf.Area= 150 sf Storage= 4 cf

Plug-Flow detention time= 5.7 min calculated for 316 cf (100% of inflow)  
 Center-of-Mass det. time= 5.7 min ( 864.5 - 858.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	5.10'	90 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 300 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
5.10	150	0	0
7.10	150	300	300

Device	Routing	Invert	Outlet Devices
#1	Primary	5.10'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.64 hrs HW=5.18' (Free Discharge)  
 ↑1=Orifice/Grate (Orifice Controls 0.02 cfs @ 0.99 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 59

**Summary for Pond 7B-P: BB 7B PONDING**

Inflow Area = 4,942 sf, 88.73% Impervious, Inflow Depth = 1.20" for 1.7-in event  
 Inflow = 0.17 cfs @ 12.13 hrs, Volume= 494 cf  
 Outflow = 0.03 cfs @ 12.61 hrs, Volume= 494 cf, Atten= 84%, Lag= 28.9 min  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach BMP7 O : BMP-7 OVERFLOW  
 Secondary = 0.03 cfs @ 12.61 hrs, Volume= 494 cf  
 Routed to Pond 7B-S : BB 7B - STONE

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.47' @ 12.61 hrs Surf.Area= 434 sf Storage= 162 cf

Plug-Flow detention time= 48.4 min calculated for 493 cf (100% of inflow)  
 Center-of-Mass det. time= 48.4 min ( 853.7 - 805.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	324 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	250	0	0
10.80	560	324	324

Device	Routing	Invert	Outlet Devices
#1	Primary	8.90'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.90' / 8.80' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.00'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.90'
#3	Device 1	10.60'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=10.00' (Free Discharge)

- ↑1=Culvert (Passes 0.00 cfs of 2.58 cfs potential flow)
- ↑3=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.03 cfs @ 12.61 hrs HW=10.47' (Free Discharge)

- ↑2=Exfiltration ( Controls 0.03 cfs)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 60

## Summary for Pond 7B-S: BB 7B - STONE

Inflow = 0.03 cfs @ 12.61 hrs, Volume= 494 cf  
 Outflow = 0.03 cfs @ 12.64 hrs, Volume= 494 cf, Atten= 0%, Lag= 1.7 min  
 Primary = 0.03 cfs @ 12.64 hrs, Volume= 494 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5.21' @ 12.64 hrs Surf.Area= 150 sf Storage= 5 cf

Plug-Flow detention time= 4.6 min calculated for 493 cf (100% of inflow)  
 Center-of-Mass det. time= 4.7 min ( 858.4 - 853.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	5.10'	90 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 300 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
5.10	150	0	0
7.10	150	300	300

Device	Routing	Invert	Outlet Devices
#1	Primary	5.10'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.03 cfs @ 12.64 hrs HW=5.21' (Free Discharge)  
 ↑1=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.11 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 61

**Summary for Pond 8a-P: BB 8A PONDING**

Inflow Area = 3,978 sf, 79.99% Impervious, Inflow Depth = 1.04" for 1.7-in event  
 Inflow = 0.12 cfs @ 12.13 hrs, Volume= 346 cf  
 Outflow = 0.03 cfs @ 12.49 hrs, Volume= 346 cf, Atten= 78%, Lag= 21.9 min  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE  
 Secondary = 0.03 cfs @ 12.49 hrs, Volume= 346 cf  
 Routed to Pond 8a-s : BB 8A - STONE

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.71' @ 12.49 hrs Surf.Area= 452 sf Storage= 87 cf

Plug-Flow detention time= 22.8 min calculated for 345 cf (100% of inflow)  
 Center-of-Mass det. time= 22.8 min ( 842.6 - 819.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.50'	575 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.50	360	0	0
9.50	790	575	575

Device	Routing	Invert	Outlet Devices
#1	Primary	7.40'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.40' / 7.30' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.40'
#3	Device 1	9.00'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	9.40'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.50' (Free Discharge)

- ↑1=Culvert (Passes 0.00 cfs of 2.58 cfs potential flow)
- ↑3=24inch-Dome Grate Capacity ( Controls 0.00 cfs)
- ↑4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.03 cfs @ 12.49 hrs HW=8.71' (Free Discharge)

- ↑2=Exfiltration ( Controls 0.03 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 62

**Summary for Pond 8a-s: BB 8A - STONE**

Inflow = 0.03 cfs @ 12.49 hrs, Volume= 346 cf  
Outflow = 0.03 cfs @ 12.55 hrs, Volume= 346 cf, Atten= 0%, Lag= 3.5 min  
Primary = 0.03 cfs @ 12.55 hrs, Volume= 346 cf

Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Peak Elev= 4.51' @ 12.55 hrs Surf.Area= 300 sf Storage= 10 cf

Plug-Flow detention time= 10.3 min calculated for 345 cf (100% of inflow)  
Center-of-Mass det. time= 10.3 min ( 852.9 - 842.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	4.40'	180 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 600 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
4.40	300	0	0
6.40	300	600	600

Device	Routing	Invert	Outlet Devices
#1	Primary	4.40'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.03 cfs @ 12.55 hrs HW=4.51' (Free Discharge)  
↑**1=Orifice/Grate** (Orifice Controls 0.03 cfs @ 1.11 fps)



# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 63

## Summary for Pond 8B-P: BB 8B-PONDING

Inflow Area = 5,598 sf, 87.78% Impervious, Inflow Depth = 1.20" for 1.7-in event  
 Inflow = 0.19 cfs @ 12.13 hrs, Volume= 559 cf  
 Outflow = 0.03 cfs @ 12.58 hrs, Volume= 559 cf, Atten= 82%, Lag= 27.2 min  
**Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf**  
 Routed to Reach H ST : HUDSON STREET DRAINAGE  
 Secondary = 0.03 cfs @ 12.58 hrs, Volume= 559 cf  
 Routed to Pond 8B-S : BB 8B-Stone

**NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM**

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.60' @ 12.58 hrs Surf.Area= 547 sf Storage= 186 cf

Plug-Flow detention time= 48.0 min calculated for 559 cf (100% of inflow)  
 Center-of-Mass det. time= 48.0 min ( 853.4 - 805.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.10'	306 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.10	190	0	0
9.80	685	306	306

Device	Routing	Invert	Outlet Devices
#1	Primary	7.90'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.90' / 7.80' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.10'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.90'
#3	Device 1	9.65'	<b>24inch-Dome Grate Capacity X 2.00</b>

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=9.10' (Free Discharge)

- ↑1=Culvert (Passes 0.00 cfs of 2.86 cfs potential flow)
- ↑3=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.03 cfs @ 12.58 hrs HW=9.60' (Free Discharge)

- ↑2=Exfiltration ( Controls 0.03 cfs)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 64

## Summary for Pond 8B-S: BB 8B-Stone

Inflow = 0.03 cfs @ 12.58 hrs, Volume= 559 cf  
 Outflow = 0.03 cfs @ 12.63 hrs, Volume= 559 cf, Atten= 0%, Lag= 2.8 min  
 Primary = 0.03 cfs @ 12.63 hrs, Volume= 559 cf  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 4.52' @ 12.63 hrs Surf.Area= 300 sf Storage= 11 cf

Plug-Flow detention time= 8.8 min calculated for 559 cf (100% of inflow)  
 Center-of-Mass det. time= 8.8 min ( 862.2 - 853.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	4.40'	180 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 600 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
4.40	300	0	0
6.40	300	600	600

Device	Routing	Invert	Outlet Devices
#1	Primary	4.40'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.03 cfs @ 12.63 hrs HW=4.52' (Free Discharge)  
 ↑1=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.18 fps)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 65

## Summary for Pond 9-P: BB9 - POND

Inflow Area = 29,651 sf, 74.77% Impervious, Inflow Depth = 0.97" for 1.7-in event  
 Inflow = 0.84 cfs @ 12.13 hrs, Volume= 2,402 cf  
 Outflow = 0.81 cfs @ 12.15 hrs, Volume= 2,402 cf, Atten= 4%, Lag= 1.2 min  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach BMP9 O : BMP-9 OVERFLOW  
 Secondary = 0.04 cfs @ 12.15 hrs, Volume= 1,207 cf  
 Routed to Pond 9-PS : BB9 - STONE  
 Tertiary = 0.77 cfs @ 12.15 hrs, Volume= 1,195 cf  
 Routed to Reach 1R : ISOLATOR ROW C

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.66' @ 12.15 hrs Surf.Area= 579 sf Storage= 253 cf

Plug-Flow detention time= 34.9 min calculated for 2,400 cf (100% of inflow)  
 Center-of-Mass det. time= 34.9 min ( 860.9 - 826.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	485 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	190	0	0
9.00	780	485	485

Device	Routing	Invert	Outlet Devices
#1	Primary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.00'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.00'
#3	Device 1	8.80'	<b>24inchDome Grate Capacity</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Tertiary	8.50'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.00' (Free Discharge)  
 ↑1=Culvert (Passes 0.00 cfs of 2.27 cfs potential flow)  
 ↑3=24inchDome Grate Capacity ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.15 hrs HW=8.66' (Free Discharge)  
 ↑2=Exfiltration ( Controls 0.04 cfs)

**Tertiary OutFlow** Max=0.77 cfs @ 12.15 hrs HW=8.66' (Free Discharge)  
 ↑4=15inch-Dome Grate Capacity (Custom Controls 0.77 cfs)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 66

## Summary for Pond 9-PS: BB9 - STONE

Inflow = 0.04 cfs @ 12.15 hrs, Volume= 1,207 cf  
 Outflow = 0.04 cfs @ 12.18 hrs, Volume= 1,207 cf, Atten= 1%, Lag= 1.7 min  
 Primary = 0.04 cfs @ 12.18 hrs, Volume= 1,207 cf  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.13' @ 12.18 hrs Surf.Area= 190 sf Storage= 7 cf

Plug-Flow detention time= 4.2 min calculated for 1,206 cf (100% of inflow)  
 Center-of-Mass det. time= 4.2 min ( 980.7 - 976.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	114 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 380 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	190	0	0
8.00	190	380	380

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.18 hrs HW=6.13' (Free Discharge)  
 ↑1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.21 fps)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 67

## Summary for Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST

[57] Hint: Peaked at 9.36' (Flood elevation advised)

Inflow Area = 19,582 sf, 58.17% Impervious, Inflow Depth = 0.73" for 1.7-in event  
Inflow = 0.42 cfs @ 12.13 hrs, Volume= 1,191 cf  
Outflow = 0.42 cfs @ 12.13 hrs, Volume= 1,191 cf, Atten= 0%, Lag= 0.0 min  
Primary = 0.31 cfs @ 12.13 hrs, Volume= 1,130 cf  
Routed to Pond INF-1 : INFILTRATION SYSTEM #1  
Secondary = 0.11 cfs @ 12.13 hrs, Volume= 61 cf  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Peak Elev= 9.36' @ 12.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>6.0" Vert. WATER QUALITY STORM DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	9.20'	<b>12.0" Vert. LARGE STORM OVEFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.30 cfs @ 12.13 hrs HW=9.35' (Free Discharge)  
↑1=**WATER QUALITY STORM DIVERSION**(Orifice Controls 0.30 cfs @ 2.02 fps)

**Secondary OutFlow** Max=0.10 cfs @ 12.13 hrs HW=9.35' (Free Discharge)  
↑2=**LARGE STORM OVEFLOW**(Orifice Controls 0.10 cfs @ 1.34 fps)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 68

## Summary for Pond DMH2: DIVERSION MANHOLE - HUDSON STREET

[57] Hint: Peaked at 11.81' (Flood elevation advised)

Inflow Area = 143,309 sf, 49.69% Impervious, Inflow Depth = 0.63" for 1.7-in event  
 Inflow = 2.62 cfs @ 12.14 hrs, Volume= 7,514 cf  
 Outflow = 2.62 cfs @ 12.14 hrs, Volume= 7,514 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 0.93 cfs @ 12.14 hrs, Volume= 6,224 cf  
 Routed to Pond INF-2 : INFILTRATION SYSTEM #2  
 Secondary = 1.70 cfs @ 12.14 hrs, Volume= 1,290 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.81' @ 12.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	10.60'	<b>6.0" Vert. WATER QUALITY STORM DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	11.10'	<b>12.0" Vert. LARGE STORM OVERFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.91 cfs @ 12.14 hrs HW=11.78' (Free Discharge)  
 ↖1=**WATER QUALITY STORM DIVERSION**(Orifice Controls 0.91 cfs @ 4.65 fps)

**Secondary OutFlow** Max=1.61 cfs @ 12.14 hrs HW=11.78' (Free Discharge)  
 ↖2=**LARGE STORM OVERFLOW**(Orifice Controls 1.61 cfs @ 2.82 fps)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 69

## Summary for Pond DMH3: DIVERSION MANHOLE - PORTLAND ST

[57] Hint: Peaked at 11.31' (Flood elevation advised)

Inflow Area = 19,743 sf, 50.83% Impervious, Inflow Depth = 0.63" for 1.7-in event  
 Inflow = 0.36 cfs @ 12.14 hrs, Volume= 1,035 cf  
 Outflow = 0.36 cfs @ 12.14 hrs, Volume= 1,035 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 0.31 cfs @ 12.14 hrs, Volume= 1,012 cf  
 Routed to Pond INF3 : INFILTRATION SYSTEM #1  
 Secondary = 0.05 cfs @ 12.14 hrs, Volume= 23 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.31' @ 12.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	11.00'	<b>8.0" Vert. WATER QUALITY DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	11.20'	<b>10.0" Vert. LARGE STORM OVERFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.30 cfs @ 12.14 hrs HW=11.31' (Free Discharge)  
 ↳1=**WATER QUALITY DIVERSION**(Orifice Controls 0.30 cfs @ 1.89 fps)

**Secondary OutFlow** Max=0.05 cfs @ 12.14 hrs HW=11.31' (Free Discharge)  
 ↳2=**LARGE STORM OVERFLOW**(Orifice Controls 0.05 cfs @ 1.13 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 70

**Summary for Pond INF-1: INFILTRATION SYSTEM #1**

Inflow Area = 19,582 sf, 58.17% Impervious, Inflow Depth = 0.69" for 1.7-in event  
 Inflow = 0.31 cfs @ 12.13 hrs, Volume= 1,130 cf  
 Outflow = 0.05 cfs @ 12.92 hrs, Volume= 1,130 cf, Atten= 83%, Lag= 46.9 min  
 Discarded = 0.05 cfs @ 12.92 hrs, Volume= 1,130 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.33' @ 12.92 hrs Surf.Area= 1,772 sf Storage= 364 cf

Plug-Flow detention time= 60.0 min calculated for 1,130 cf (100% of inflow)  
 Center-of-Mass det. time= 59.9 min ( 913.9 - 854.0 )

Volume	Invert	Avail.Storage	Storage Description
#1A	7.80'	1,091 cf	<b>21.50'W x 81.52'L x 2.33'H Field A</b> 4,090 cf Overall - 973 cf Embedded = 3,117 cf x 35.0% Voids
#2A	8.30'	973 cf	<b>ADS_StormTech SC-310 +Cap</b> x 66 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 66 Chambers in 6 Rows
#3	7.80'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		2,201 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	7.80'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.80'
#2	Primary	8.10'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.10' / 8.00' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.40'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.05 cfs @ 12.92 hrs HW=8.33' (Free Discharge)  
 ↑1=Exfiltration ( Controls 0.05 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=7.80' (Free Discharge)  
 ↑2=Culvert ( Controls 0.00 cfs)  
 ↑3=Sharp-Crested Rectangular Weir( Controls 0.00 cfs)



# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 71

## Pond INF-1: INFILTRATION SYSTEM #1 - Chamber Wizard Field A

**Chamber Model = ADS\_StormTechSC-310 +Cap (ADS StormTech®SC-310 with cap length)**

Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf

Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap

34.0" Wide + 6.0" Spacing = 40.0" C-C Row Spacing

11 Chambers/Row x 7.12' Long +0.60' Cap Length x 2 = 79.52' Row Length +12.0" End Stone x 2 = 81.52' Base Length

6 Rows x 34.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 21.50' Base Width

6.0" Stone Base + 16.0" Chamber Height + 6.0" Stone Cover = 2.33' Field Height

66 Chambers x 14.7 cf = 973.0 cf Chamber Storage

4,089.6 cf Field - 973.0 cf Chambers = 3,116.6 cf Stone x 35.0% Voids = 1,090.8 cf Stone Storage

Chamber Storage + Stone Storage = 2,063.8 cf = 0.047 af

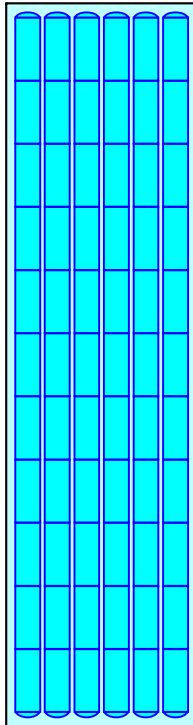
Overall Storage Efficiency = 50.5%

Overall System Size = 81.52' x 21.50' x 2.33'

66 Chambers

151.5 cy Field

115.4 cy Stone



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 72

**Summary for Pond INF-2: INFILTRATION SYSTEM #2**

Inflow Area = 143,309 sf, 49.69% Impervious, Inflow Depth = 0.52" for 1.7-in event  
 Inflow = 0.93 cfs @ 12.14 hrs, Volume= 6,224 cf  
 Outflow = 0.11 cfs @ 15.09 hrs, Volume= 6,224 cf, Atten= 89%, Lag= 177.2 min  
 Discarded = 0.11 cfs @ 15.09 hrs, Volume= 6,224 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.46' @ 15.09 hrs Surf.Area= 2,268 sf Storage= 2,921 cf

Plug-Flow detention time= 338.9 min calculated for 6,220 cf (100% of inflow)  
 Center-of-Mass det. time= 339.0 min ( 1,222.8 - 883.8 )

Volume	Invert	Avail.Storage	Storage Description
#1A	7.50'	1,790 cf	<b>25.25'W x 89.06'L x 3.50'H Field A</b> 7,870 cf Overall - 2,756 cf Embedded = 5,114 cf x 35.0% Voids
#2A	8.00'	2,756 cf	<b>ADS_StormTech SC-740 +Cap x 60 Inside #1</b> Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 60 Chambers in 5 Rows
#3	7.50'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		4,684 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	7.50'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.50'
#2	Primary	8.00'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.50'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.11 cfs @ 15.09 hrs HW=9.46' (Free Discharge)  
 ↑1=Exfiltration ( Controls 0.11 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=7.50' (Free Discharge)  
 ↑2=Culvert ( Controls 0.00 cfs)  
 ↑3=Sharp-Crested Rectangular Weir( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 73

**Pond INF-2: INFILTRATION SYSTEM #2 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-740 +Cap (ADS StormTech®SC-740 with cap length)**

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf

Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

12 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 87.06' Row Length +12.0" End Stone x 2 = 89.06' Base Length

5 Rows x 51.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.25' Base Width

6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

60 Chambers x 45.9 cf = 2,756.4 cf Chamber Storage

7,870.4 cf Field - 2,756.4 cf Chambers = 5,114.0 cf Stone x 35.0% Voids = 1,789.9 cf Stone Storage

Chamber Storage + Stone Storage = 4,546.3 cf = 0.104 af

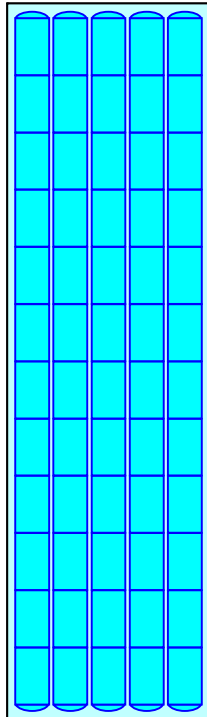
Overall Storage Efficiency = 57.8%

Overall System Size = 89.06' x 25.25' x 3.50'

60 Chambers

291.5 cy Field

189.4 cy Stone



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 74

**Summary for Pond INF3: INFILTRATION SYSTEM #1**

Inflow Area = 19,743 sf, 50.83% Impervious, Inflow Depth = 0.62" for 1.7-in event  
 Inflow = 0.31 cfs @ 12.14 hrs, Volume= 1,012 cf  
 Outflow = 0.04 cfs @ 13.22 hrs, Volume= 1,012 cf, Atten= 88%, Lag= 64.8 min  
 Discarded = 0.04 cfs @ 13.22 hrs, Volume= 1,012 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

**NO OVERFLOW TO  
CLOSE DRAINAGE  
SYSTEM**

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.82' @ 13.22 hrs Surf.Area= 1,113 sf Storage= 385 cf

Plug-Flow detention time= 100.5 min calculated for 1,011 cf (100% of inflow)  
 Center-of-Mass det. time= 100.5 min ( 960.8 - 860.3 )

Volume	Invert	Avail.Storage	Storage Description
#1A	8.10'	686 cf	<b>18.17'W x 60.16'L x 2.33'H Field A</b> 2,550 cf Overall - 590 cf Embedded = 1,960 cf x 35.0% Voids
#2A	8.60'	590 cf	<b>ADS_StormTech SC-310 +Cap x 40 Inside #1</b> Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 40 Chambers in 5 Rows
#3	8.10'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		1,413 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	8.10'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.10'
#2	Primary	8.40'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.40' / 8.30' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.50'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.04 cfs @ 13.22 hrs HW=8.82' (Free Discharge)  
 ↑1=Exfiltration ( Controls 0.04 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.10' (Free Discharge)  
 ↑2=Culvert ( Controls 0.00 cfs)  
 ↑3=Sharp-Crested Rectangular Weir( Controls 0.00 cfs)

# 14850\_Proposed-Drainage-Areas

NOAA 24-hr C 1.7-in Rainfall=1.70"

Prepared by {enter your company name here}

Printed 4/5/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 75

## Pond INF3: INFILTRATION SYSTEM #1 - Chamber Wizard Field A

**Chamber Model = ADS\_StormTechSC-310 +Cap (ADS StormTech®SC-310 with cap length)**

Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf

Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap

34.0" Wide + 6.0" Spacing = 40.0" C-C Row Spacing

8 Chambers/Row x 7.12' Long +0.60' Cap Length x 2 = 58.16' Row Length +12.0" End Stone x 2 = 60.16' Base Length

5 Rows x 34.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 18.17' Base Width

6.0" Stone Base + 16.0" Chamber Height + 6.0" Stone Cover = 2.33' Field Height

40 Chambers x 14.7 cf = 589.7 cf Chamber Storage

2,550.1 cf Field - 589.7 cf Chambers = 1,960.4 cf Stone x 35.0% Voids = 686.2 cf Stone Storage

Chamber Storage + Stone Storage = 1,275.8 cf = 0.029 af

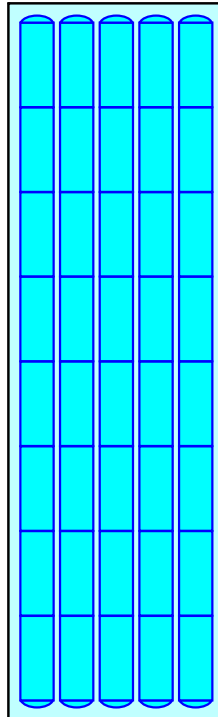
Overall Storage Efficiency = 50.0%

Overall System Size = 60.16' x 18.17' x 2.33'

40 Chambers

94.4 cy Field

72.6 cy Stone

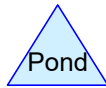
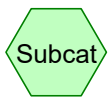
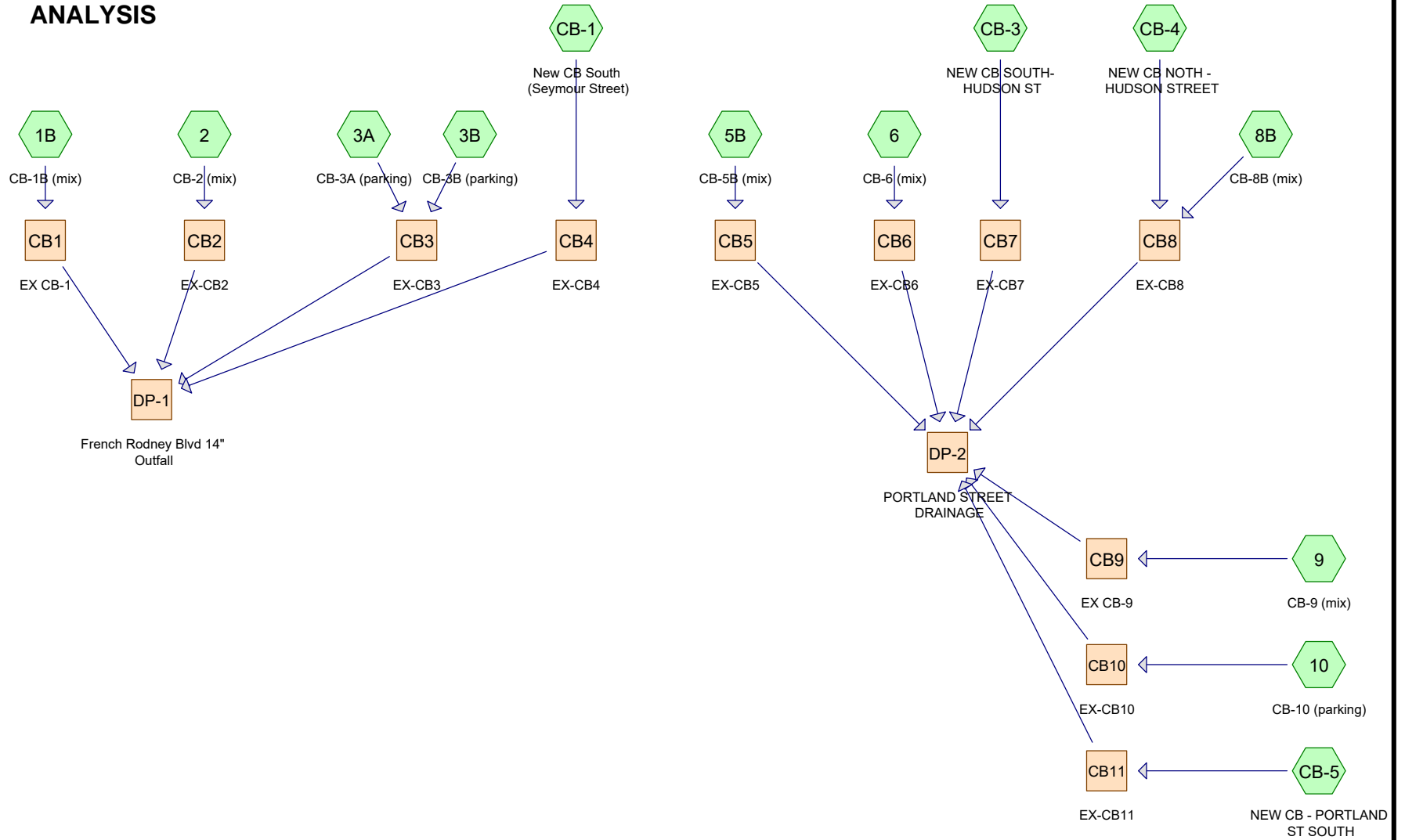


## **APPENDIX C**

---

### **Pre-Development Conditions – HydroCAD Calculations**

# EXISTING HYDROCAD ANALYSIS



**Routing Diagram for 14850\_Existing-Drainage-Areas**  
 Prepared by {enter your company name here}, Printed 4/6/2022  
 HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

### 14850\_Existing-Drainage-Areas

Prepared by {enter your company name here}

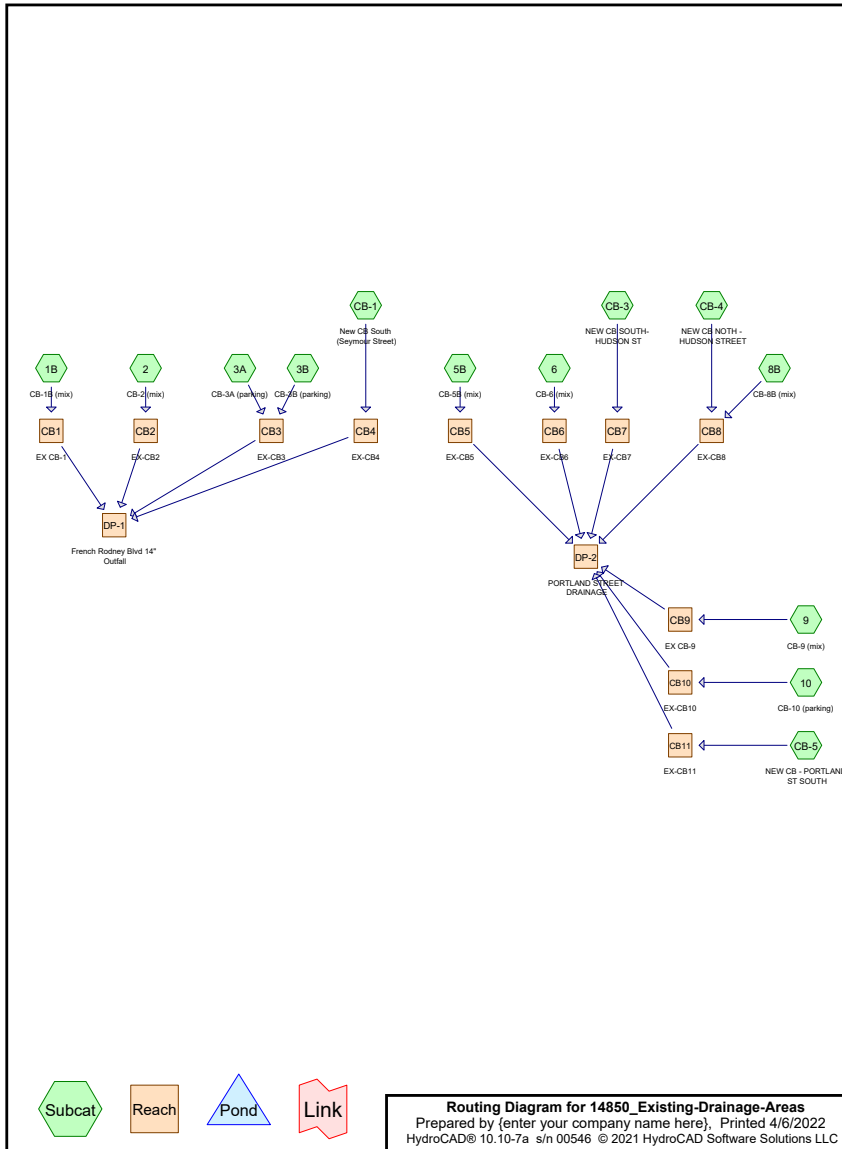
HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Printed 4/6/2022

Page 2

#### Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	NOAA 10-yr	NOAA 24-hr	C	Default	24.00	1	5.02	2
2	NOAA 100-yr	NOAA 24-hr	C	Default	24.00	1	7.59	2
3	NOAA 2-yr	NOAA 24-hr	C	Default	24.00	1	3.40	2
4	NOAA 25-yr	NOAA 24-hr	C	Default	24.00	1	6.04	2





**14850\_Existing-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Printed 4/6/2022

Page 3

**Area Listing (all nodes)**

Area (acres)	CN	Description (subcatchment-numbers)
5.059	83	1/4 acre lots, 38% imp, HSG C (1B, 2, 5B, 6, 8B, 9, CB-1, CB-3, CB-4, CB-5)
2.771	98	Paved parking, HSG C (1B, 2, 3A, 3B, 5B, 6, 8B, 9, 10)
0.861	98	Roadway (CB-1, CB-3, CB-4, CB-5)
<b>8.690</b>	<b>89</b>	<b>TOTAL AREA</b>

**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Printed 4/6/2022

Page 4

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1B: CB-1B (mix)</b>	Runoff Area=33,097 sf 61.34% Impervious Runoff Depth=3.79" Tc=6.0 min CN=89 Runoff=3.50 cfs 0.240 af
<b>Subcatchment2: CB-2 (mix)</b>	Runoff Area=22,628 sf 71.14% Impervious Runoff Depth=4.00" Tc=6.0 min CN=91 Runoff=2.48 cfs 0.173 af
<b>Subcatchment3A: CB-3A (parking)</b>	Runoff Area=7,758 sf 100.00% Impervious Runoff Depth=4.78" Tc=6.0 min CN=98 Runoff=0.92 cfs 0.071 af
<b>Subcatchment3B: CB-3B (parking)</b>	Runoff Area=3,797 sf 100.00% Impervious Runoff Depth=4.78" Tc=6.0 min CN=98 Runoff=0.45 cfs 0.035 af
<b>Subcatchment5B: CB-5B (mix)</b>	Runoff Area=22,974 sf 57.51% Impervious Runoff Depth=3.69" Tc=6.0 min CN=88 Runoff=2.38 cfs 0.162 af
<b>Subcatchment6: CB-6 (mix)</b>	Runoff Area=34,524 sf 93.00% Impervious Runoff Depth=4.55" Tc=6.0 min CN=96 Runoff=4.05 cfs 0.301 af
<b>Subcatchment8B: CB-8B (mix)</b>	Runoff Area=28,528 sf 54.52% Impervious Runoff Depth=3.59" Tc=6.0 min CN=87 Runoff=2.89 cfs 0.196 af
<b>Subcatchment9: CB-9 (mix)</b>	Runoff Area=36,104 sf 94.03% Impervious Runoff Depth=4.67" Tc=6.0 min CN=97 Runoff=4.27 cfs 0.322 af
<b>Subcatchment10: CB-10 (parking)</b>	Runoff Area=6,492 sf 100.00% Impervious Runoff Depth=4.78" Tc=6.0 min CN=98 Runoff=0.77 cfs 0.059 af
<b>SubcatchmentCB-1: New CB South</b>	Runoff Area=19,582 sf 58.17% Impervious Runoff Depth=3.69" Flow Length=512' Tc=6.0 min CN=88 Runoff=2.03 cfs 0.138 af
<b>SubcatchmentCB-3: NEW CB SOUTH-</b>	Runoff Area=25,183 sf 51.84% Impervious Runoff Depth=3.49" Flow Length=635' Tc=6.0 min CN=86 Runoff=2.49 cfs 0.168 af
<b>SubcatchmentCB-4: NEW CB NOTH -</b>	Runoff Area=118,126 sf 49.24% Impervious Runoff Depth=3.49" Flow Length=822' Tc=6.0 min CN=86 Runoff=11.70 cfs 0.788 af
<b>SubcatchmentCB-5: NEW CB -</b>	Runoff Area=19,743 sf 50.83% Impervious Runoff Depth=3.49" Flow Length=574' Tc=6.0 min CN=86 Runoff=1.96 cfs 0.132 af
<b>Reach CB1: EX CB-1</b>	Inflow=3.50 cfs 0.240 af Outflow=3.50 cfs 0.240 af
<b>Reach CB10: EX-CB10</b>	Inflow=0.77 cfs 0.059 af Outflow=0.77 cfs 0.059 af
<b>Reach CB11: EX-CB11</b>	Inflow=1.96 cfs 0.132 af Outflow=1.96 cfs 0.132 af

**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 5

<b>Reach CB2: EX-CB2</b>	Inflow=2.48 cfs 0.173 af Outflow=2.48 cfs 0.173 af
<b>Reach CB3: EX-CB3</b>	Inflow=1.37 cfs 0.106 af Outflow=1.37 cfs 0.106 af
<b>Reach CB4: EX-CB4</b>	Inflow=2.03 cfs 0.138 af Outflow=2.03 cfs 0.138 af
<b>Reach CB5: EX-CB5</b>	Inflow=2.38 cfs 0.162 af Outflow=2.38 cfs 0.162 af
<b>Reach CB6: EX-CB6</b>	Inflow=4.05 cfs 0.301 af Outflow=4.05 cfs 0.301 af
<b>Reach CB7: EX-CB7</b>	Inflow=2.49 cfs 0.168 af Outflow=2.49 cfs 0.168 af
<b>Reach CB8: EX-CB8</b>	Inflow=14.59 cfs 0.983 af Outflow=14.59 cfs 0.983 af
<b>Reach CB9: EX CB-9</b>	Inflow=4.27 cfs 0.322 af Outflow=4.27 cfs 0.322 af
<b>Reach DP-1: French Rodney Blvd 14" Outfall</b>	Inflow=9.38 cfs 0.657 af Outflow=9.38 cfs 0.657 af
<b>Reach DP-2: PORTLANDSTREET DRAINAGE</b>	Inflow=30.51 cfs 2.128 af Outflow=30.51 cfs 2.128 af

Total Runoff Area = 8.690 ac Runoff Volume = 2.785 af Average Runoff Depth = 3.85"  
36.09% Pervious = 3.136 ac 63.91% Impervious = 5.554 ac

**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 6

**Summary for Subcatchment 1B: CB-1B (mix)**

Runoff = 3.50 cfs @ 12.13 hrs, Volume= 0.240 af, Depth= 3.79"  
Routed to Reach CB1 : EX CB-1

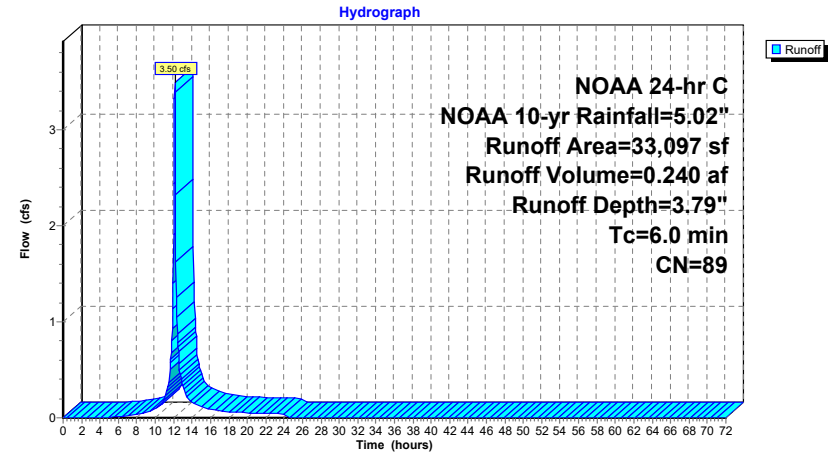
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
20,636	83	1/4 acre lots, 38% imp, HSG C
12,461	98	Paved parking, HSG C
33,097	89	Weighted Average
12,794		38.66% Pervious Area
20,303		61.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 1B: CB-1B (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 7

**Summary for Subcatchment 2: CB-2 (mix)**

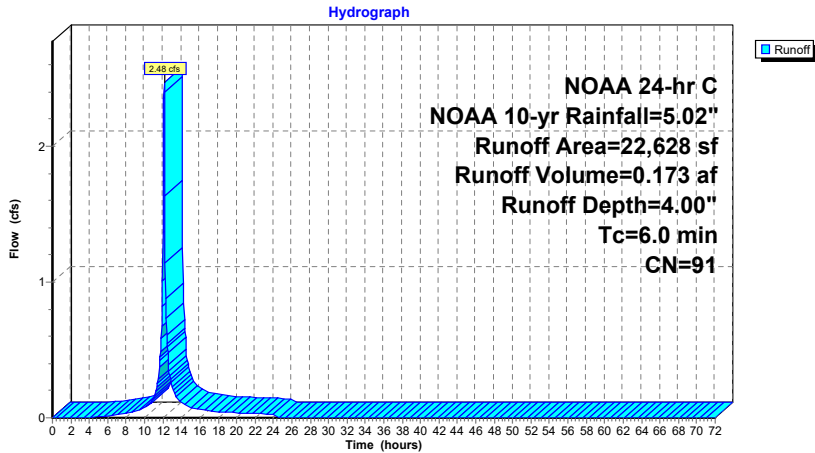
Runoff = 2.48 cfs @ 12.13 hrs, Volume= 0.173 af, Depth= 4.00"  
Routed to Reach CB2 : EX-CB2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
10,533	83	1/4 acre lots, 38% imp, HSG C
12,095	98	Paved parking, HSG C
22,628	91	Weighted Average
6,530		28.86% Pervious Area
16,098		71.14% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2: CB-2 (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 8

**Summary for Subcatchment 3A: CB-3A (parking)**

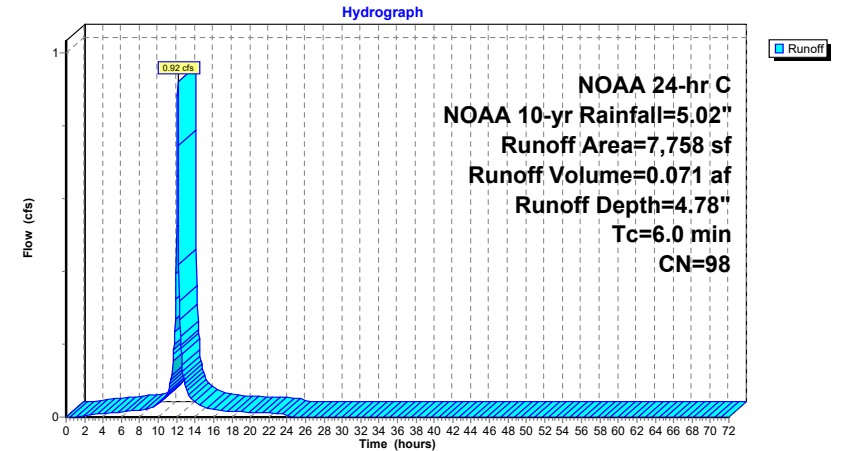
Runoff = 0.92 cfs @ 12.13 hrs, Volume= 0.071 af, Depth= 4.78"  
Routed to Reach CB3 : EX-CB3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
7,758	98	Paved parking, HSG C
7,758		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3A: CB-3A (parking)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 9

**Summary for Subcatchment 3B: CB-3B (parking)**

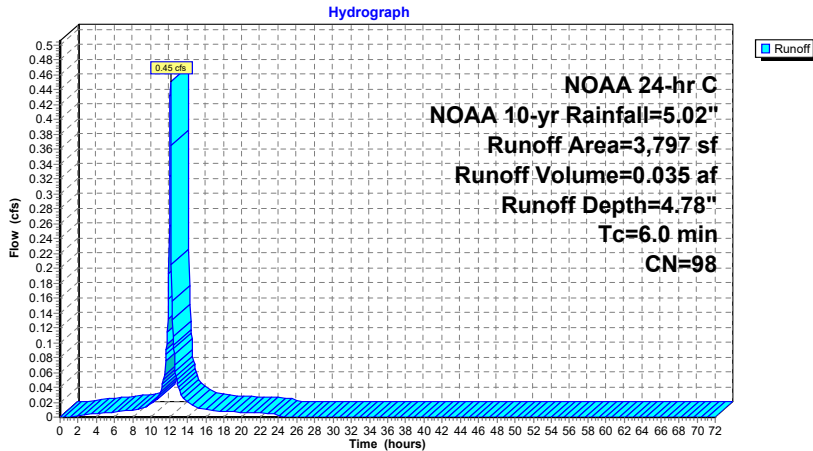
Runoff = 0.45 cfs @ 12.13 hrs, Volume= 0.035 af, Depth= 4.78"  
 Routed to Reach CB3 : EX-CB3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
3,797	98	Paved parking, HSG C
3,797		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3B: CB-3B (parking)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 10

**Summary for Subcatchment 5B: CB-5B (mix)**

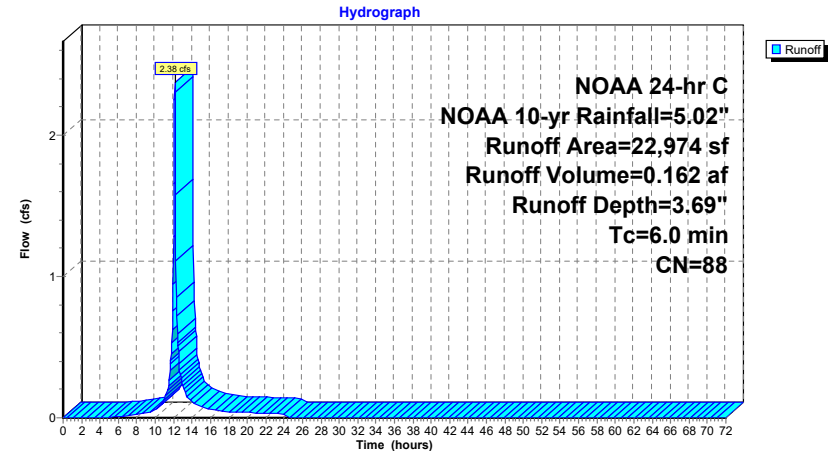
Runoff = 2.38 cfs @ 12.13 hrs, Volume= 0.162 af, Depth= 3.69"  
 Routed to Reach CB5 : EX-CB5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
15,743	83	1/4 acre lots, 38% imp, HSG C
7,231	98	Paved parking, HSG C
22,974	88	Weighted Average
9,761		42.49% Pervious Area
13,213		57.51% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5B: CB-5B (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 11

**Summary for Subcatchment 6: CB-6 (mix)**

Runoff = 4.05 cfs @ 12.13 hrs, Volume= 0.301 af, Depth= 4.55"  
Routed to Reach CB6 : EX-CB6

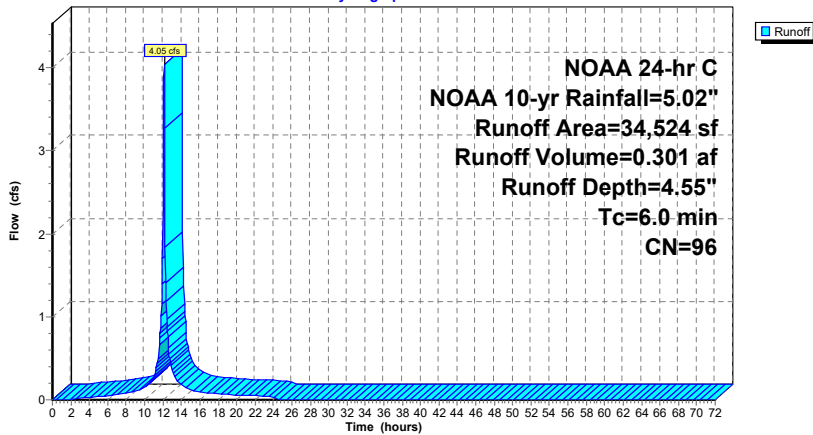
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
3,898	83	1/4 acre lots, 38% imp, HSG C
30,626	98	Paved parking, HSG C
34,524	96	Weighted Average
2,417		7.00% Pervious Area
32,107		93.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6: CB-6 (mix)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 12

**Summary for Subcatchment 8B: CB-8B (mix)**

Runoff = 2.89 cfs @ 12.13 hrs, Volume= 0.196 af, Depth= 3.59"  
Routed to Reach CB8 : EX-CB8

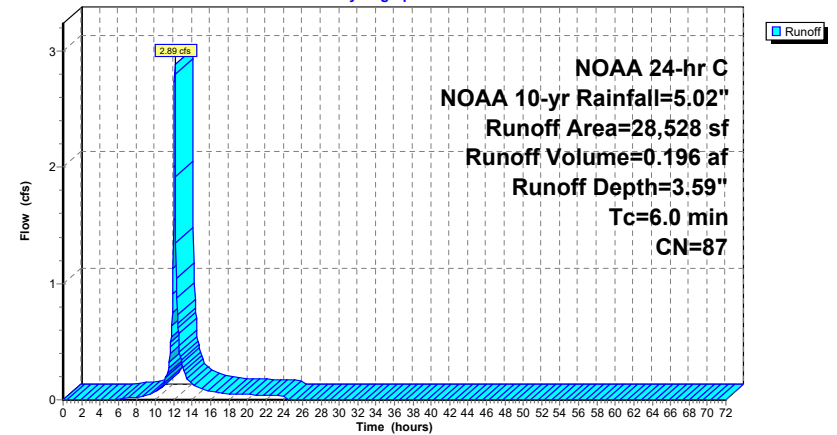
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
20,925	83	1/4 acre lots, 38% imp, HSG C
7,603	98	Paved parking, HSG C
28,528	87	Weighted Average
12,974		45.48% Pervious Area
15,555		54.52% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8B: CB-8B (mix)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 13

**Summary for Subcatchment 9: CB-9 (mix)**

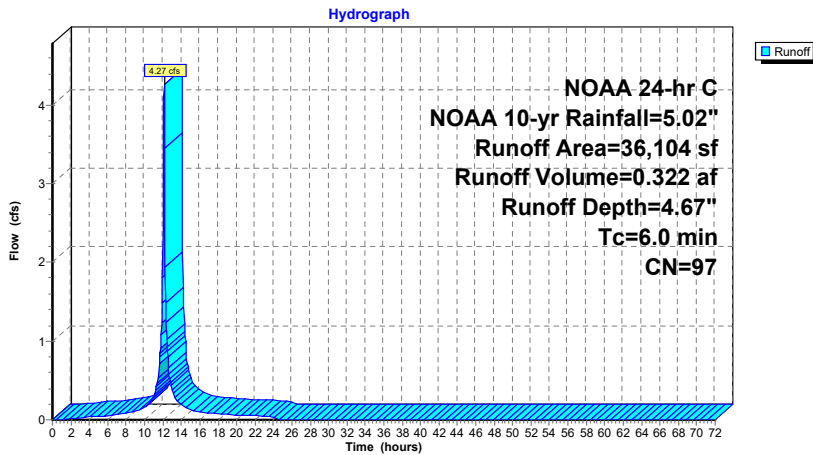
Runoff = 4.27 cfs @ 12.13 hrs, Volume= 0.322 af, Depth= 4.67"  
Routed to Reach CB9 : EX CB-9

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
3,474	83	1/4 acre lots, 38% imp, HSG C
32,630	98	Paved parking, HSG C
36,104	97	Weighted Average
2,154		5.97% Pervious Area
33,950		94.03% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 9: CB-9 (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 14

**Summary for Subcatchment 10: CB-10 (parking)**

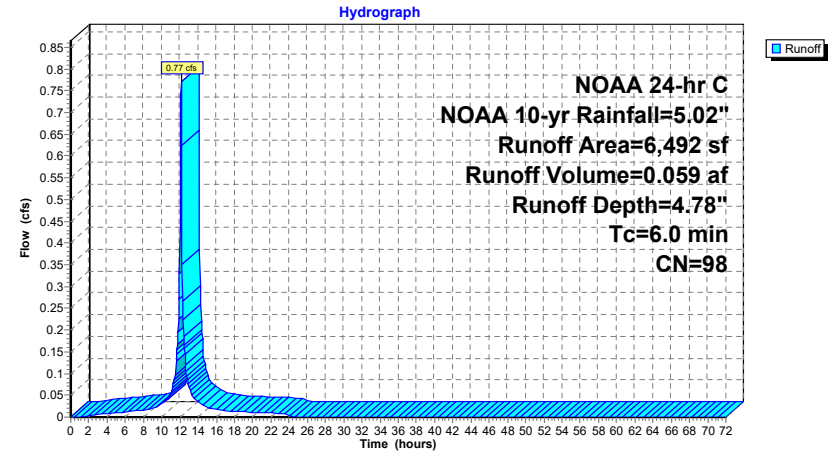
Runoff = 0.77 cfs @ 12.13 hrs, Volume= 0.059 af, Depth= 4.78"  
Routed to Reach CB10 : EX-CB10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
6,492	98	Paved parking, HSG C
6,492		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 10: CB-10 (parking)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 15

**Summary for Subcatchment CB-1: New CB South (Seymour Street)**

Runoff = 2.03 cfs @ 12.13 hrs, Volume= 0.138 af, Depth= 3.69"  
Routed to Reach CB4 : EX-CB4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

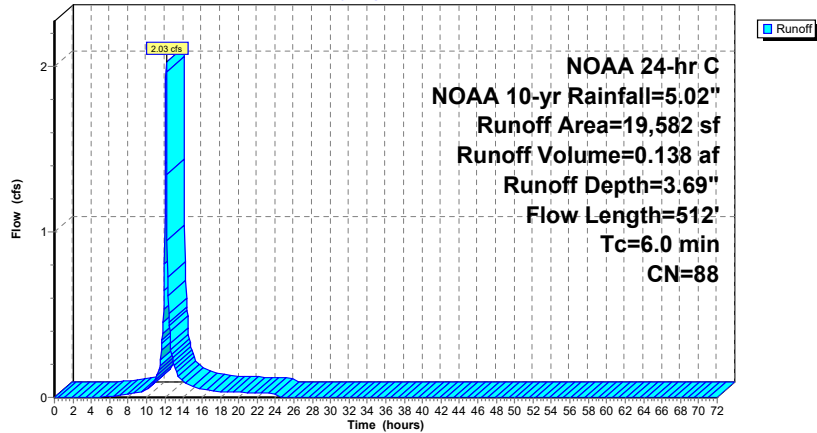
Area (sf)	CN	Description
13,211	83	1/4 acre lots, 38% imp, HSG C
* 6,371	98	Roadway
19,582	88	Weighted Average
8,191		41.83% Pervious Area
11,391		58.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	50	0.0300	1.45		<b>Sheet Flow, A-B</b> Smooth surfaces n= 0.011 P2= 3.40"
2.4	462	0.0249	3.20		<b>Shallow Concentrated Flow, Paved</b> Paved Kv= 20.3 fps
3.0					<b>Direct Entry, Direct entry to 6</b>
6.0	512	Total			

**Subcatchment CB-1: New CB South (Seymour Street)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 16

**Summary for Subcatchment CB-3: NEW CB SOUTH- HUDSON ST**

Runoff = 2.49 cfs @ 12.13 hrs, Volume= 0.168 af, Depth= 3.49"  
Routed to Reach CB7 : EX-CB7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

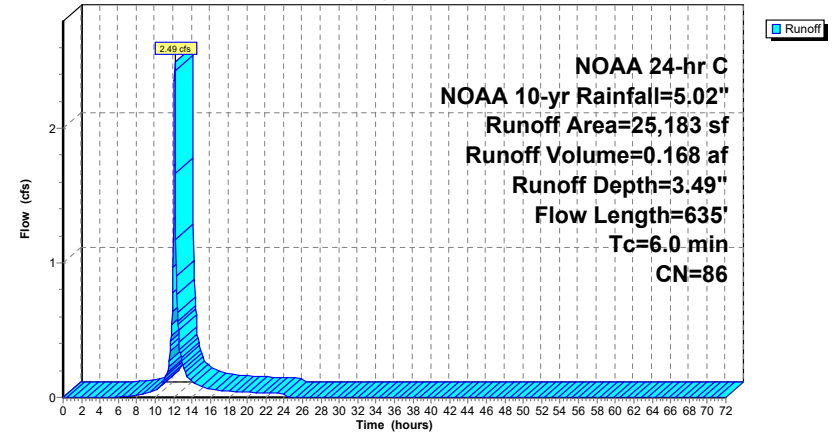
Area (sf)	CN	Description
19,562	83	1/4 acre lots, 38% imp, HSG C
* 5,621	98	Roadway
25,183	86	Weighted Average
12,128		48.16% Pervious Area
13,055		51.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0444	1.70		<b>Sheet Flow, A-B (sheet flow)</b> Smooth surfaces n= 0.011 P2= 3.40"
3.0	585	0.0256	3.25		<b>Shallow Concentrated Flow, B-C</b> Paved Kv= 20.3 fps
2.5					<b>Direct Entry, direct entry to 6</b>
6.0	635	Total			

**Subcatchment CB-3: NEW CB SOUTH- HUDSON ST**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 17

**Summary for Subcatchment CB-4: NEW CB NOTH - HUDSON STREET**

Runoff = 11.70 cfs @ 12.13 hrs, Volume= 0.788 af, Depth= 3.49"  
Routed to Reach CB8 : EX-CB8

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

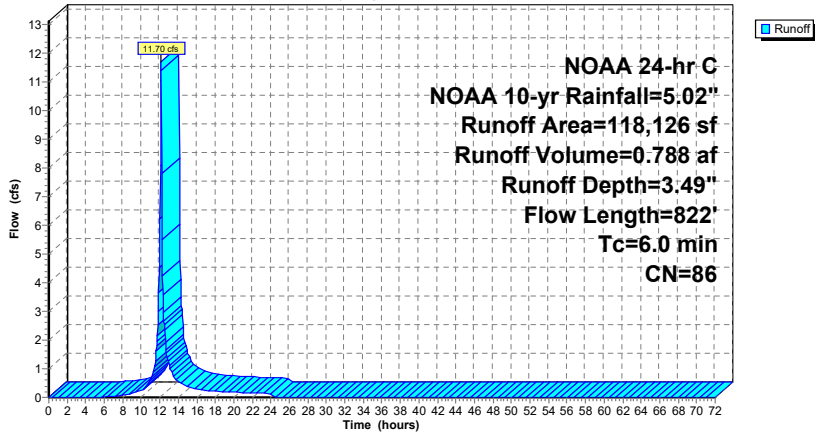
Area (sf)	CN	Description
96,716	83	1/4 acre lots, 38% imp, HSG C
* 21,410	98	Roadway
118,126	86	Weighted Average
59,964		50.76% Pervious Area
58,162		49.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
4.0	772	0.0245	3.18		Shallow Concentrated Flow, B-C (shallow concentrated) Paved Kv= 20.3 fps
1.5					Direct Entry, direct entry to 6
6.0	822	Total			

**Subcatchment CB-4: NEW CB NOTH - HUDSON STREET**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 18

**Summary for Subcatchment CB-5: NEW CB - PORTLAND ST SOUTH**

Runoff = 1.96 cfs @ 12.13 hrs, Volume= 0.132 af, Depth= 3.49"  
Routed to Reach CB11 : EX-CB11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

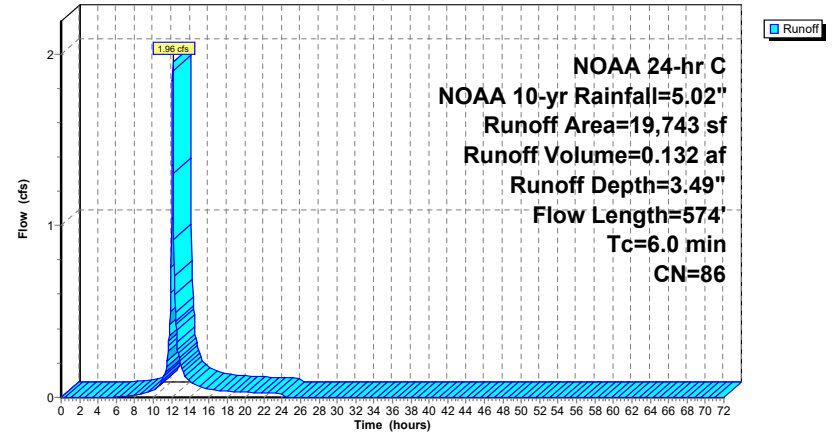
Area (sf)	CN	Description
15,657	83	1/4 acre lots, 38% imp, HSG C
* 4,086	98	Roadway
19,743	86	Weighted Average
9,707		49.17% Pervious Area
10,036		50.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
2.3	524	0.0346	3.78		Shallow Concentrated Flow, B-C (shallow conc.) Paved Kv= 20.3 fps
3.2					Direct Entry, direct to 6
6.0	574	Total			

**Subcatchment CB-5: NEW CB - PORTLAND ST SOUTH**

Hydrograph





**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 19

**Summary for Reach CB1: EX CB-1**

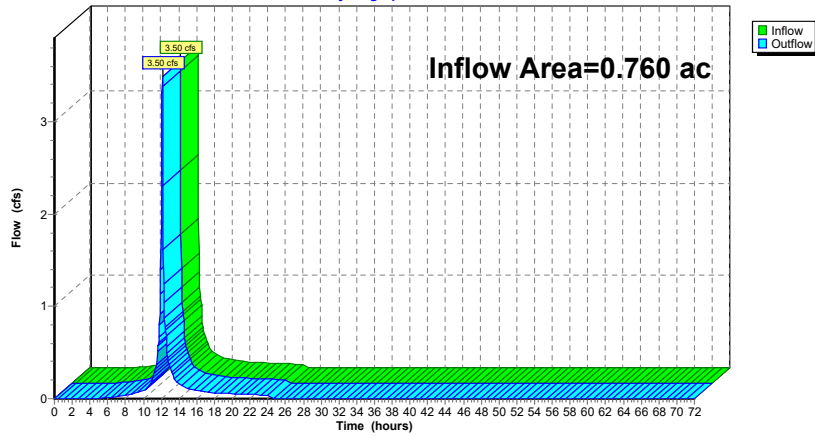
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.760 ac, 61.34% Impervious, Inflow Depth = 3.79" for NOAA 10-yr event  
Inflow = 3.50 cfs @ 12.13 hrs, Volume= 0.240 af  
Outflow = 3.50 cfs @ 12.13 hrs, Volume= 0.240 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB1: EX CB-1**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 20

**Summary for Reach CB10: EX-CB10**

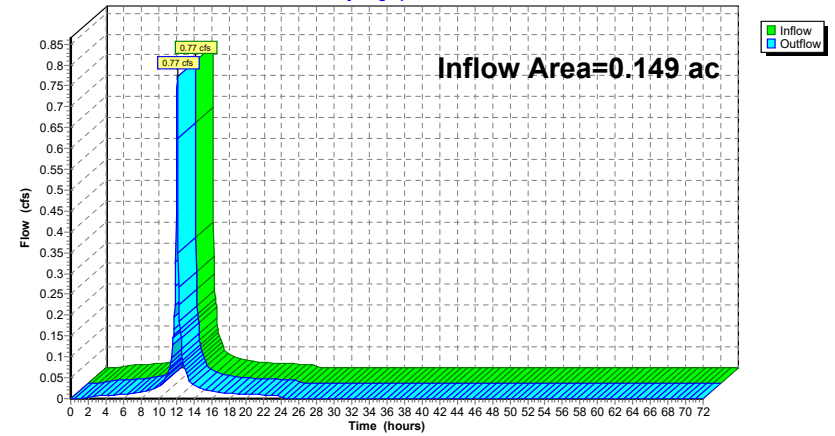
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.149 ac, 100.00% Impervious, Inflow Depth = 4.78" for NOAA 10-yr event  
Inflow = 0.77 cfs @ 12.13 hrs, Volume= 0.059 af  
Outflow = 0.77 cfs @ 12.13 hrs, Volume= 0.059 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB10: EX-CB10**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 21

**Summary for Reach CB11: EX-CB11**

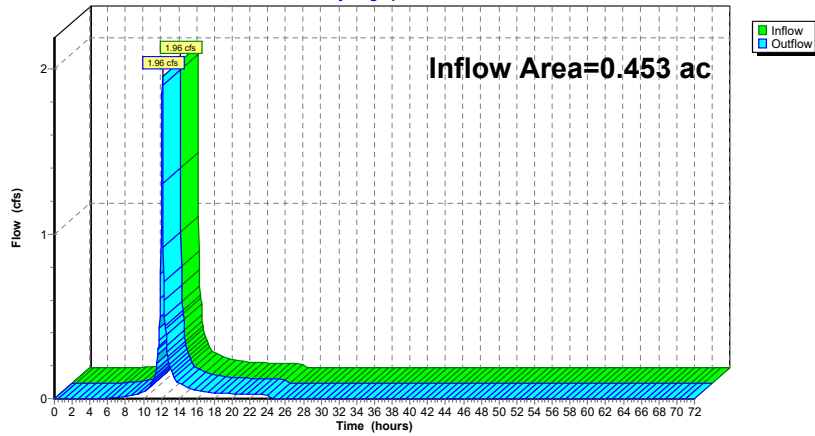
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.453 ac, 50.83% Impervious, Inflow Depth = 3.49" for NOAA 10-yr event  
Inflow = 1.96 cfs @ 12.13 hrs, Volume= 0.132 af  
Outflow = 1.96 cfs @ 12.13 hrs, Volume= 0.132 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB11: EX-CB11**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 22

**Summary for Reach CB2: EX-CB2**

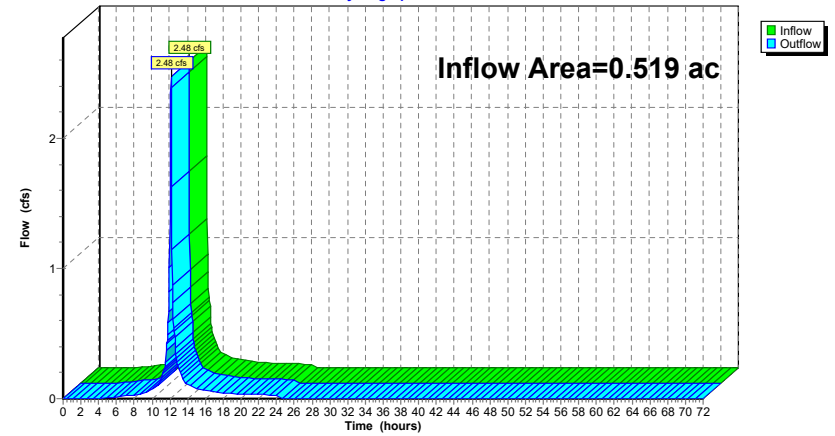
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.519 ac, 71.14% Impervious, Inflow Depth = 4.00" for NOAA 10-yr event  
Inflow = 2.48 cfs @ 12.13 hrs, Volume= 0.173 af  
Outflow = 2.48 cfs @ 12.13 hrs, Volume= 0.173 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB2: EX-CB2**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 23

**Summary for Reach CB3: EX-CB3**

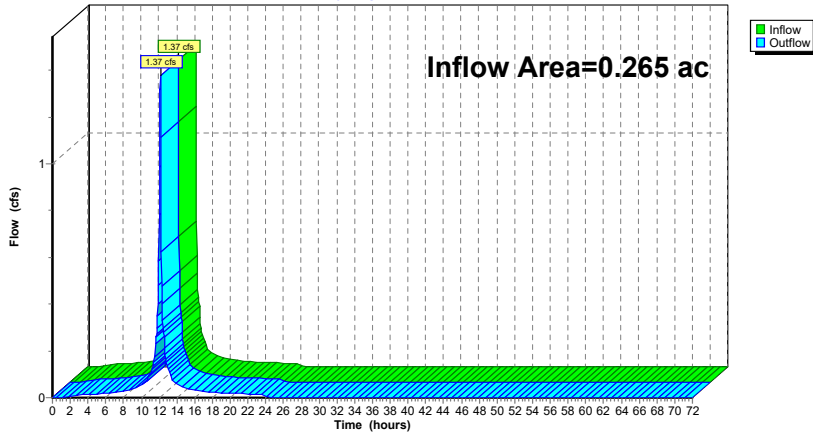
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.265 ac, 100.00% Impervious, Inflow Depth = 4.78" for NOAA 10-yr event  
Inflow = 1.37 cfs @ 12.13 hrs, Volume= 0.106 af  
Outflow = 1.37 cfs @ 12.13 hrs, Volume= 0.106 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB3: EX-CB3**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 24

**Summary for Reach CB4: EX-CB4**

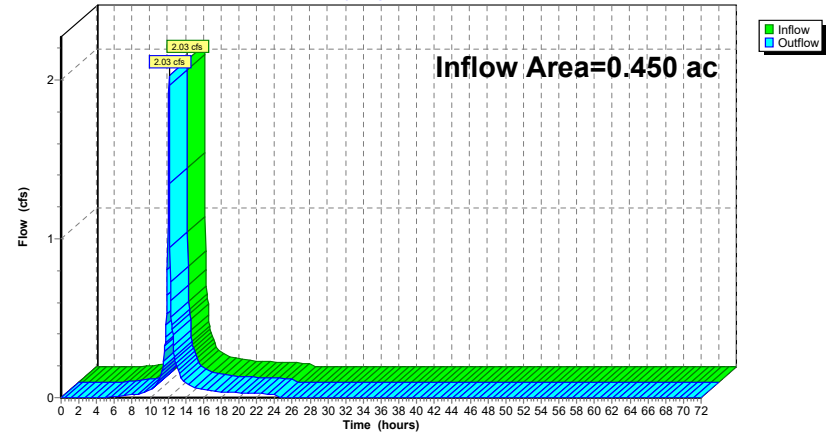
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.450 ac, 58.17% Impervious, Inflow Depth = 3.69" for NOAA 10-yr event  
Inflow = 2.03 cfs @ 12.13 hrs, Volume= 0.138 af  
Outflow = 2.03 cfs @ 12.13 hrs, Volume= 0.138 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB4: EX-CB4**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 25

**Summary for Reach CB5: EX-CB5**

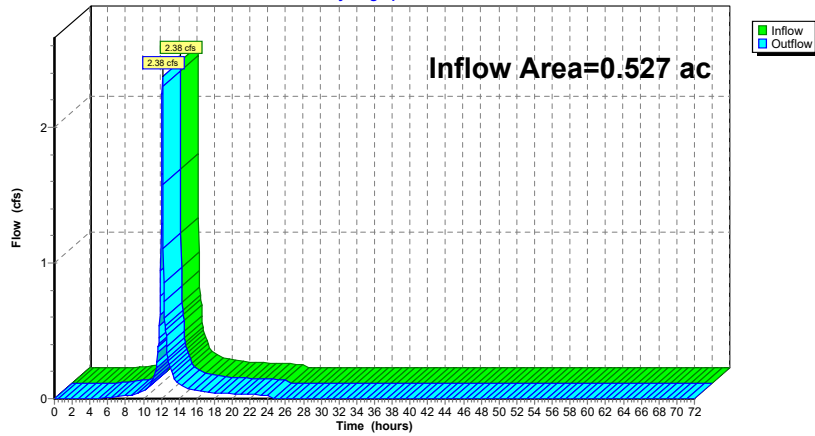
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.527 ac, 57.51% Impervious, Inflow Depth = 3.69" for NOAA 10-yr event  
Inflow = 2.38 cfs @ 12.13 hrs, Volume= 0.162 af  
Outflow = 2.38 cfs @ 12.13 hrs, Volume= 0.162 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB5: EX-CB5**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 26

**Summary for Reach CB6: EX-CB6**

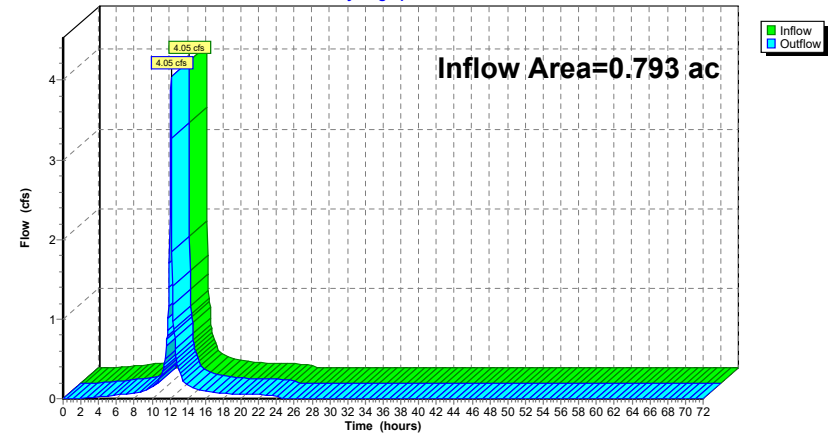
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.793 ac, 93.00% Impervious, Inflow Depth = 4.55" for NOAA 10-yr event  
Inflow = 4.05 cfs @ 12.13 hrs, Volume= 0.301 af  
Outflow = 4.05 cfs @ 12.13 hrs, Volume= 0.301 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB6: EX-CB6**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 27

**Summary for Reach CB7: EX-CB7**

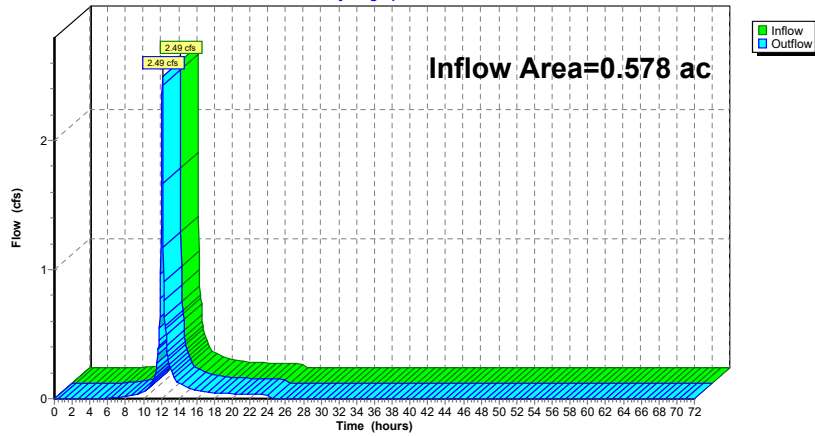
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.578 ac, 51.84% Impervious, Inflow Depth = 3.49" for NOAA 10-yr event  
Inflow = 2.49 cfs @ 12.13 hrs, Volume= 0.168 af  
Outflow = 2.49 cfs @ 12.13 hrs, Volume= 0.168 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB7: EX-CB7**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 28

**Summary for Reach CB8: EX-CB8**

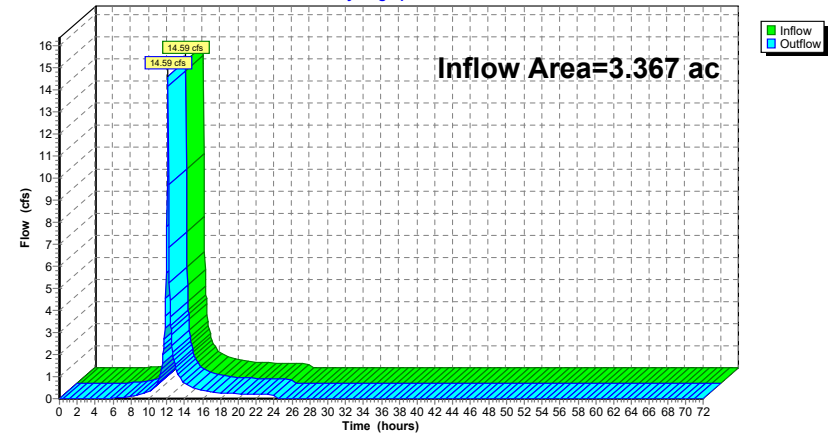
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.367 ac, 50.27% Impervious, Inflow Depth = 3.51" for NOAA 10-yr event  
Inflow = 14.59 cfs @ 12.13 hrs, Volume= 0.983 af  
Outflow = 14.59 cfs @ 12.13 hrs, Volume= 0.983 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB8: EX-CB8**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 29

**Summary for Reach CB9: EX CB-9**

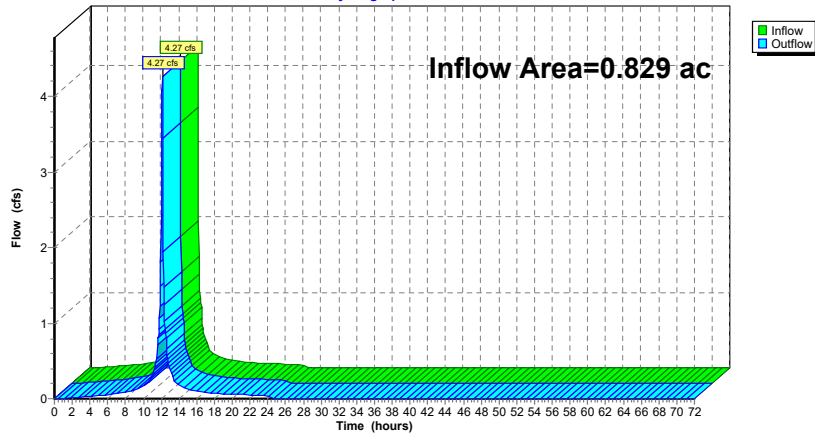
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.829 ac, 94.03% Impervious, Inflow Depth = 4.67" for NOAA 10-yr event  
Inflow = 4.27 cfs @ 12.13 hrs, Volume= 0.322 af  
Outflow = 4.27 cfs @ 12.13 hrs, Volume= 0.322 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB9: EX CB-9**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 30

**Summary for Reach DP-1: French Rodney Blvd 14" Outfall**

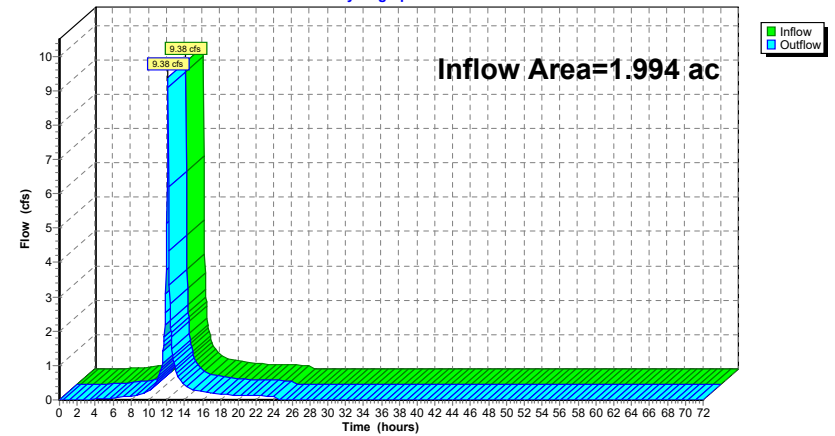
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.994 ac, 68.32% Impervious, Inflow Depth = 3.95" for NOAA 10-yr event  
Inflow = 9.38 cfs @ 12.13 hrs, Volume= 0.657 af  
Outflow = 9.38 cfs @ 12.13 hrs, Volume= 0.657 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-1: French Rodney Blvd 14" Outfall**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 31

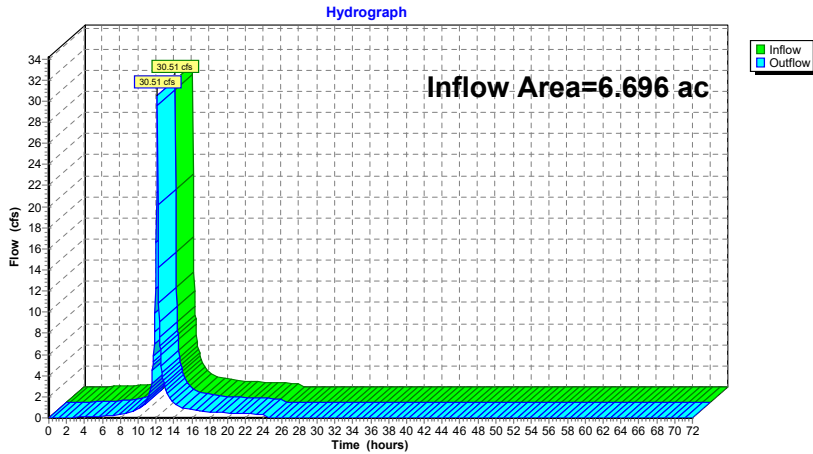
**Summary for Reach DP-2: PORTLAND STREET DRAINAGE**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.696 ac, 62.59% Impervious, Inflow Depth = 3.81" for NOAA 10-yr event  
 Inflow = 30.51 cfs @ 12.13 hrs, Volume= 2.128 af  
 Outflow = 30.51 cfs @ 12.13 hrs, Volume= 2.128 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-2: PORTLAND STREET DRAINAGE**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 32

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1B: CB-1B (mix)</b>	Runoff Area=33,097 sf 61.34% Impervious Runoff Depth=6.28" Tc=6.0 min CN=89 Runoff=5.61 cfs 0.398 af
<b>Subcatchment2: CB-2 (mix)</b>	Runoff Area=22,628 sf 71.14% Impervious Runoff Depth=6.52" Tc=6.0 min CN=91 Runoff=3.91 cfs 0.282 af
<b>Subcatchment3A: CB-3A (parking)</b>	Runoff Area=7,758 sf 100.00% Impervious Runoff Depth=7.35" Tc=6.0 min CN=98 Runoff=1.40 cfs 0.109 af
<b>Subcatchment3B: CB-3B (parking)</b>	Runoff Area=3,797 sf 100.00% Impervious Runoff Depth=7.35" Tc=6.0 min CN=98 Runoff=0.69 cfs 0.053 af
<b>Subcatchment5B: CB-5B (mix)</b>	Runoff Area=22,974 sf 57.51% Impervious Runoff Depth=6.17" Tc=6.0 min CN=88 Runoff=3.85 cfs 0.271 af
<b>Subcatchment6: CB-6 (mix)</b>	Runoff Area=34,524 sf 93.00% Impervious Runoff Depth=7.11" Tc=6.0 min CN=96 Runoff=6.18 cfs 0.470 af
<b>Subcatchment8B: CB-8B (mix)</b>	Runoff Area=28,528 sf 54.52% Impervious Runoff Depth=6.05" Tc=6.0 min CN=87 Runoff=4.73 cfs 0.330 af
<b>Subcatchment9: CB-9 (mix)</b>	Runoff Area=36,104 sf 94.03% Impervious Runoff Depth=7.23" Tc=6.0 min CN=97 Runoff=6.49 cfs 0.499 af
<b>Subcatchment10: CB-10 (parking)</b>	Runoff Area=6,492 sf 100.00% Impervious Runoff Depth=7.35" Tc=6.0 min CN=98 Runoff=1.17 cfs 0.091 af
<b>SubcatchmentCB-1: New CB South</b>	Runoff Area=19,582 sf 58.17% Impervious Runoff Depth=6.17" Flow Length=512' Tc=6.0 min CN=88 Runoff=3.28 cfs 0.231 af
<b>SubcatchmentCB-3: NEW CB SOUTH-</b>	Runoff Area=25,183 sf 51.84% Impervious Runoff Depth=5.93" Flow Length=635' Tc=6.0 min CN=86 Runoff=4.12 cfs 0.286 af
<b>SubcatchmentCB-4: NEW CB NOTH -</b>	Runoff Area=118,126 sf 49.24% Impervious Runoff Depth=5.93" Flow Length=822' Tc=6.0 min CN=86 Runoff=19.32 cfs 1.341 af
<b>SubcatchmentCB-5: NEW CB -</b>	Runoff Area=19,743 sf 50.83% Impervious Runoff Depth=5.93" Flow Length=574' Tc=6.0 min CN=86 Runoff=3.23 cfs 0.224 af
<b>Reach CB1: EX CB-1</b>	Inflow=5.61 cfs 0.398 af Outflow=5.61 cfs 0.398 af
<b>Reach CB10: EX-CB10</b>	Inflow=1.17 cfs 0.091 af Outflow=1.17 cfs 0.091 af
<b>Reach CB11: EX-CB11</b>	Inflow=3.23 cfs 0.224 af Outflow=3.23 cfs 0.224 af

**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 33

<b>Reach CB2: EX-CB2</b>	Inflow=3.91 cfs 0.282 af Outflow=3.91 cfs 0.282 af
<b>Reach CB3: EX-CB3</b>	Inflow=2.08 cfs 0.162 af Outflow=2.08 cfs 0.162 af
<b>Reach CB4: EX-CB4</b>	Inflow=3.28 cfs 0.231 af Outflow=3.28 cfs 0.231 af
<b>Reach CB5: EX-CB5</b>	Inflow=3.85 cfs 0.271 af Outflow=3.85 cfs 0.271 af
<b>Reach CB6: EX-CB6</b>	Inflow=6.18 cfs 0.470 af Outflow=6.18 cfs 0.470 af
<b>Reach CB7: EX-CB7</b>	Inflow=4.12 cfs 0.286 af Outflow=4.12 cfs 0.286 af
<b>Reach CB8: EX-CB8</b>	Inflow=24.05 cfs 1.671 af Outflow=24.05 cfs 1.671 af
<b>Reach CB9: EX CB-9</b>	Inflow=6.49 cfs 0.499 af Outflow=6.49 cfs 0.499 af
<b>Reach DP-1: French Rodney Blvd 14" Outfall</b>	Inflow=14.90 cfs 1.074 af Outflow=14.90 cfs 1.074 af
<b>Reach DP-2: PORTLANDSTREET DRAINAGE</b>	Inflow=49.10 cfs 3.513 af Outflow=49.10 cfs 3.513 af

Total Runoff Area = 8.690 ac Runoff Volume = 4.587 af Average Runoff Depth = 6.33"  
36.09% Pervious = 3.136 ac 63.91% Impervious = 5.554 ac

**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 34

**Summary for Subcatchment 1B: CB-1B (mix)**

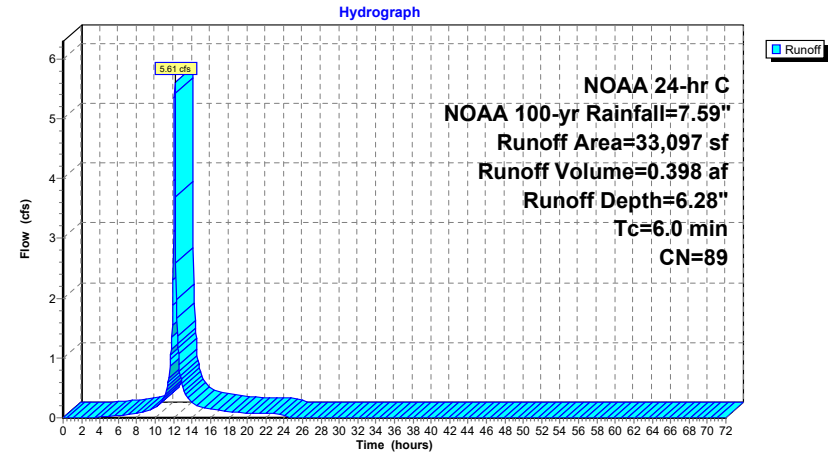
Runoff = 5.61 cfs @ 12.13 hrs, Volume= 0.398 af, Depth= 6.28"  
Routed to Reach CB1 : EX CB-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
20,636	83	1/4 acre lots, 38% imp, HSG C
12,461	98	Paved parking, HSG C
33,097	89	Weighted Average
12,794		38.66% Pervious Area
20,303		61.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 1B: CB-1B (mix)**





**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 35

**Summary for Subcatchment 2: CB-2 (mix)**

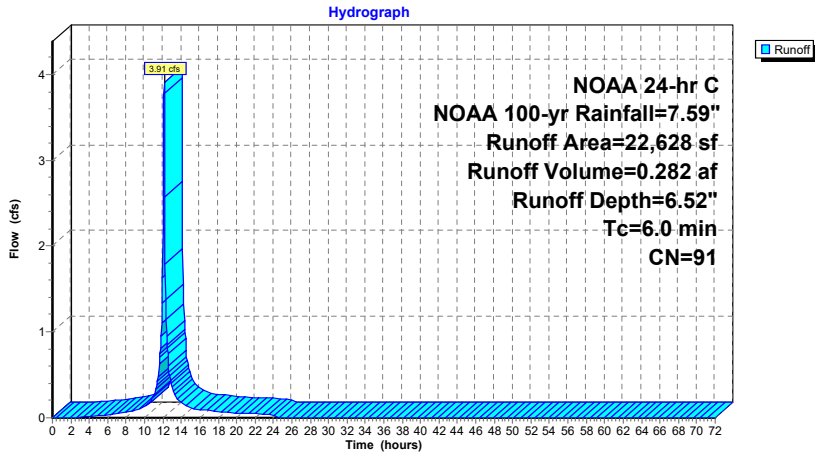
Runoff = 3.91 cfs @ 12.13 hrs, Volume= 0.282 af, Depth= 6.52"  
Routed to Reach CB2 : EX-CB2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
10,533	83	1/4 acre lots, 38% imp, HSG C
12,095	98	Paved parking, HSG C
22,628	91	Weighted Average
6,530		28.86% Pervious Area
16,098		71.14% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2: CB-2 (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 36

**Summary for Subcatchment 3A: CB-3A (parking)**

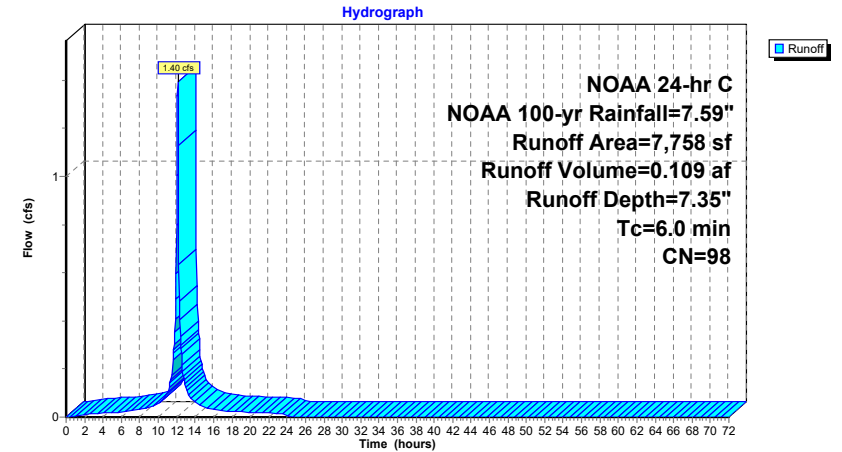
Runoff = 1.40 cfs @ 12.13 hrs, Volume= 0.109 af, Depth= 7.35"  
Routed to Reach CB3 : EX-CB3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
7,758	98	Paved parking, HSG C
7,758		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3A: CB-3A (parking)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 37

**Summary for Subcatchment 3B: CB-3B (parking)**

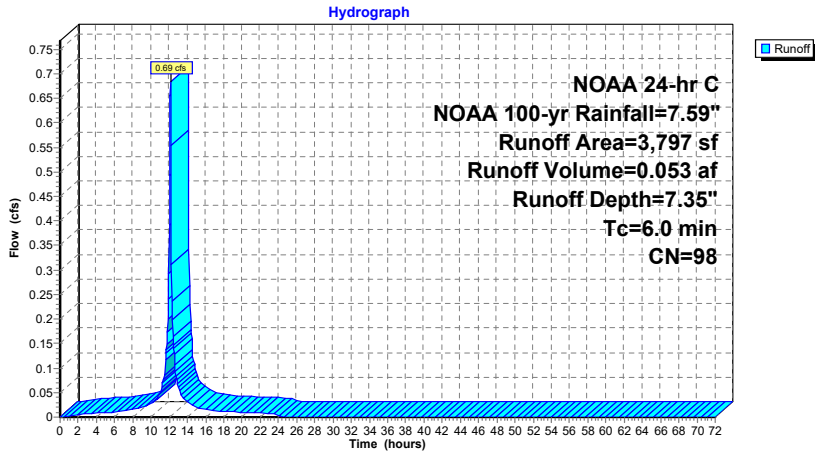
Runoff = 0.69 cfs @ 12.13 hrs, Volume= 0.053 af, Depth= 7.35"  
Routed to Reach CB3 : EX-CB3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
3,797	98	Paved parking, HSG C
3,797		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3B: CB-3B (parking)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 38

**Summary for Subcatchment 5B: CB-5B (mix)**

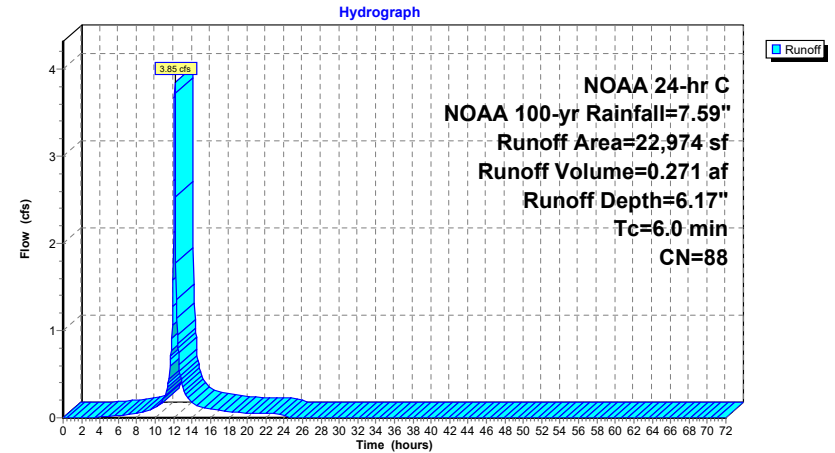
Runoff = 3.85 cfs @ 12.13 hrs, Volume= 0.271 af, Depth= 6.17"  
Routed to Reach CB5 : EX-CB5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
15,743	83	1/4 acre lots, 38% imp, HSG C
7,231	98	Paved parking, HSG C
22,974	88	Weighted Average
9,761		42.49% Pervious Area
13,213		57.51% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5B: CB-5B (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 39

**Summary for Subcatchment 6: CB-6 (mix)**

Runoff = 6.18 cfs @ 12.13 hrs, Volume= 0.470 af, Depth= 7.11"  
Routed to Reach CB6 : EX-CB6

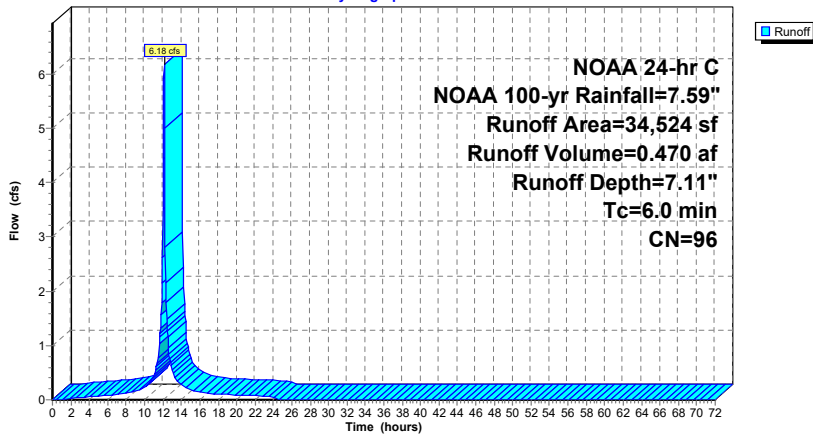
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
3,898	83	1/4 acre lots, 38% imp, HSG C
30,626	98	Paved parking, HSG C
34,524	96	Weighted Average
2,417		7.00% Pervious Area
32,107		93.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6: CB-6 (mix)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 40

**Summary for Subcatchment 8B: CB-8B (mix)**

Runoff = 4.73 cfs @ 12.13 hrs, Volume= 0.330 af, Depth= 6.05"  
Routed to Reach CB8 : EX-CB8

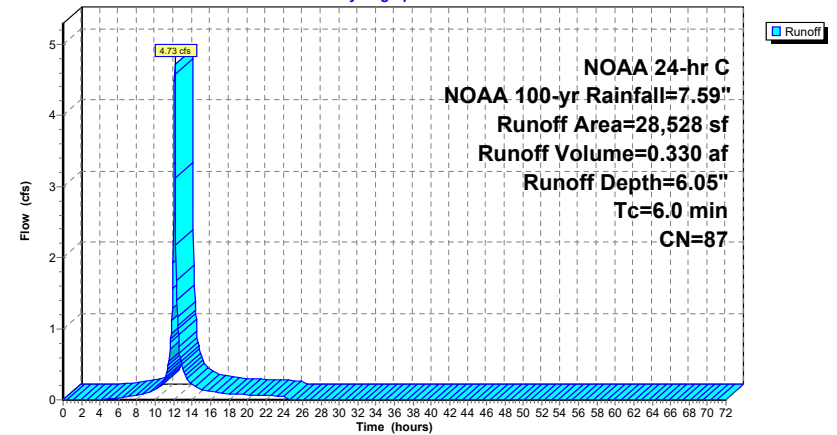
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
20,925	83	1/4 acre lots, 38% imp, HSG C
7,603	98	Paved parking, HSG C
28,528	87	Weighted Average
12,974		45.48% Pervious Area
15,555		54.52% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8B: CB-8B (mix)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 41

**Summary for Subcatchment 9: CB-9 (mix)**

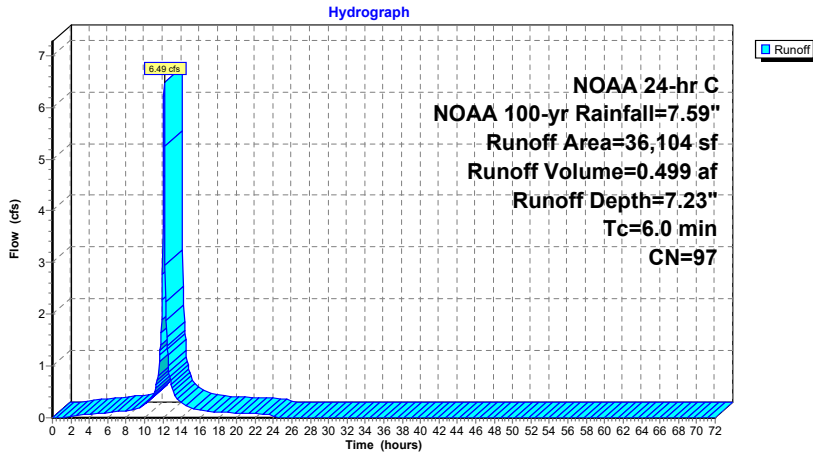
Runoff = 6.49 cfs @ 12.13 hrs, Volume= 0.499 af, Depth= 7.23"  
Routed to Reach CB9 : EX CB-9

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
3,474	83	1/4 acre lots, 38% imp, HSG C
32,630	98	Paved parking, HSG C
36,104	97	Weighted Average
2,154		5.97% Pervious Area
33,950		94.03% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 9: CB-9 (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 42

**Summary for Subcatchment 10: CB-10 (parking)**

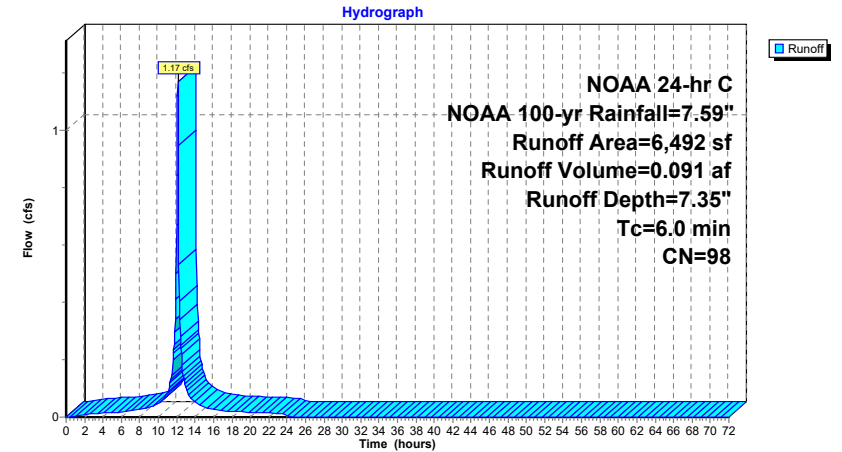
Runoff = 1.17 cfs @ 12.13 hrs, Volume= 0.091 af, Depth= 7.35"  
Routed to Reach CB10 : EX-CB10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
6,492	98	Paved parking, HSG C
6,492		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 10: CB-10 (parking)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 43

**Summary for Subcatchment CB-1: New CB South (Seymour Street)**

Runoff = 3.28 cfs @ 12.13 hrs, Volume= 0.231 af, Depth= 6.17"  
Routed to Reach CB4 : EX-CB4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

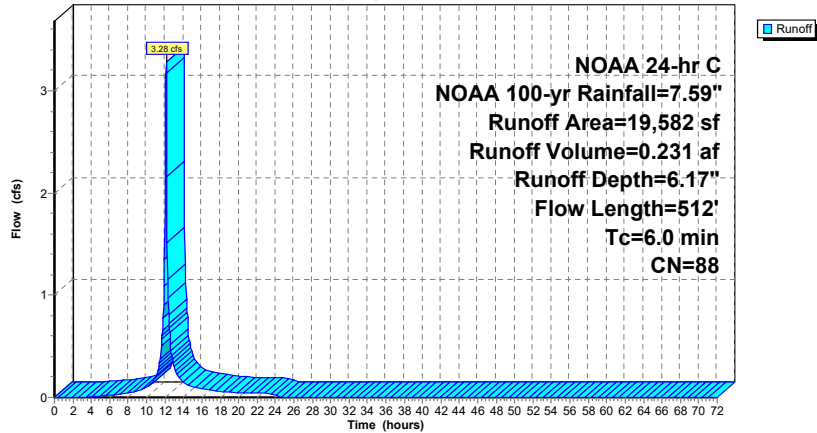
Area (sf)	CN	Description
13,211	83	1/4 acre lots, 38% imp, HSG C
* 6,371	98	Roadway
19,582	88	Weighted Average
8,191		41.83% Pervious Area
11,391		58.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	50	0.0300	1.45		<b>Sheet Flow, A-B</b> Smooth surfaces n= 0.011 P2= 3.40"
2.4	462	0.0249	3.20		<b>Shallow Concentrated Flow, Paved</b> Paved Kv= 20.3 fps
3.0					<b>Direct Entry, Direct entry to 6</b>
6.0	512	Total			

**Subcatchment CB-1: New CB South (Seymour Street)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 44

**Summary for Subcatchment CB-3: NEW CB SOUTH- HUDSON ST**

Runoff = 4.12 cfs @ 12.13 hrs, Volume= 0.286 af, Depth= 5.93"  
Routed to Reach CB7 : EX-CB7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

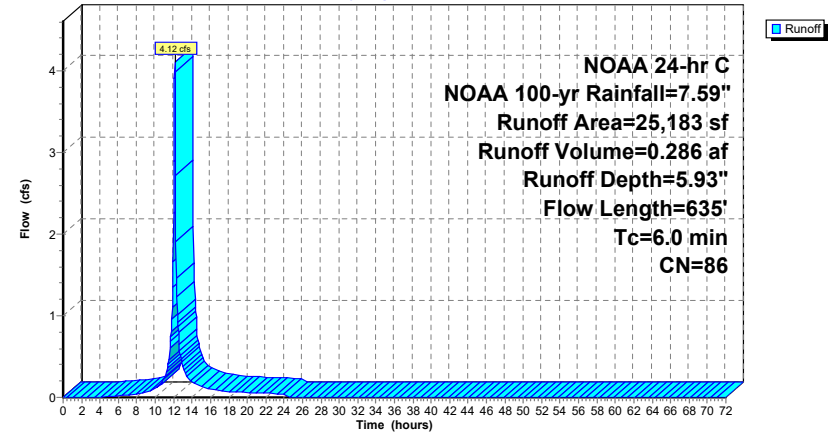
Area (sf)	CN	Description
19,562	83	1/4 acre lots, 38% imp, HSG C
* 5,621	98	Roadway
25,183	86	Weighted Average
12,128		48.16% Pervious Area
13,055		51.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0444	1.70		<b>Sheet Flow, A-B (sheet flow)</b> Smooth surfaces n= 0.011 P2= 3.40"
3.0	585	0.0256	3.25		<b>Shallow Concentrated Flow, B-C</b> Paved Kv= 20.3 fps
2.5					<b>Direct Entry, direct entry to 6</b>
6.0	635	Total			

**Subcatchment CB-3: NEW CB SOUTH- HUDSON ST**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 45

**Summary for Subcatchment CB-4: NEW CB NOTH - HUDSON STREET**

Runoff = 19.32 cfs @ 12.13 hrs, Volume= 1.341 af, Depth= 5.93"  
Routed to Reach CB8 : EX-CB8

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

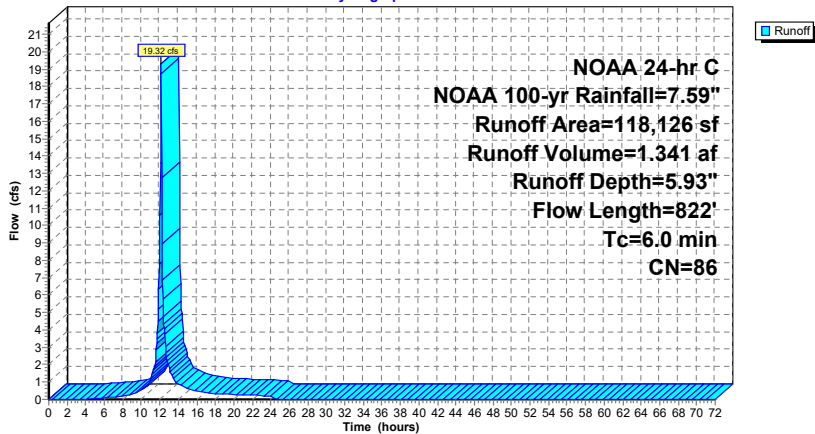
Area (sf)	CN	Description
96,716	83	1/4 acre lots, 38% imp, HSG C
* 21,410	98	Roadway
118,126	86	Weighted Average
59,964		50.76% Pervious Area
58,162		49.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
4.0	772	0.0245	3.18		Shallow Concentrated Flow, B-C (shallow concentrated) Paved Kv= 20.3 fps
1.5					Direct Entry, direct entry to 6
6.0	822	Total			

**Subcatchment CB-4: NEW CB NOTH - HUDSON STREET**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 46

**Summary for Subcatchment CB-5: NEW CB - PORTLAND ST SOUTH**

Runoff = 3.23 cfs @ 12.13 hrs, Volume= 0.224 af, Depth= 5.93"  
Routed to Reach CB11 : EX-CB11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

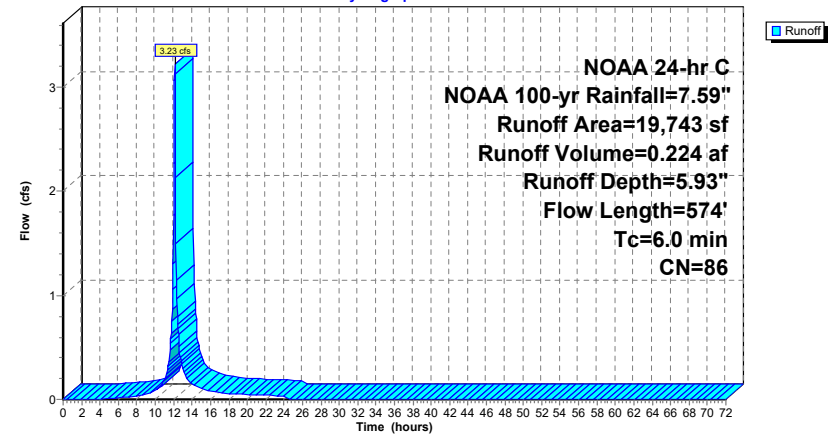
Area (sf)	CN	Description
15,657	83	1/4 acre lots, 38% imp, HSG C
* 4,086	98	Roadway
19,743	86	Weighted Average
9,707		49.17% Pervious Area
10,036		50.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
2.3	524	0.0346	3.78		Shallow Concentrated Flow, B-C (shallow conc.) Paved Kv= 20.3 fps
3.2					Direct Entry, direct to 6
6.0	574	Total			

**Subcatchment CB-5: NEW CB - PORTLAND ST SOUTH**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 47

**Summary for Reach CB1: EX CB-1**

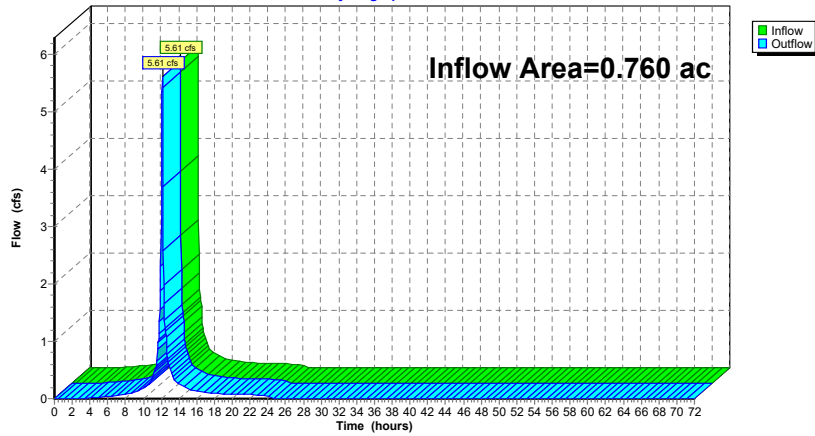
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.760 ac, 61.34% Impervious, Inflow Depth = 6.28" for NOAA 100-yr event  
Inflow = 5.61 cfs @ 12.13 hrs, Volume= 0.398 af  
Outflow = 5.61 cfs @ 12.13 hrs, Volume= 0.398 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB1: EX CB-1**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 48

**Summary for Reach CB10: EX-CB10**

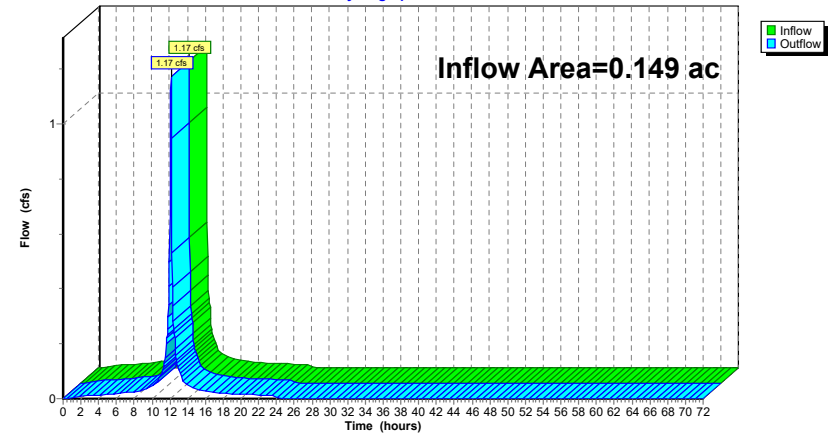
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.149 ac, 100.00% Impervious, Inflow Depth = 7.35" for NOAA 100-yr event  
Inflow = 1.17 cfs @ 12.13 hrs, Volume= 0.091 af  
Outflow = 1.17 cfs @ 12.13 hrs, Volume= 0.091 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB10: EX-CB10**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 49

**Summary for Reach CB11: EX-CB11**

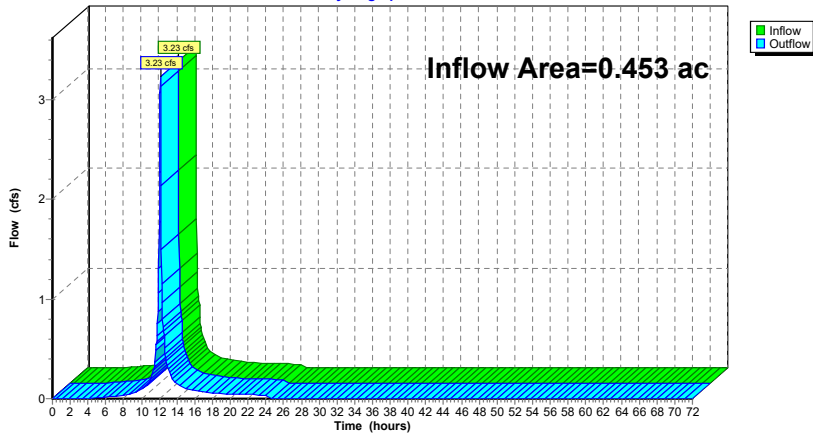
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.453 ac, 50.83% Impervious, Inflow Depth = 5.93" for NOAA 100-yr event  
Inflow = 3.23 cfs @ 12.13 hrs, Volume= 0.224 af  
Outflow = 3.23 cfs @ 12.13 hrs, Volume= 0.224 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB11: EX-CB11**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 50

**Summary for Reach CB2: EX-CB2**

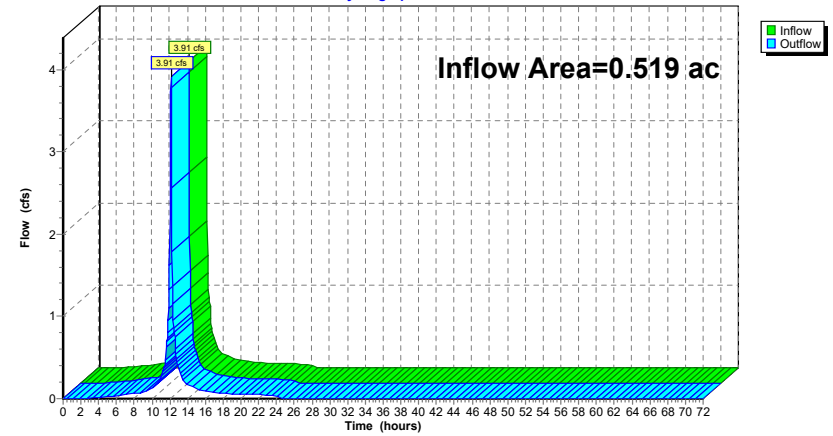
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.519 ac, 71.14% Impervious, Inflow Depth = 6.52" for NOAA 100-yr event  
Inflow = 3.91 cfs @ 12.13 hrs, Volume= 0.282 af  
Outflow = 3.91 cfs @ 12.13 hrs, Volume= 0.282 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB2: EX-CB2**

Hydrograph





**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 51

**Summary for Reach CB3: EX-CB3**

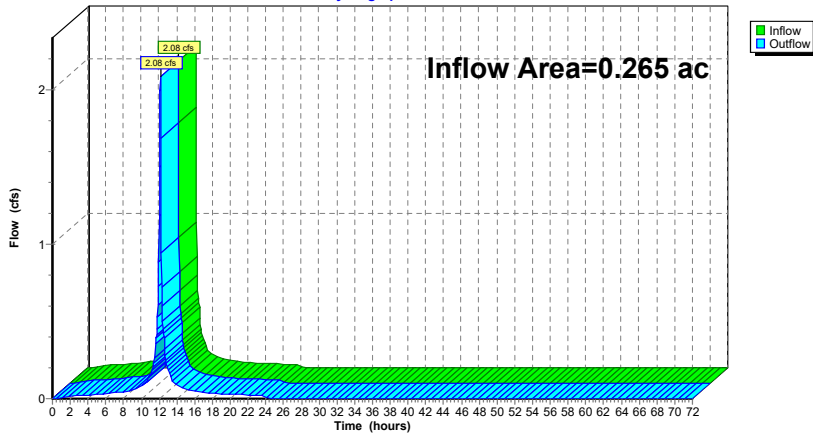
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.265 ac, 100.00% Impervious, Inflow Depth = 7.35" for NOAA 100-yr event  
Inflow = 2.08 cfs @ 12.13 hrs, Volume= 0.162 af  
Outflow = 2.08 cfs @ 12.13 hrs, Volume= 0.162 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB3: EX-CB3**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 52

**Summary for Reach CB4: EX-CB4**

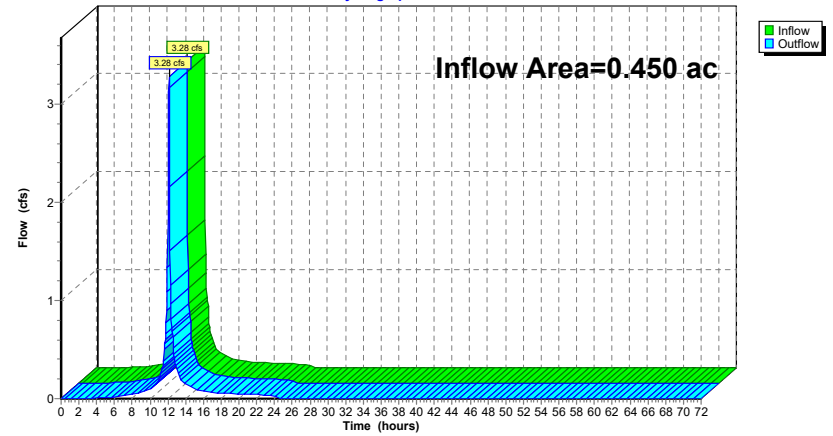
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.450 ac, 58.17% Impervious, Inflow Depth = 6.17" for NOAA 100-yr event  
Inflow = 3.28 cfs @ 12.13 hrs, Volume= 0.231 af  
Outflow = 3.28 cfs @ 12.13 hrs, Volume= 0.231 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB4: EX-CB4**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 53

**Summary for Reach CB5: EX-CB5**

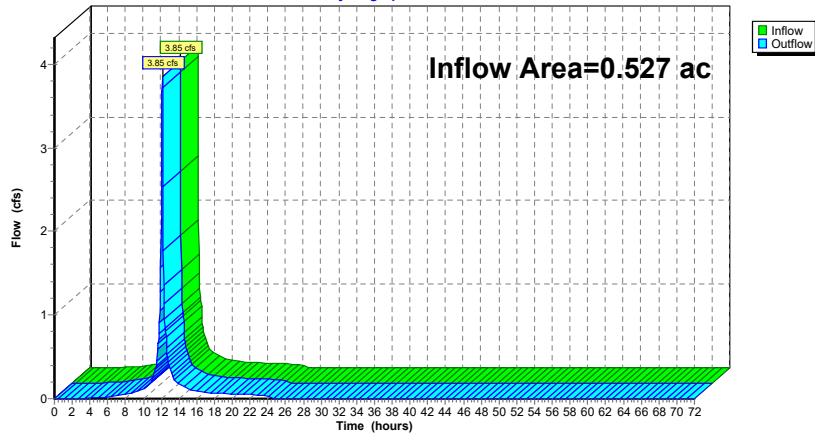
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.527 ac, 57.51% Impervious, Inflow Depth = 6.17" for NOAA 100-yr event  
Inflow = 3.85 cfs @ 12.13 hrs, Volume= 0.271 af  
Outflow = 3.85 cfs @ 12.13 hrs, Volume= 0.271 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB5: EX-CB5**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 54

**Summary for Reach CB6: EX-CB6**

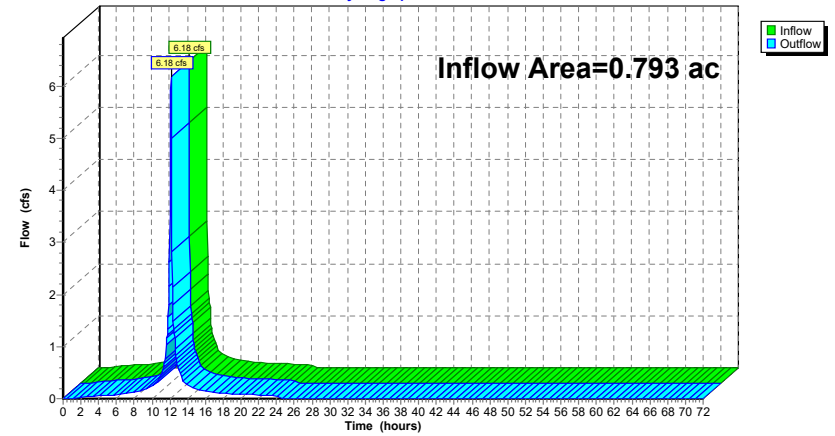
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.793 ac, 93.00% Impervious, Inflow Depth = 7.11" for NOAA 100-yr event  
Inflow = 6.18 cfs @ 12.13 hrs, Volume= 0.470 af  
Outflow = 6.18 cfs @ 12.13 hrs, Volume= 0.470 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB6: EX-CB6**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 55

**Summary for Reach CB7: EX-CB7**

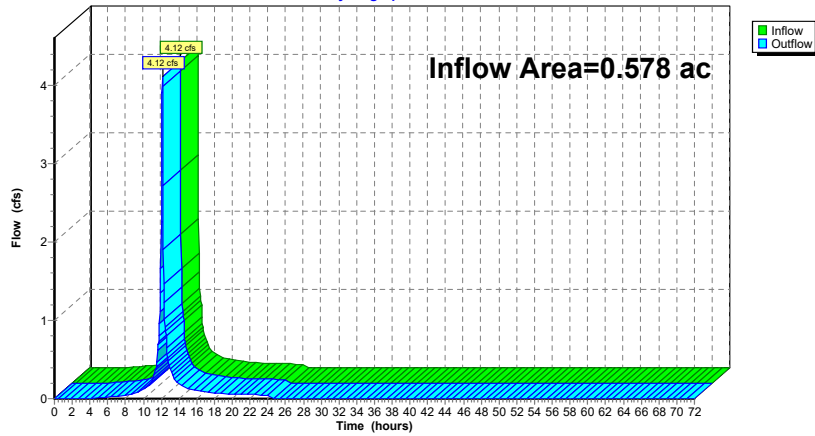
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.578 ac, 51.84% Impervious, Inflow Depth = 5.93" for NOAA 100-yr event  
Inflow = 4.12 cfs @ 12.13 hrs, Volume= 0.286 af  
Outflow = 4.12 cfs @ 12.13 hrs, Volume= 0.286 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB7: EX-CB7**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 56

**Summary for Reach CB8: EX-CB8**

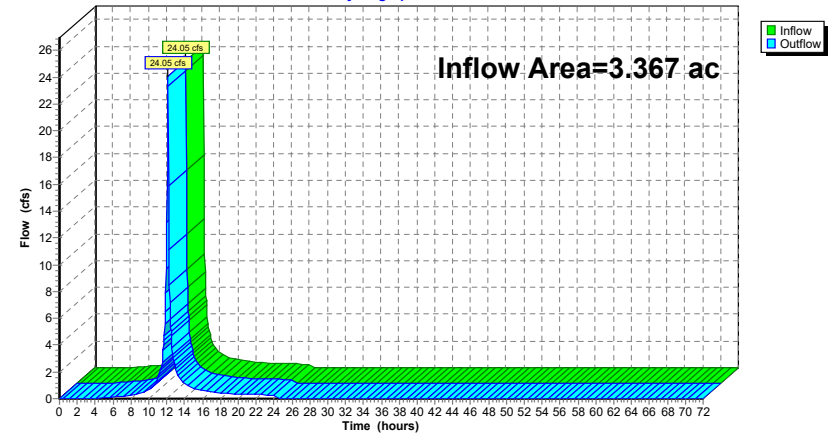
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.367 ac, 50.27% Impervious, Inflow Depth = 5.96" for NOAA 100-yr event  
Inflow = 24.05 cfs @ 12.13 hrs, Volume= 1.671 af  
Outflow = 24.05 cfs @ 12.13 hrs, Volume= 1.671 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB8: EX-CB8**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 57

**Summary for Reach CB9: EX CB-9**

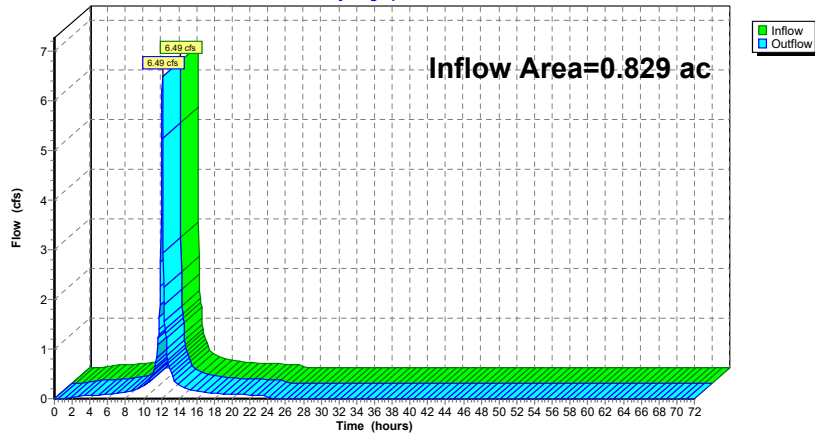
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.829 ac, 94.03% Impervious, Inflow Depth = 7.23" for NOAA 100-yr event  
Inflow = 6.49 cfs @ 12.13 hrs, Volume= 0.499 af  
Outflow = 6.49 cfs @ 12.13 hrs, Volume= 0.499 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB9: EX CB-9**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 58

**Summary for Reach DP-1: French Rodney Blvd 14" Outfall**

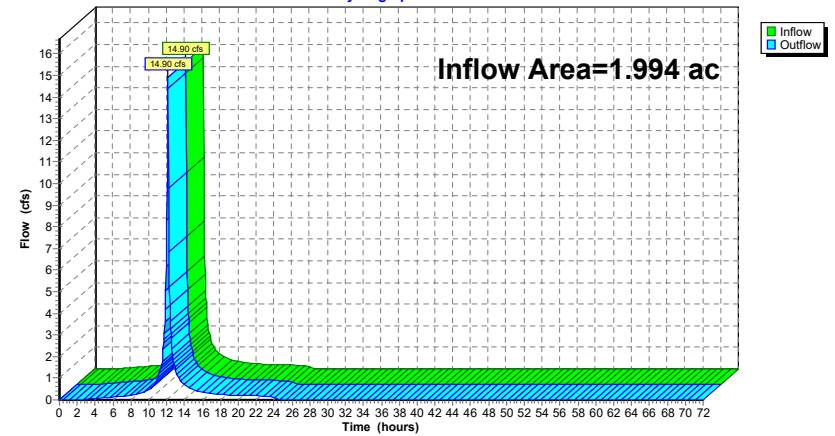
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.994 ac, 68.32% Impervious, Inflow Depth = 6.46" for NOAA 100-yr event  
Inflow = 14.90 cfs @ 12.13 hrs, Volume= 1.074 af  
Outflow = 14.90 cfs @ 12.13 hrs, Volume= 1.074 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-1: French Rodney Blvd 14" Outfall**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

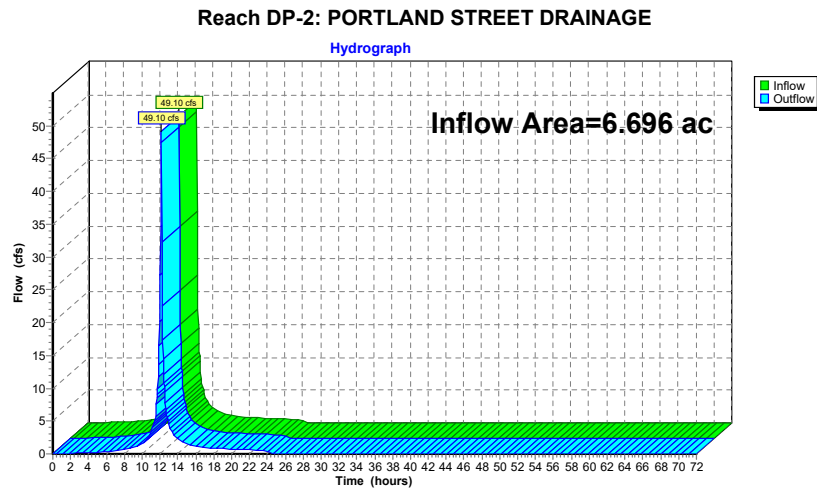
Page 59

**Summary for Reach DP-2: PORTLAND STREET DRAINAGE**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.696 ac, 62.59% Impervious, Inflow Depth = 6.30" for NOAA 100-yr event  
 Inflow = 49.10 cfs @ 12.13 hrs, Volume= 3.513 af  
 Outflow = 49.10 cfs @ 12.13 hrs, Volume= 3.513 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 60

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1B: CB-1B (mix)</b>	Runoff Area=33,097 sf 61.34% Impervious Runoff Depth=2.26" Tc=6.0 min CN=89 Runoff=2.15 cfs 0.143 af
<b>Subcatchment2: CB-2 (mix)</b>	Runoff Area=22,628 sf 71.14% Impervious Runoff Depth=2.45" Tc=6.0 min CN=91 Runoff=1.56 cfs 0.106 af
<b>Subcatchment3A: CB-3A (parking)</b>	Runoff Area=7,758 sf 100.00% Impervious Runoff Depth=3.17" Tc=6.0 min CN=98 Runoff=0.62 cfs 0.047 af
<b>Subcatchment3B: CB-3B (parking)</b>	Runoff Area=3,797 sf 100.00% Impervious Runoff Depth=3.17" Tc=6.0 min CN=98 Runoff=0.30 cfs 0.023 af
<b>Subcatchment5B: CB-5B (mix)</b>	Runoff Area=22,974 sf 57.51% Impervious Runoff Depth=2.18" Tc=6.0 min CN=88 Runoff=1.44 cfs 0.096 af
<b>Subcatchment6: CB-6 (mix)</b>	Runoff Area=34,524 sf 93.00% Impervious Runoff Depth=2.95" Tc=6.0 min CN=96 Runoff=2.69 cfs 0.195 af
<b>Subcatchment8B: CB-8B (mix)</b>	Runoff Area=28,528 sf 54.52% Impervious Runoff Depth=2.09" Tc=6.0 min CN=87 Runoff=1.73 cfs 0.114 af
<b>Subcatchment9: CB-9 (mix)</b>	Runoff Area=36,104 sf 94.03% Impervious Runoff Depth=3.06" Tc=6.0 min CN=97 Runoff=2.86 cfs 0.211 af
<b>Subcatchment10: CB-10 (parking)</b>	Runoff Area=6,492 sf 100.00% Impervious Runoff Depth=3.17" Tc=6.0 min CN=98 Runoff=0.52 cfs 0.039 af
<b>SubcatchmentCB-1: New CB South</b>	Runoff Area=19,582 sf 58.17% Impervious Runoff Depth=2.18" Flow Length=512' Tc=6.0 min CN=88 Runoff=1.23 cfs 0.082 af
<b>SubcatchmentCB-3: NEW CB SOUTH-</b>	Runoff Area=25,183 sf 51.84% Impervious Runoff Depth=2.01" Flow Length=635' Tc=6.0 min CN=86 Runoff=1.47 cfs 0.097 af
<b>SubcatchmentCB-4: NEW CB NOTH -</b>	Runoff Area=118,126 sf 49.24% Impervious Runoff Depth=2.01" Flow Length=822' Tc=6.0 min CN=86 Runoff=6.90 cfs 0.454 af
<b>SubcatchmentCB-5: NEW CB -</b>	Runoff Area=19,743 sf 50.83% Impervious Runoff Depth=2.01" Flow Length=574' Tc=6.0 min CN=86 Runoff=1.15 cfs 0.076 af
<b>Reach CB1: EX CB-1</b>	Inflow=2.15 cfs 0.143 af Outflow=2.15 cfs 0.143 af
<b>Reach CB10: EX-CB10</b>	Inflow=0.52 cfs 0.039 af Outflow=0.52 cfs 0.039 af
<b>Reach CB11: EX-CB11</b>	Inflow=1.15 cfs 0.076 af Outflow=1.15 cfs 0.076 af

**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 61

<b>Reach CB2: EX-CB2</b>	Inflow=1.56 cfs 0.106 af Outflow=1.56 cfs 0.106 af
<b>Reach CB3: EX-CB3</b>	Inflow=0.93 cfs 0.070 af Outflow=0.93 cfs 0.070 af
<b>Reach CB4: EX-CB4</b>	Inflow=1.23 cfs 0.082 af Outflow=1.23 cfs 0.082 af
<b>Reach CB5: EX-CB5</b>	Inflow=1.44 cfs 0.096 af Outflow=1.44 cfs 0.096 af
<b>Reach CB6: EX-CB6</b>	Inflow=2.69 cfs 0.195 af Outflow=2.69 cfs 0.195 af
<b>Reach CB7: EX-CB7</b>	Inflow=1.47 cfs 0.097 af Outflow=1.47 cfs 0.097 af
<b>Reach CB8: EX-CB8</b>	Inflow=8.63 cfs 0.568 af Outflow=8.63 cfs 0.568 af
<b>Reach CB9: EX CB-9</b>	Inflow=2.86 cfs 0.211 af Outflow=2.86 cfs 0.211 af
<b>Reach DP-1: French Rodney Blvd 14" Outfall</b>	Inflow=5.86 cfs 0.401 af Outflow=5.86 cfs 0.401 af
<b>Reach DP-2: PORTLANDSTREET DRAINAGE</b>	Inflow=18.76 cfs 1.282 af Outflow=18.76 cfs 1.282 af

Total Runoff Area = 8.690 ac Runoff Volume = 1.683 af Average Runoff Depth = 2.32"  
36.09% Pervious = 3.136 ac 63.91% Impervious = 5.554 ac

**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 62

**Summary for Subcatchment 1B: CB-1B (mix)**

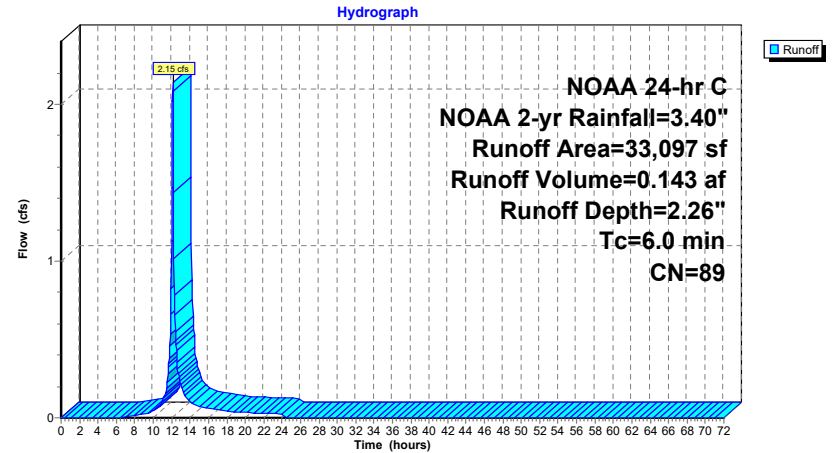
Runoff = 2.15 cfs @ 12.13 hrs, Volume= 0.143 af, Depth= 2.26"  
Routed to Reach CB1 : EX CB-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
20,636	83	1/4 acre lots, 38% imp, HSG C
12,461	98	Paved parking, HSG C
33,097	89	Weighted Average
12,794		38.66% Pervious Area
20,303		61.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 1B: CB-1B (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 63

**Summary for Subcatchment 2: CB-2 (mix)**

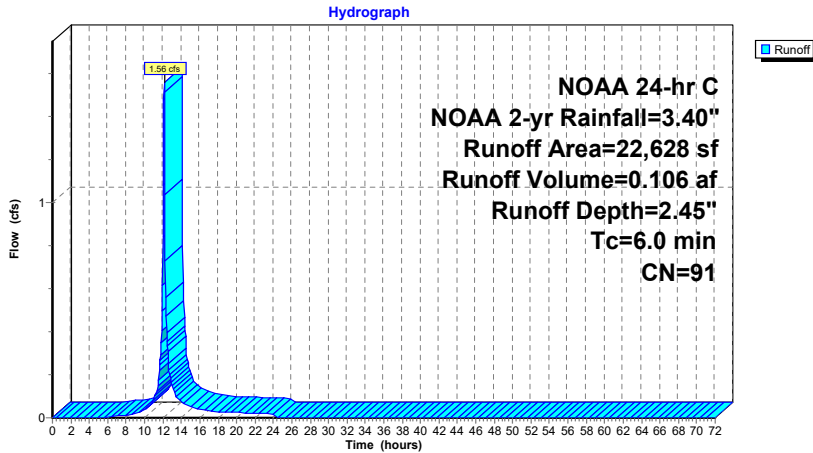
Runoff = 1.56 cfs @ 12.13 hrs, Volume= 0.106 af, Depth= 2.45"  
Routed to Reach CB2 : EX-CB2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
10,533	83	1/4 acre lots, 38% imp, HSG C
12,095	98	Paved parking, HSG C
22,628	91	Weighted Average
6,530		28.86% Pervious Area
16,098		71.14% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2: CB-2 (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 64

**Summary for Subcatchment 3A: CB-3A (parking)**

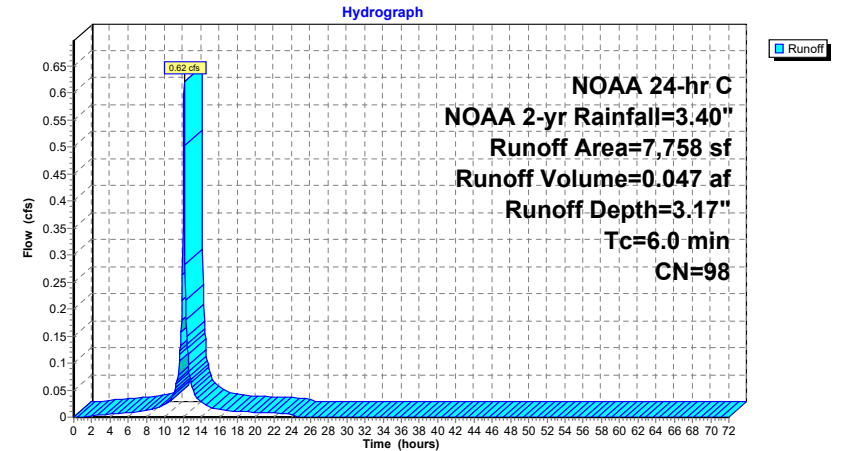
Runoff = 0.62 cfs @ 12.13 hrs, Volume= 0.047 af, Depth= 3.17"  
Routed to Reach CB3 : EX-CB3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
7,758	98	Paved parking, HSG C
7,758		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3A: CB-3A (parking)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 65

**Summary for Subcatchment 3B: CB-3B (parking)**

Runoff = 0.30 cfs @ 12.13 hrs, Volume= 0.023 af, Depth= 3.17"  
Routed to Reach CB3 : EX-CB3

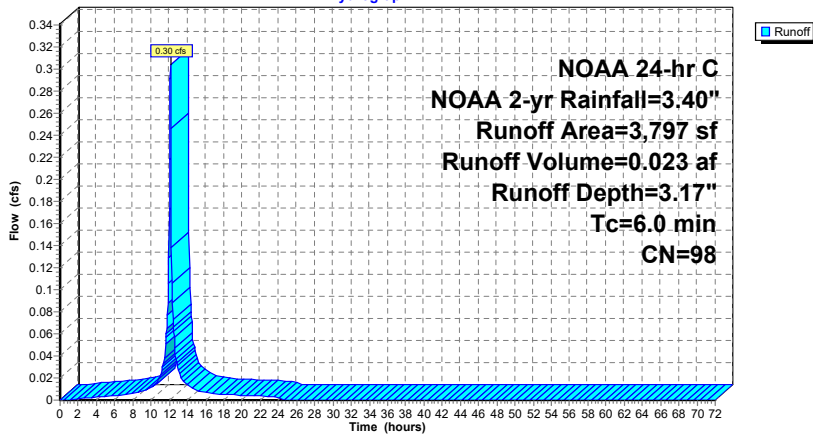
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
3,797	98	Paved parking, HSG C
3,797		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3B: CB-3B (parking)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 66

**Summary for Subcatchment 5B: CB-5B (mix)**

Runoff = 1.44 cfs @ 12.13 hrs, Volume= 0.096 af, Depth= 2.18"  
Routed to Reach CB5 : EX-CB5

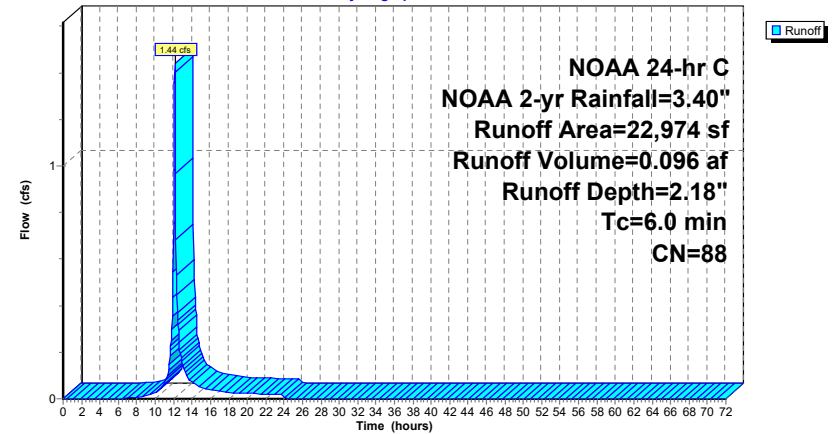
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
15,743	83	1/4 acre lots, 38% imp, HSG C
7,231	98	Paved parking, HSG C
22,974	88	Weighted Average
9,761		42.49% Pervious Area
13,213		57.51% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5B: CB-5B (mix)**

Hydrograph





**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 67

**Summary for Subcatchment 6: CB-6 (mix)**

Runoff = 2.69 cfs @ 12.13 hrs, Volume= 0.195 af, Depth= 2.95"  
Routed to Reach CB6 : EX-CB6

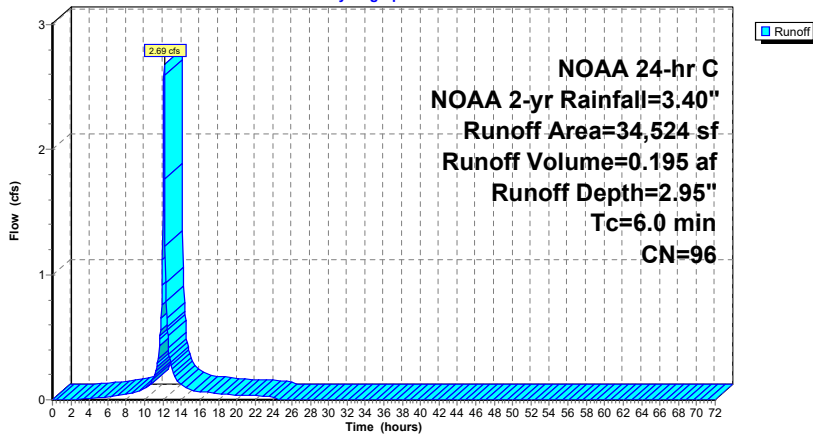
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
3,898	83	1/4 acre lots, 38% imp, HSG C
30,626	98	Paved parking, HSG C
34,524	96	Weighted Average
2,417		7.00% Pervious Area
32,107		93.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6: CB-6 (mix)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 68

**Summary for Subcatchment 8B: CB-8B (mix)**

Runoff = 1.73 cfs @ 12.13 hrs, Volume= 0.114 af, Depth= 2.09"  
Routed to Reach CB8 : EX-CB8

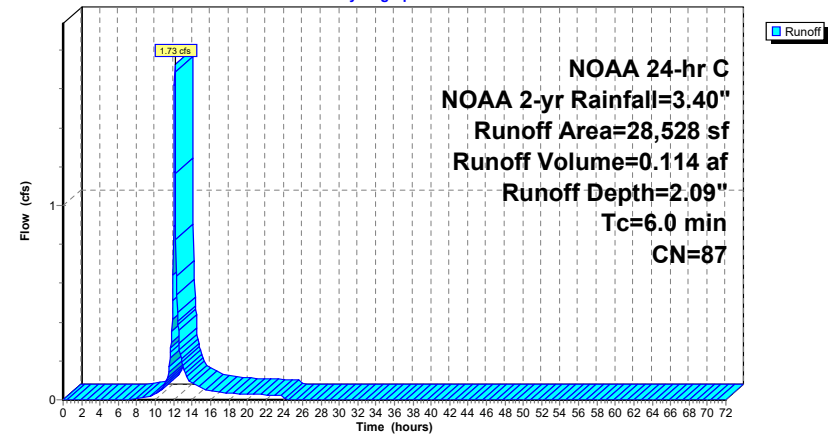
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
20,925	83	1/4 acre lots, 38% imp, HSG C
7,603	98	Paved parking, HSG C
28,528	87	Weighted Average
12,974		45.48% Pervious Area
15,555		54.52% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8B: CB-8B (mix)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 69

**Summary for Subcatchment 9: CB-9 (mix)**

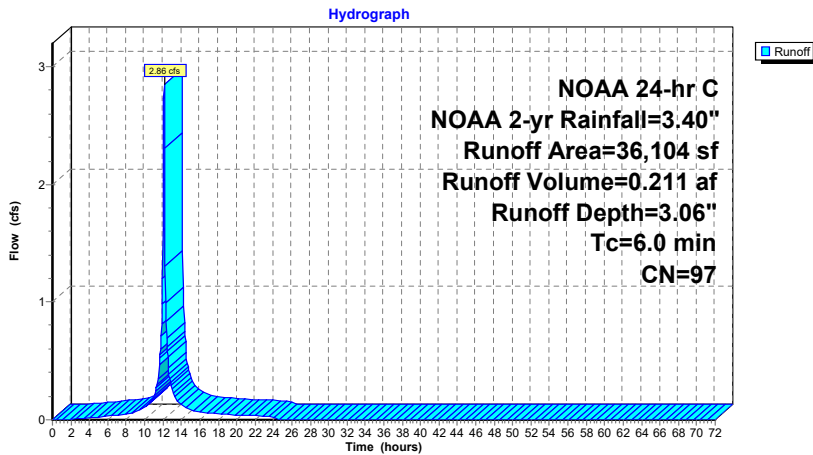
Runoff = 2.86 cfs @ 12.13 hrs, Volume= 0.211 af, Depth= 3.06"  
Routed to Reach CB9 : EX CB-9

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
3,474	83	1/4 acre lots, 38% imp, HSG C
32,630	98	Paved parking, HSG C
36,104	97	Weighted Average
2,154		5.97% Pervious Area
33,950		94.03% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 9: CB-9 (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 70

**Summary for Subcatchment 10: CB-10 (parking)**

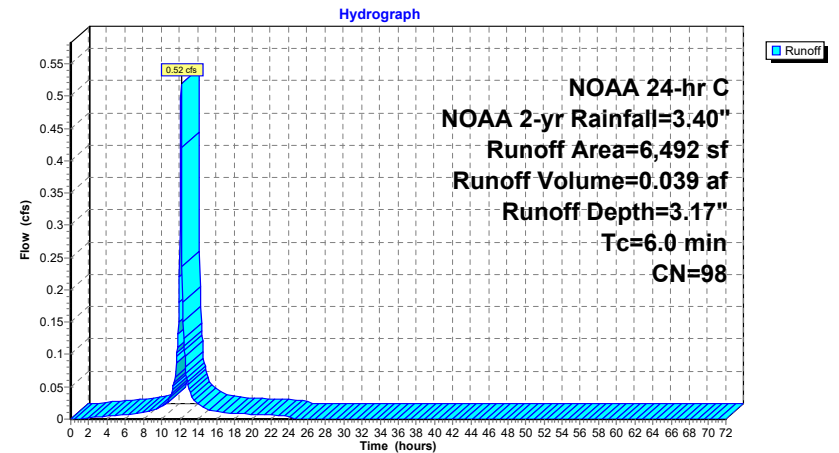
Runoff = 0.52 cfs @ 12.13 hrs, Volume= 0.039 af, Depth= 3.17"  
Routed to Reach CB10 : EX-CB10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
6,492	98	Paved parking, HSG C
6,492		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 10: CB-10 (parking)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 71

**Summary for Subcatchment CB-1: New CB South (Seymour Street)**

Runoff = 1.23 cfs @ 12.13 hrs, Volume= 0.082 af, Depth= 2.18"  
Routed to Reach CB4 : EX-CB4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

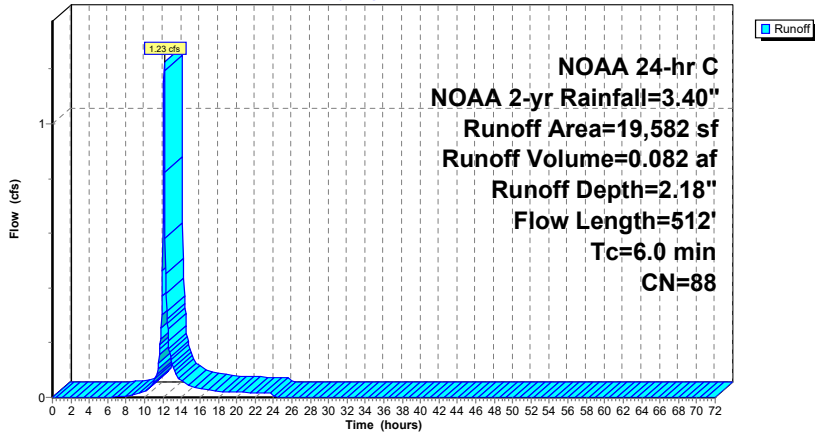
Area (sf)	CN	Description
13,211	83	1/4 acre lots, 38% imp, HSG C
* 6,371	98	Roadway
19,582	88	Weighted Average
8,191		41.83% Pervious Area
11,391		58.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	50	0.0300	1.45		Sheet Flow, A-B Smooth surfaces n= 0.011 P2= 3.40"
2.4	462	0.0249	3.20		Shallow Concentrated Flow, Paved Paved Kv= 20.3 fps
3.0					Direct Entry, Direct entry to 6
6.0	512	Total			

**Subcatchment CB-1: New CB South (Seymour Street)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 72

**Summary for Subcatchment CB-3: NEW CB SOUTH- HUDSON ST**

Runoff = 1.47 cfs @ 12.13 hrs, Volume= 0.097 af, Depth= 2.01"  
Routed to Reach CB7 : EX-CB7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

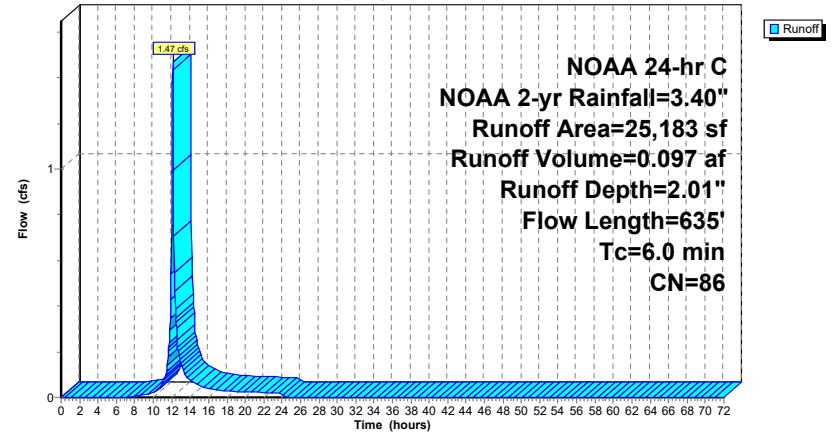
Area (sf)	CN	Description
19,562	83	1/4 acre lots, 38% imp, HSG C
* 5,621	98	Roadway
25,183	86	Weighted Average
12,128		48.16% Pervious Area
13,055		51.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0444	1.70		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
3.0	585	0.0256	3.25		Shallow Concentrated Flow, B-C Paved Kv= 20.3 fps
2.5					Direct Entry, direct entry to 6
6.0	635	Total			

**Subcatchment CB-3: NEW CB SOUTH- HUDSON ST**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 73

**Summary for Subcatchment CB-4: NEW CB NOTH - HUDSON STREET**

Runoff = 6.90 cfs @ 12.13 hrs, Volume= 0.454 af, Depth= 2.01"  
Routed to Reach CB8 : EX-CB8

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

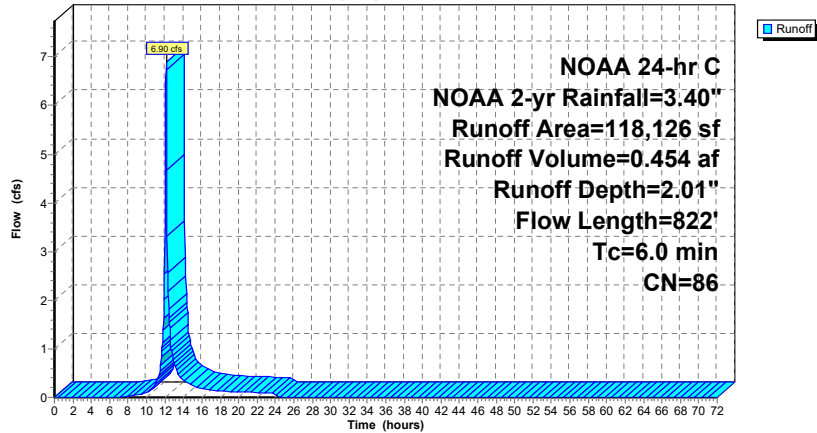
Area (sf)	CN	Description
96,716	83	1/4 acre lots, 38% imp, HSG C
* 21,410	98	Roadway
118,126	86	Weighted Average
59,964		50.76% Pervious Area
58,162		49.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
4.0	772	0.0245	3.18		Shallow Concentrated Flow, B-C (shallow concentrated) Paved Kv= 20.3 fps
1.5					Direct Entry, direct entry to 6
6.0	822	Total			

**Subcatchment CB-4: NEW CB NOTH - HUDSON STREET**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 74

**Summary for Subcatchment CB-5: NEW CB - PORTLAND ST SOUTH**

Runoff = 1.15 cfs @ 12.13 hrs, Volume= 0.076 af, Depth= 2.01"  
Routed to Reach CB11 : EX-CB11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

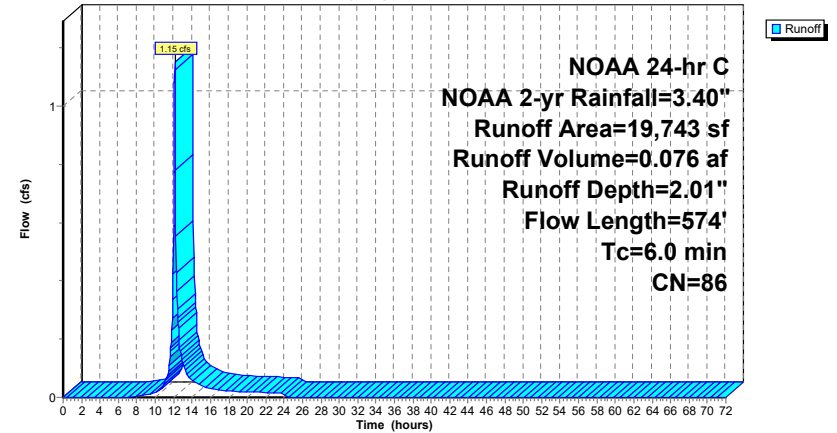
Area (sf)	CN	Description
15,657	83	1/4 acre lots, 38% imp, HSG C
* 4,086	98	Roadway
19,743	86	Weighted Average
9,707		49.17% Pervious Area
10,036		50.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
2.3	524	0.0346	3.78		Shallow Concentrated Flow, B-C (shallow conc.) Paved Kv= 20.3 fps
3.2					Direct Entry, direct to 6
6.0	574	Total			

**Subcatchment CB-5: NEW CB - PORTLAND ST SOUTH**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 75

**Summary for Reach CB1: EX CB-1**

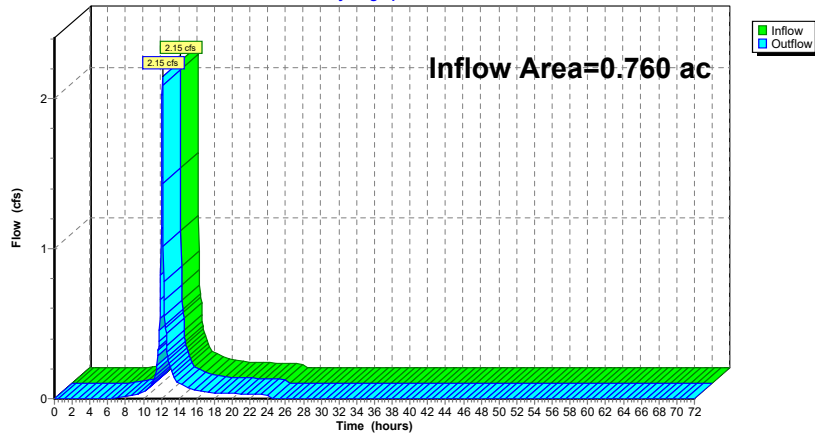
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.760 ac, 61.34% Impervious, Inflow Depth = 2.26" for NOAA 2-yr event  
Inflow = 2.15 cfs @ 12.13 hrs, Volume= 0.143 af  
Outflow = 2.15 cfs @ 12.13 hrs, Volume= 0.143 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB1: EX CB-1**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 76

**Summary for Reach CB10: EX-CB10**

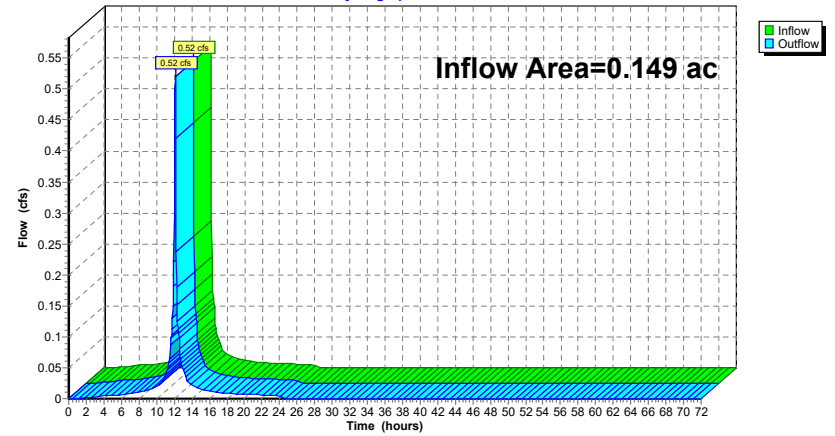
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.149 ac, 100.00% Impervious, Inflow Depth = 3.17" for NOAA 2-yr event  
Inflow = 0.52 cfs @ 12.13 hrs, Volume= 0.039 af  
Outflow = 0.52 cfs @ 12.13 hrs, Volume= 0.039 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB10: EX-CB10**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 77

**Summary for Reach CB11: EX-CB11**

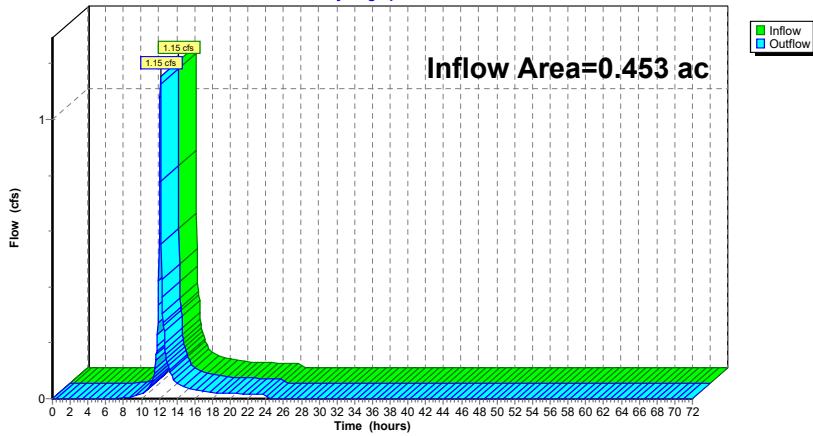
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.453 ac, 50.83% Impervious, Inflow Depth = 2.01" for NOAA 2-yr event  
Inflow = 1.15 cfs @ 12.13 hrs, Volume= 0.076 af  
Outflow = 1.15 cfs @ 12.13 hrs, Volume= 0.076 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB11: EX-CB11**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 78

**Summary for Reach CB2: EX-CB2**

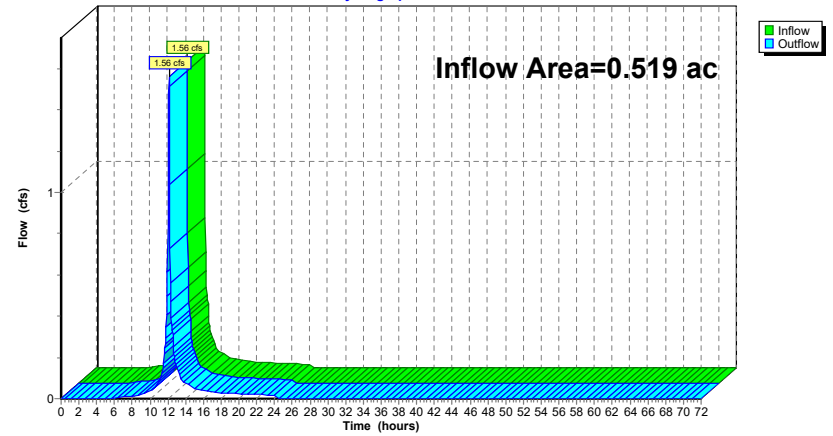
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.519 ac, 71.14% Impervious, Inflow Depth = 2.45" for NOAA 2-yr event  
Inflow = 1.56 cfs @ 12.13 hrs, Volume= 0.106 af  
Outflow = 1.56 cfs @ 12.13 hrs, Volume= 0.106 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB2: EX-CB2**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 79

**Summary for Reach CB3: EX-CB3**

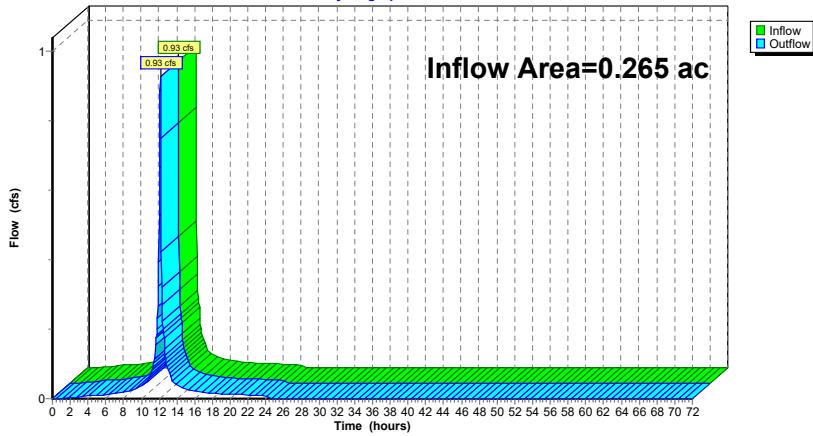
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.265 ac, 100.00% Impervious, Inflow Depth = 3.17" for NOAA 2-yr event  
Inflow = 0.93 cfs @ 12.13 hrs, Volume= 0.070 af  
Outflow = 0.93 cfs @ 12.13 hrs, Volume= 0.070 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB3: EX-CB3**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 80

**Summary for Reach CB4: EX-CB4**

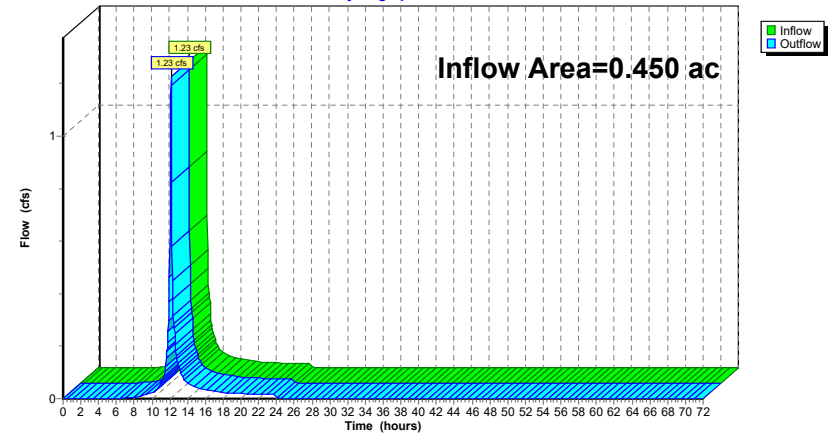
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.450 ac, 58.17% Impervious, Inflow Depth = 2.18" for NOAA 2-yr event  
Inflow = 1.23 cfs @ 12.13 hrs, Volume= 0.082 af  
Outflow = 1.23 cfs @ 12.13 hrs, Volume= 0.082 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB4: EX-CB4**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 81

**Summary for Reach CB5: EX-CB5**

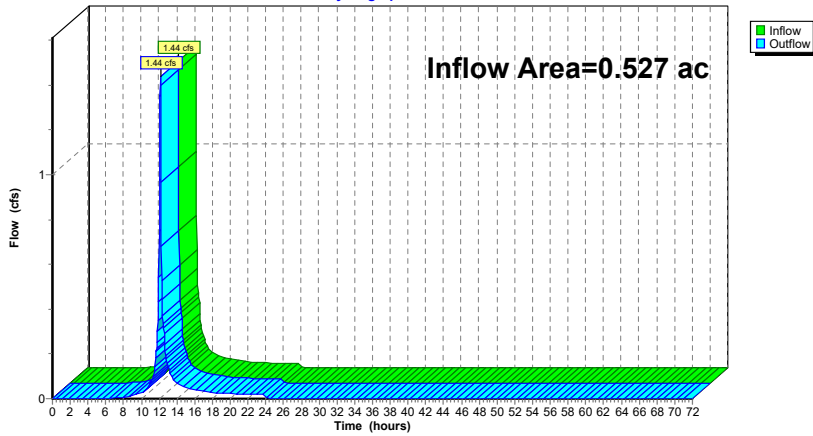
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.527 ac, 57.51% Impervious, Inflow Depth = 2.18" for NOAA 2-yr event  
Inflow = 1.44 cfs @ 12.13 hrs, Volume= 0.096 af  
Outflow = 1.44 cfs @ 12.13 hrs, Volume= 0.096 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB5: EX-CB5**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 82

**Summary for Reach CB6: EX-CB6**

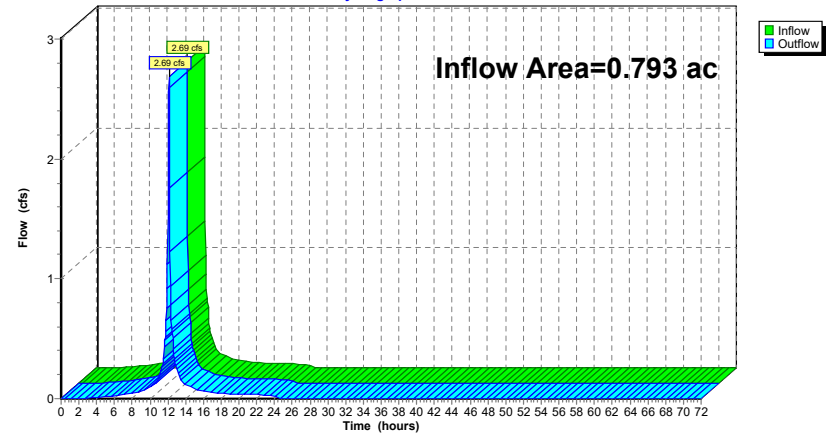
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.793 ac, 93.00% Impervious, Inflow Depth = 2.95" for NOAA 2-yr event  
Inflow = 2.69 cfs @ 12.13 hrs, Volume= 0.195 af  
Outflow = 2.69 cfs @ 12.13 hrs, Volume= 0.195 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB6: EX-CB6**

Hydrograph





**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 83

**Summary for Reach CB7: EX-CB7**

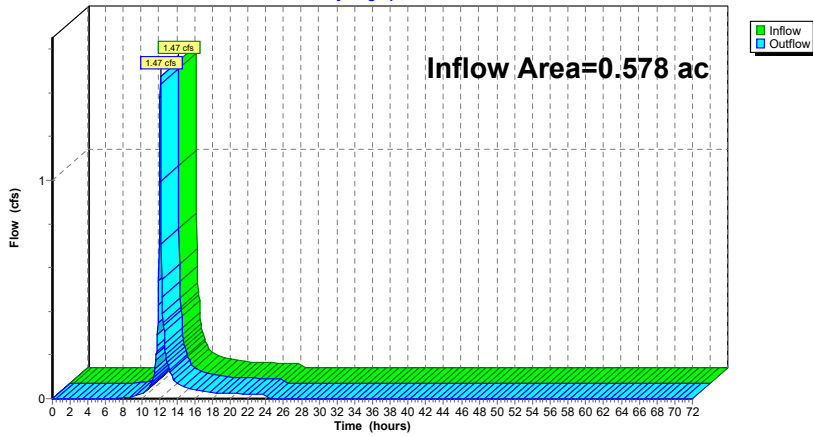
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.578 ac, 51.84% Impervious, Inflow Depth = 2.01" for NOAA 2-yr event  
Inflow = 1.47 cfs @ 12.13 hrs, Volume= 0.097 af  
Outflow = 1.47 cfs @ 12.13 hrs, Volume= 0.097 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB7: EX-CB7**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 84

**Summary for Reach CB8: EX-CB8**

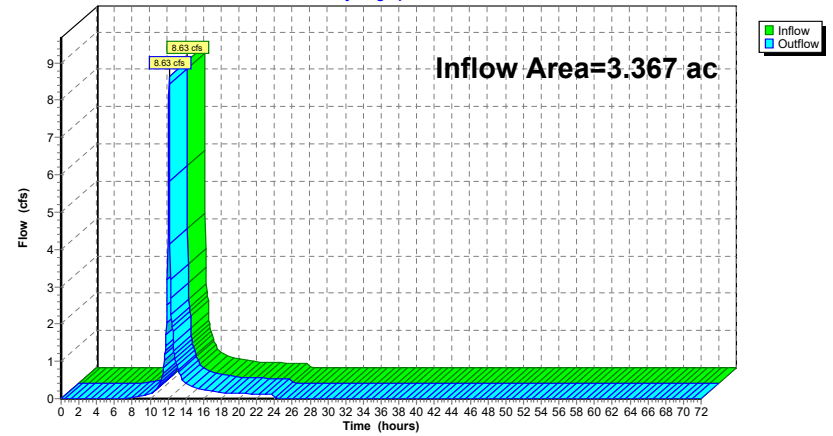
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.367 ac, 50.27% Impervious, Inflow Depth = 2.03" for NOAA 2-yr event  
Inflow = 8.63 cfs @ 12.13 hrs, Volume= 0.568 af  
Outflow = 8.63 cfs @ 12.13 hrs, Volume= 0.568 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB8: EX-CB8**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 85

**Summary for Reach CB9: EX CB-9**

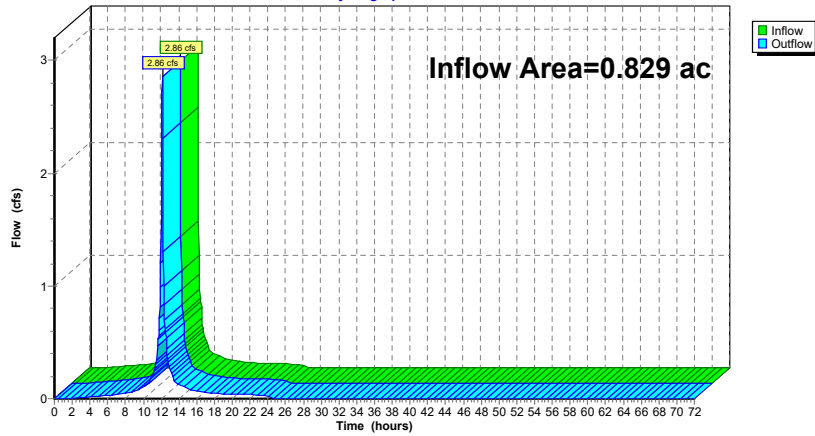
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.829 ac, 94.03% Impervious, Inflow Depth = 3.06" for NOAA 2-yr event  
Inflow = 2.86 cfs @ 12.13 hrs, Volume= 0.211 af  
Outflow = 2.86 cfs @ 12.13 hrs, Volume= 0.211 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB9: EX CB-9**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 86

**Summary for Reach DP-1: French Rodney Blvd 14" Outfall**

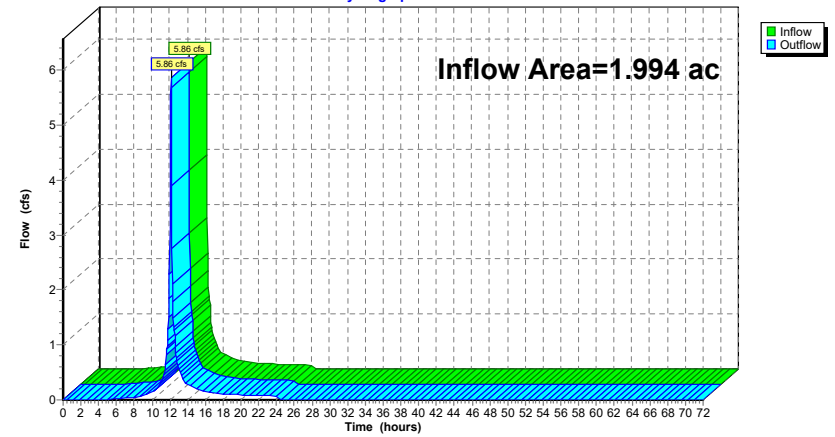
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.994 ac, 68.32% Impervious, Inflow Depth = 2.41" for NOAA 2-yr event  
Inflow = 5.86 cfs @ 12.13 hrs, Volume= 0.401 af  
Outflow = 5.86 cfs @ 12.13 hrs, Volume= 0.401 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-1: French Rodney Blvd 14" Outfall**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

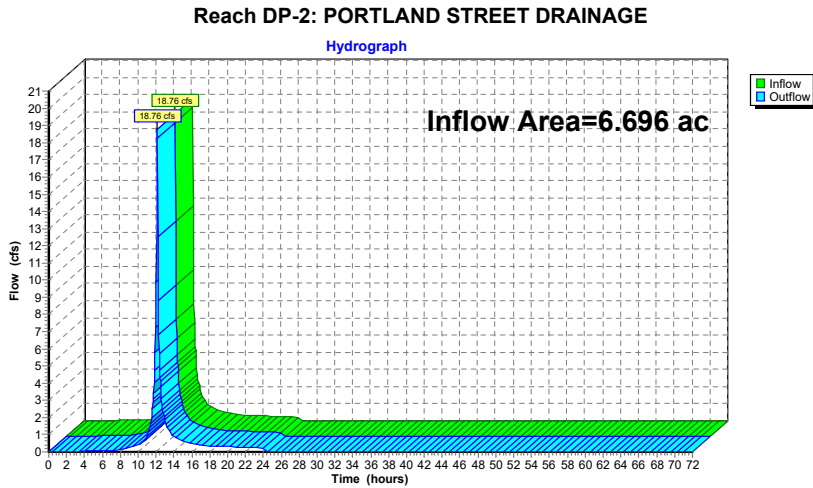
Page 87

**Summary for Reach DP-2: PORTLAND STREET DRAINAGE**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.696 ac, 62.59% Impervious, Inflow Depth = 2.30" for NOAA 2-yr event  
 Inflow = 18.76 cfs @ 12.13 hrs, Volume= 1.282 af  
 Outflow = 18.76 cfs @ 12.13 hrs, Volume= 1.282 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 88

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1B: CB-1B (mix)</b>	Runoff Area=33,097 sf 61.34% Impervious Runoff Depth=4.77" Tc=6.0 min CN=89 Runoff=4.34 cfs 0.302 af
<b>Subcatchment2: CB-2 (mix)</b>	Runoff Area=22,628 sf 71.14% Impervious Runoff Depth=5.00" Tc=6.0 min CN=91 Runoff=3.05 cfs 0.216 af
<b>Subcatchment3A: CB-3A (parking)</b>	Runoff Area=7,758 sf 100.00% Impervious Runoff Depth=5.80" Tc=6.0 min CN=98 Runoff=1.11 cfs 0.086 af
<b>Subcatchment3B: CB-3B (parking)</b>	Runoff Area=3,797 sf 100.00% Impervious Runoff Depth=5.80" Tc=6.0 min CN=98 Runoff=0.54 cfs 0.042 af
<b>Subcatchment5B: CB-5B (mix)</b>	Runoff Area=22,974 sf 57.51% Impervious Runoff Depth=4.66" Tc=6.0 min CN=88 Runoff=2.97 cfs 0.205 af
<b>Subcatchment6: CB-6 (mix)</b>	Runoff Area=34,524 sf 93.00% Impervious Runoff Depth=5.57" Tc=6.0 min CN=96 Runoff=4.90 cfs 0.368 af
<b>Subcatchment8B: CB-8B (mix)</b>	Runoff Area=28,528 sf 54.52% Impervious Runoff Depth=4.56" Tc=6.0 min CN=87 Runoff=3.62 cfs 0.249 af
<b>Subcatchment9: CB-9 (mix)</b>	Runoff Area=36,104 sf 94.03% Impervious Runoff Depth=5.68" Tc=6.0 min CN=97 Runoff=5.15 cfs 0.393 af
<b>Subcatchment10: CB-10 (parking)</b>	Runoff Area=6,492 sf 100.00% Impervious Runoff Depth=5.80" Tc=6.0 min CN=98 Runoff=0.93 cfs 0.072 af
<b>SubcatchmentCB-1: New CB South</b>	Runoff Area=19,582 sf 58.17% Impervious Runoff Depth=4.66" Flow Length=512' Tc=6.0 min CN=88 Runoff=2.53 cfs 0.175 af
<b>SubcatchmentCB-3: NEW CB SOUTH-</b>	Runoff Area=25,183 sf 51.84% Impervious Runoff Depth=4.45" Flow Length=635' Tc=6.0 min CN=86 Runoff=3.14 cfs 0.214 af
<b>SubcatchmentCB-4: NEW CB NOTH -</b>	Runoff Area=118,126 sf 49.24% Impervious Runoff Depth=4.45" Flow Length=822' Tc=6.0 min CN=86 Runoff=14.73 cfs 1.005 af
<b>SubcatchmentCB-5: NEW CB -</b>	Runoff Area=19,743 sf 50.83% Impervious Runoff Depth=4.45" Flow Length=574' Tc=6.0 min CN=86 Runoff=2.46 cfs 0.168 af
<b>Reach CB1: EX CB-1</b>	Inflow=4.34 cfs 0.302 af Outflow=4.34 cfs 0.302 af
<b>Reach CB10: EX-CB10</b>	Inflow=0.93 cfs 0.072 af Outflow=0.93 cfs 0.072 af
<b>Reach CB11: EX-CB11</b>	Inflow=2.46 cfs 0.168 af Outflow=2.46 cfs 0.168 af

**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 89

<b>Reach CB2: EX-CB2</b>	Inflow=3.05 cfs 0.216 af Outflow=3.05 cfs 0.216 af
<b>Reach CB3: EX-CB3</b>	Inflow=1.66 cfs 0.128 af Outflow=1.66 cfs 0.128 af
<b>Reach CB4: EX-CB4</b>	Inflow=2.53 cfs 0.175 af Outflow=2.53 cfs 0.175 af
<b>Reach CB5: EX-CB5</b>	Inflow=2.97 cfs 0.205 af Outflow=2.97 cfs 0.205 af
<b>Reach CB6: EX-CB6</b>	Inflow=4.90 cfs 0.368 af Outflow=4.90 cfs 0.368 af
<b>Reach CB7: EX-CB7</b>	Inflow=3.14 cfs 0.214 af Outflow=3.14 cfs 0.214 af
<b>Reach CB8: EX-CB8</b>	Inflow=18.36 cfs 1.254 af Outflow=18.36 cfs 1.254 af
<b>Reach CB9: EX CB-9</b>	Inflow=5.15 cfs 0.393 af Outflow=5.15 cfs 0.393 af
<b>Reach DP-1: French Rodney Blvd 14" Outfall</b>	Inflow=11.57 cfs 0.822 af Outflow=11.57 cfs 0.822 af
<b>Reach DP-2: PORTLANDSTREET DRAINAGE</b>	Inflow=37.90 cfs 2.673 af Outflow=37.90 cfs 2.673 af

Total Runoff Area = 8.690 ac Runoff Volume = 3.495 af Average Runoff Depth = 4.83"  
36.09% Pervious = 3.136 ac 63.91% Impervious = 5.554 ac

**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 90

**Summary for Subcatchment 1B: CB-1B (mix)**

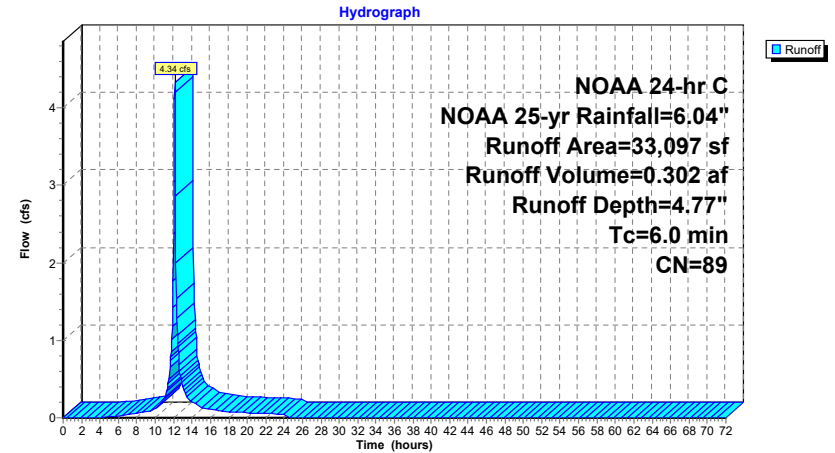
Runoff = 4.34 cfs @ 12.13 hrs, Volume= 0.302 af, Depth= 4.77"  
Routed to Reach CB1 : EX CB-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
20,636	83	1/4 acre lots, 38% imp, HSG C
12,461	98	Paved parking, HSG C
33,097	89	Weighted Average
12,794		38.66% Pervious Area
20,303		61.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 1B: CB-1B (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 91

**Summary for Subcatchment 2: CB-2 (mix)**

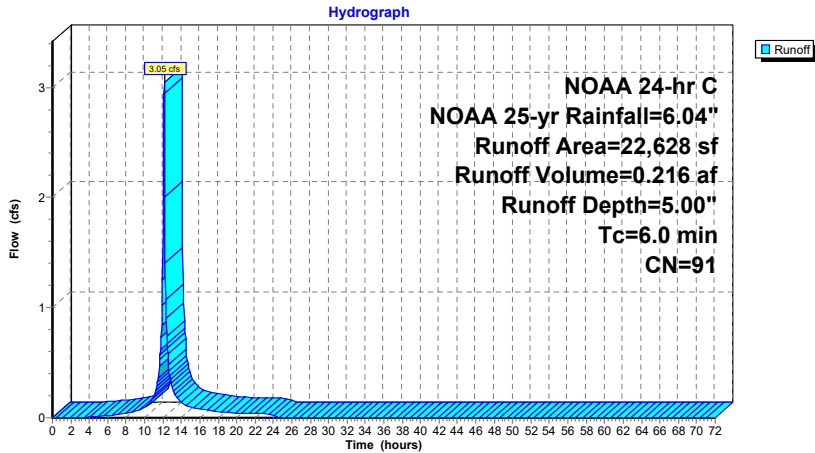
Runoff = 3.05 cfs @ 12.13 hrs, Volume= 0.216 af, Depth= 5.00"  
Routed to Reach CB2 : EX-CB2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
10,533	83	1/4 acre lots, 38% imp, HSG C
12,095	98	Paved parking, HSG C
22,628	91	Weighted Average
6,530		28.86% Pervious Area
16,098		71.14% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2: CB-2 (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 92

**Summary for Subcatchment 3A: CB-3A (parking)**

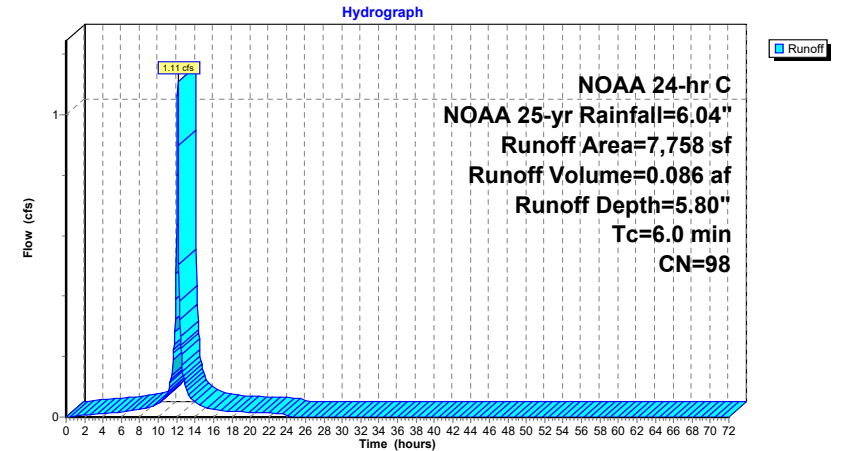
Runoff = 1.11 cfs @ 12.13 hrs, Volume= 0.086 af, Depth= 5.80"  
Routed to Reach CB3 : EX-CB3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
7,758	98	Paved parking, HSG C
7,758		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3A: CB-3A (parking)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 93

**Summary for Subcatchment 3B: CB-3B (parking)**

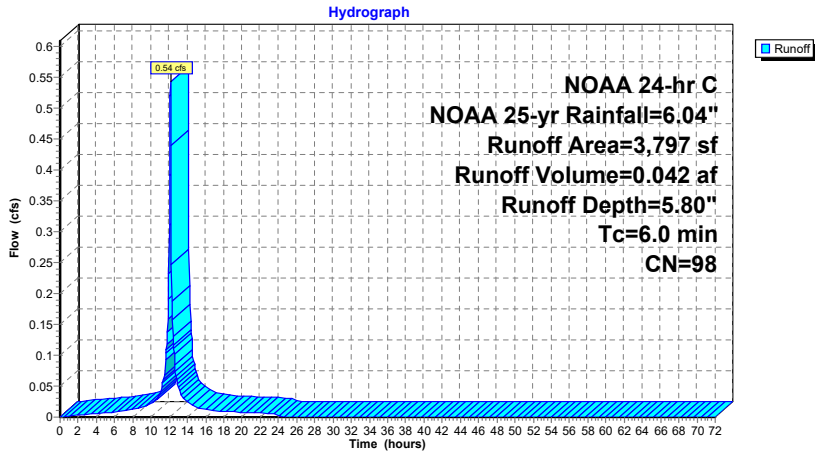
Runoff = 0.54 cfs @ 12.13 hrs, Volume= 0.042 af, Depth= 5.80"  
Routed to Reach CB3 : EX-CB3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
3,797	98	Paved parking, HSG C
3,797		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3B: CB-3B (parking)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 94

**Summary for Subcatchment 5B: CB-5B (mix)**

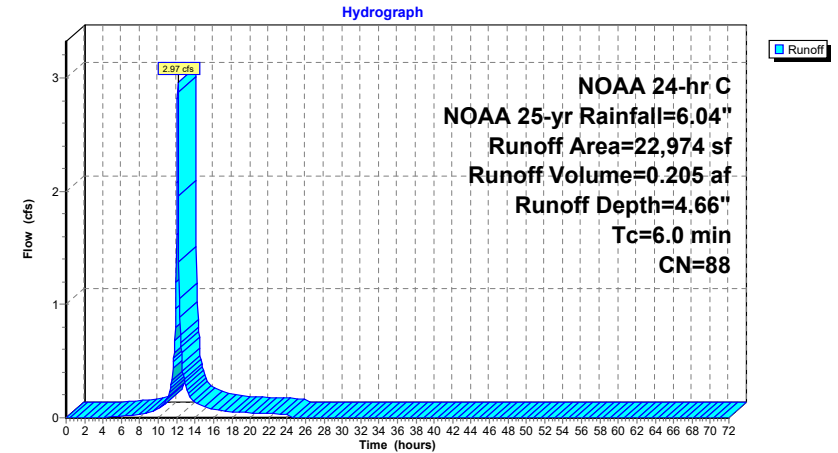
Runoff = 2.97 cfs @ 12.13 hrs, Volume= 0.205 af, Depth= 4.66"  
Routed to Reach CB5 : EX-CB5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
15,743	83	1/4 acre lots, 38% imp, HSG C
7,231	98	Paved parking, HSG C
22,974	88	Weighted Average
9,761		42.49% Pervious Area
13,213		57.51% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5B: CB-5B (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 95

**Summary for Subcatchment 6: CB-6 (mix)**

Runoff = 4.90 cfs @ 12.13 hrs, Volume= 0.368 af, Depth= 5.57"  
Routed to Reach CB6 : EX-CB6

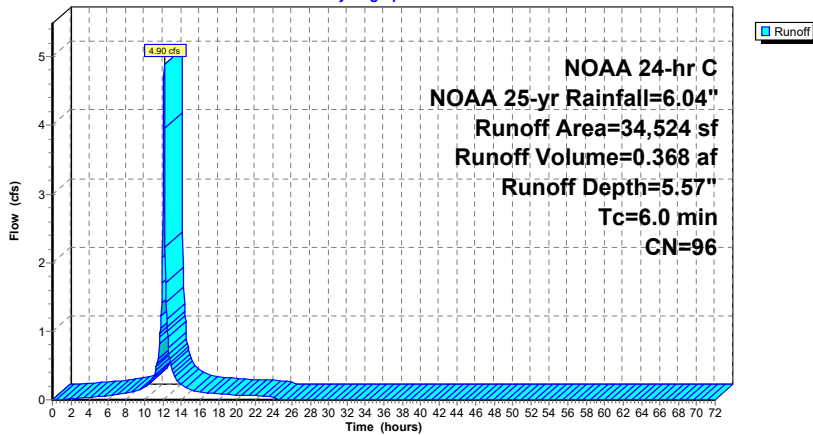
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
3,898	83	1/4 acre lots, 38% imp, HSG C
30,626	98	Paved parking, HSG C
34,524	96	Weighted Average
2,417		7.00% Pervious Area
32,107		93.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6: CB-6 (mix)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 96

**Summary for Subcatchment 8B: CB-8B (mix)**

Runoff = 3.62 cfs @ 12.13 hrs, Volume= 0.249 af, Depth= 4.56"  
Routed to Reach CB8 : EX-CB8

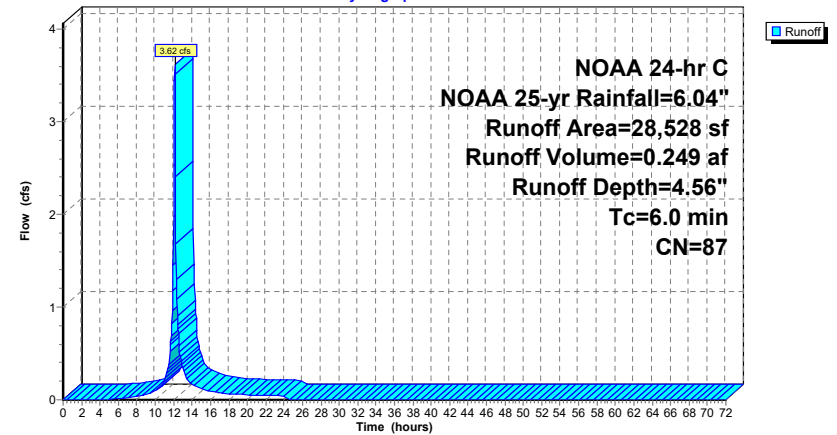
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
20,925	83	1/4 acre lots, 38% imp, HSG C
7,603	98	Paved parking, HSG C
28,528	87	Weighted Average
12,974		45.48% Pervious Area
15,555		54.52% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8B: CB-8B (mix)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 97

**Summary for Subcatchment 9: CB-9 (mix)**

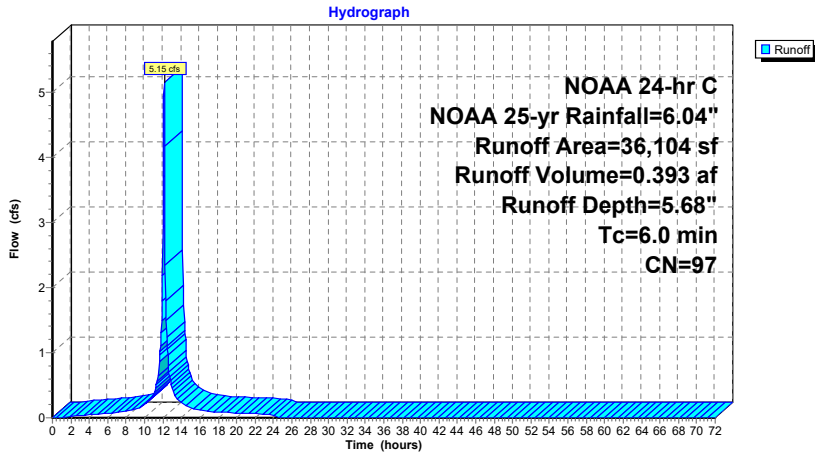
Runoff = 5.15 cfs @ 12.13 hrs, Volume= 0.393 af, Depth= 5.68"  
Routed to Reach CB9 : EX CB-9

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
3,474	83	1/4 acre lots, 38% imp, HSG C
32,630	98	Paved parking, HSG C
36,104	97	Weighted Average
2,154		5.97% Pervious Area
33,950		94.03% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 9: CB-9 (mix)**



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 98

**Summary for Subcatchment 10: CB-10 (parking)**

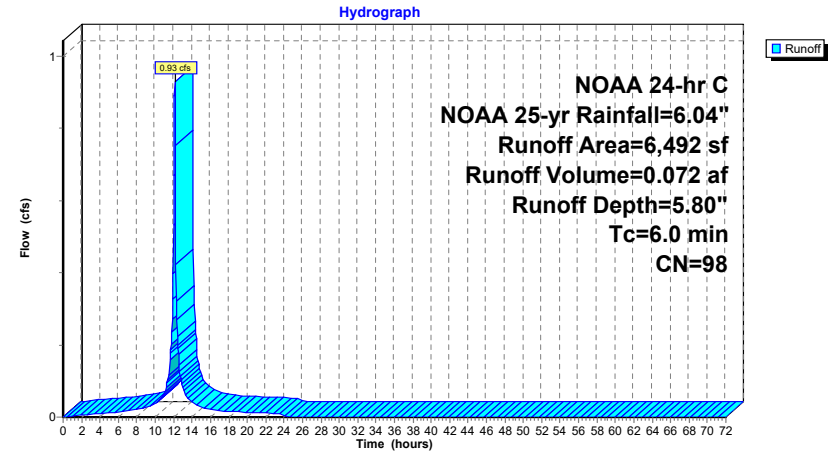
Runoff = 0.93 cfs @ 12.13 hrs, Volume= 0.072 af, Depth= 5.80"  
Routed to Reach CB10 : EX-CB10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
6,492	98	Paved parking, HSG C
6,492		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 10: CB-10 (parking)**





**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 99

**Summary for Subcatchment CB-1: New CB South (Seymour Street)**

Runoff = 2.53 cfs @ 12.13 hrs, Volume= 0.175 af, Depth= 4.66"  
Routed to Reach CB4 : EX-CB4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

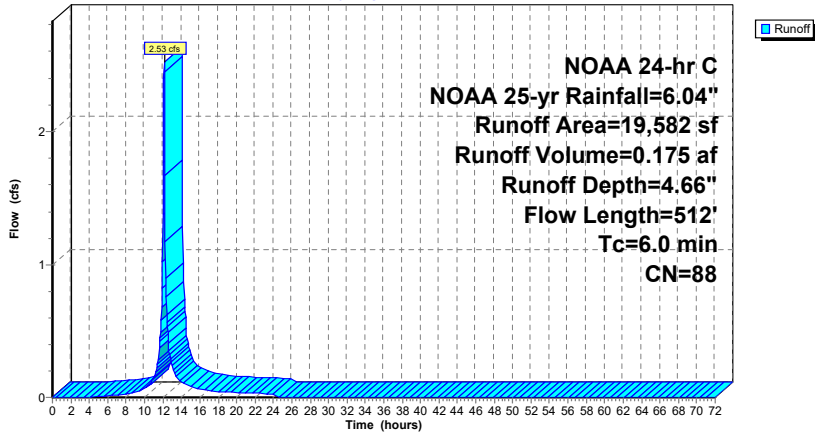
Area (sf)	CN	Description
13,211	83	1/4 acre lots, 38% imp, HSG C
* 6,371	98	Roadway
19,582	88	Weighted Average
8,191		41.83% Pervious Area
11,391		58.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	50	0.0300	1.45		<b>Sheet Flow, A-B</b> Smooth surfaces n= 0.011 P2= 3.40"
2.4	462	0.0249	3.20		<b>Shallow Concentrated Flow, Paved</b> Paved Kv= 20.3 fps
3.0					<b>Direct Entry, Direct entry to 6</b>
6.0	512	Total			

**Subcatchment CB-1: New CB South (Seymour Street)**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 100

**Summary for Subcatchment CB-3: NEW CB SOUTH- HUDSON ST**

Runoff = 3.14 cfs @ 12.13 hrs, Volume= 0.214 af, Depth= 4.45"  
Routed to Reach CB7 : EX-CB7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

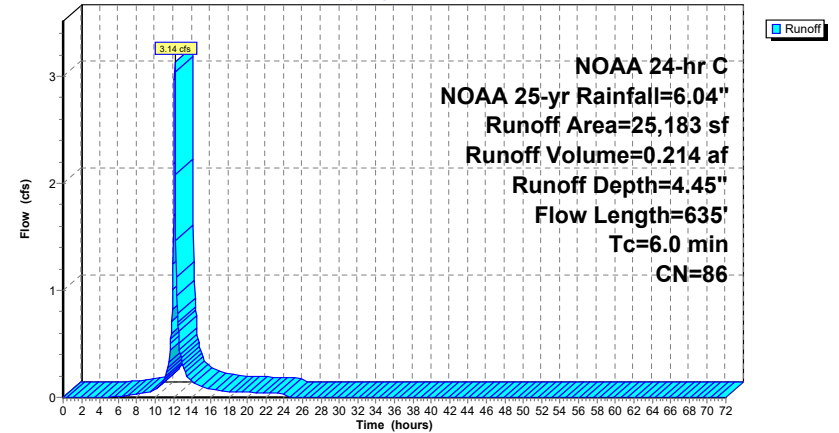
Area (sf)	CN	Description
19,562	83	1/4 acre lots, 38% imp, HSG C
* 5,621	98	Roadway
25,183	86	Weighted Average
12,128		48.16% Pervious Area
13,055		51.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0444	1.70		<b>Sheet Flow, A-B (sheet flow)</b> Smooth surfaces n= 0.011 P2= 3.40"
3.0	585	0.0256	3.25		<b>Shallow Concentrated Flow, B-C</b> Paved Kv= 20.3 fps
2.5					<b>Direct Entry, direct entry to 6</b>
6.0	635	Total			

**Subcatchment CB-3: NEW CB SOUTH- HUDSON ST**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 101

**Summary for Subcatchment CB-4: NEW CB NOTH - HUDSON STREET**

Runoff = 14.73 cfs @ 12.13 hrs, Volume= 1.005 af, Depth= 4.45"  
Routed to Reach CB8 : EX-CB8

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

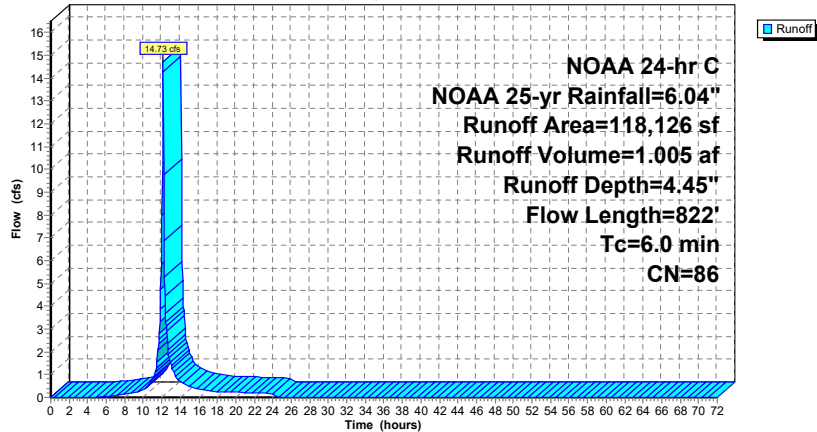
Area (sf)	CN	Description
96,716	83	1/4 acre lots, 38% imp, HSG C
* 21,410	98	Roadway
118,126	86	Weighted Average
59,964		50.76% Pervious Area
58,162		49.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
4.0	772	0.0245	3.18		Shallow Concentrated Flow, B-C (shallow concentrated) Paved Kv= 20.3 fps
1.5					Direct Entry, direct entry to 6
6.0	822	Total			

**Subcatchment CB-4: NEW CB NOTH - HUDSON STREET**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 102

**Summary for Subcatchment CB-5: NEW CB - PORTLAND ST SOUTH**

Runoff = 2.46 cfs @ 12.13 hrs, Volume= 0.168 af, Depth= 4.45"  
Routed to Reach CB11 : EX-CB11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

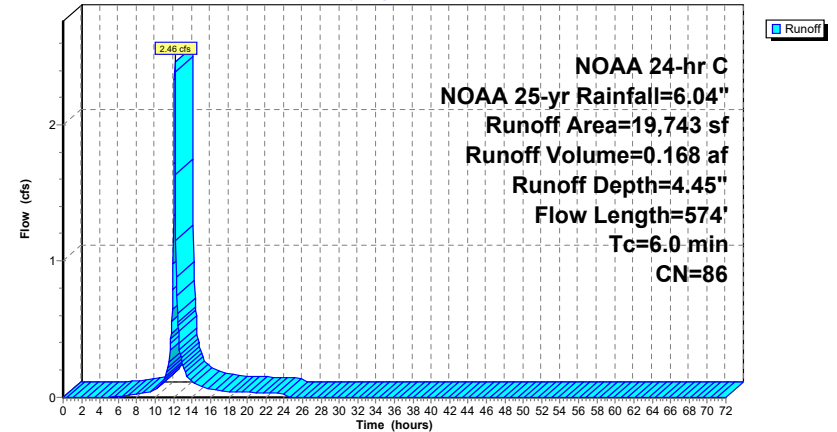
Area (sf)	CN	Description
15,657	83	1/4 acre lots, 38% imp, HSG C
* 4,086	98	Roadway
19,743	86	Weighted Average
9,707		49.17% Pervious Area
10,036		50.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
2.3	524	0.0346	3.78		Shallow Concentrated Flow, B-C (shallow conc.) Paved Kv= 20.3 fps
3.2					Direct Entry, direct to 6
6.0	574	Total			

**Subcatchment CB-5: NEW CB - PORTLAND ST SOUTH**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 103

**Summary for Reach CB1: EX CB-1**

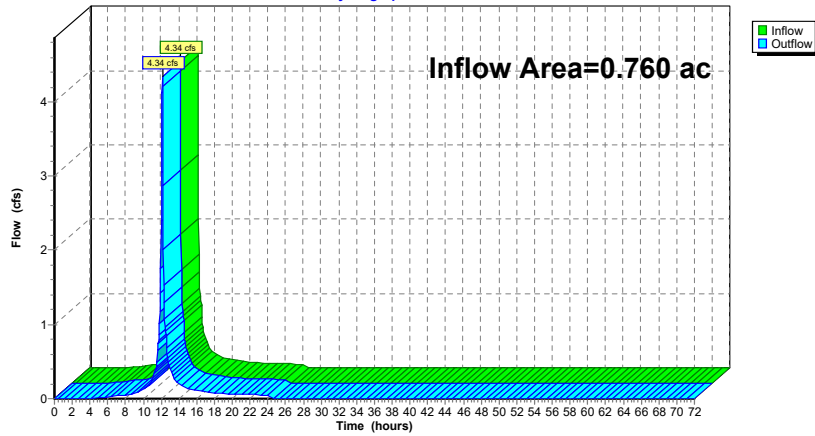
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.760 ac, 61.34% Impervious, Inflow Depth = 4.77" for NOAA 25-yr event  
Inflow = 4.34 cfs @ 12.13 hrs, Volume= 0.302 af  
Outflow = 4.34 cfs @ 12.13 hrs, Volume= 0.302 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB1: EX CB-1**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 104

**Summary for Reach CB10: EX-CB10**

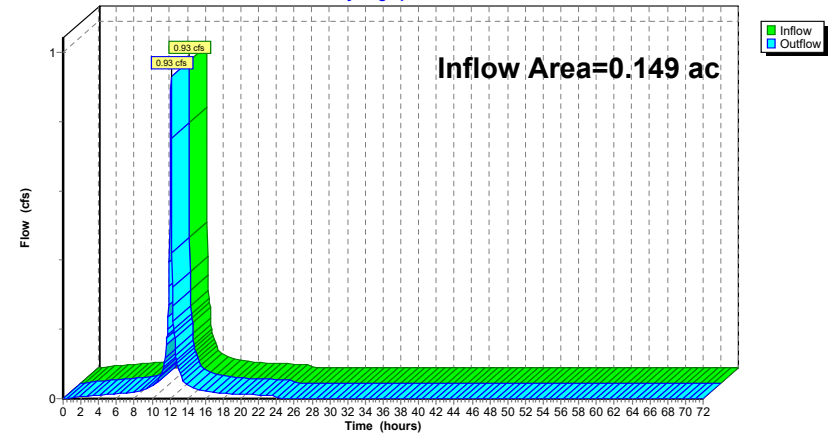
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.149 ac, 100.00% Impervious, Inflow Depth = 5.80" for NOAA 25-yr event  
Inflow = 0.93 cfs @ 12.13 hrs, Volume= 0.072 af  
Outflow = 0.93 cfs @ 12.13 hrs, Volume= 0.072 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB10: EX-CB10**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 105

**Summary for Reach CB11: EX-CB11**

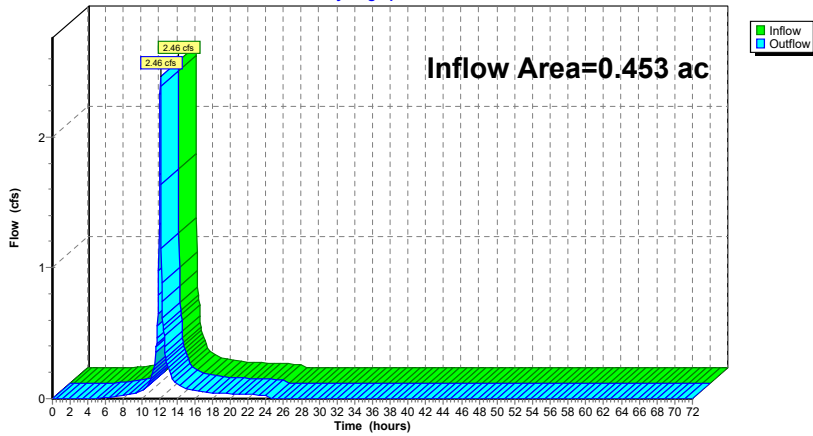
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.453 ac, 50.83% Impervious, Inflow Depth = 4.45" for NOAA 25-yr event  
Inflow = 2.46 cfs @ 12.13 hrs, Volume= 0.168 af  
Outflow = 2.46 cfs @ 12.13 hrs, Volume= 0.168 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB11: EX-CB11**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 106

**Summary for Reach CB2: EX-CB2**

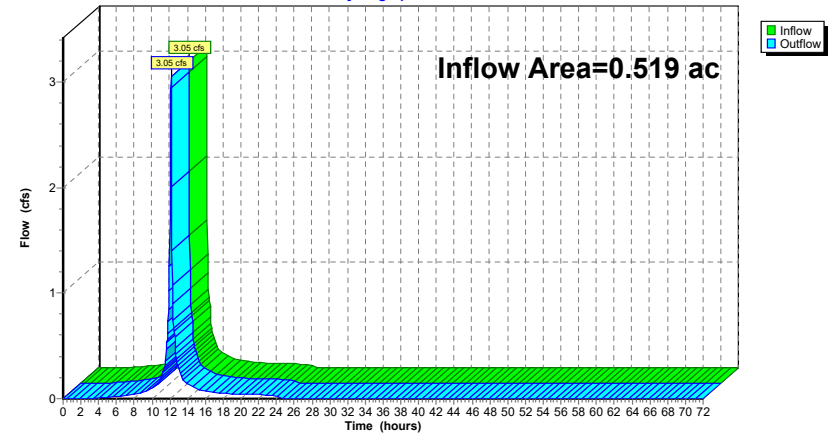
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.519 ac, 71.14% Impervious, Inflow Depth = 5.00" for NOAA 25-yr event  
Inflow = 3.05 cfs @ 12.13 hrs, Volume= 0.216 af  
Outflow = 3.05 cfs @ 12.13 hrs, Volume= 0.216 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB2: EX-CB2**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 107

**Summary for Reach CB3: EX-CB3**

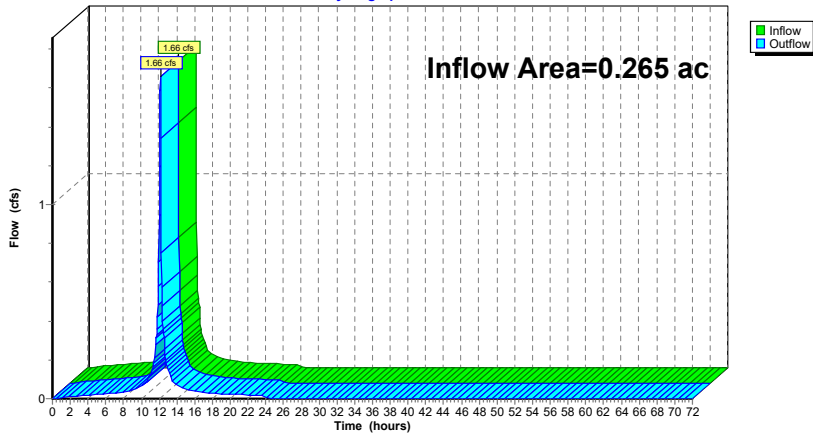
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.265 ac, 100.00% Impervious, Inflow Depth = 5.80" for NOAA 25-yr event  
Inflow = 1.66 cfs @ 12.13 hrs, Volume= 0.128 af  
Outflow = 1.66 cfs @ 12.13 hrs, Volume= 0.128 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB3: EX-CB3**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 108

**Summary for Reach CB4: EX-CB4**

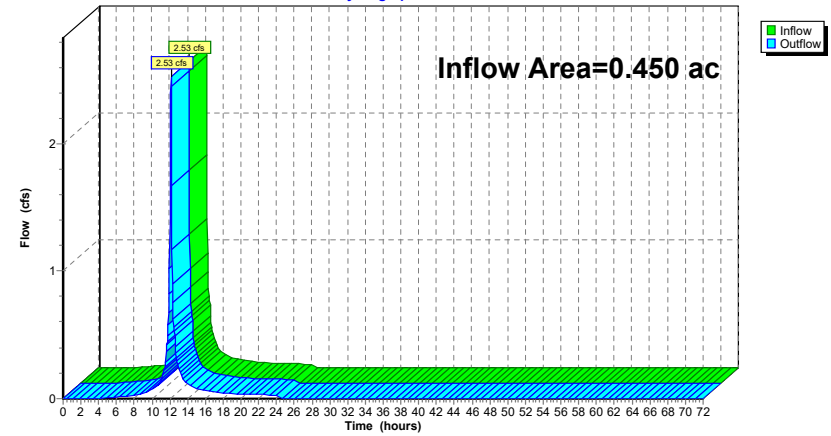
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.450 ac, 58.17% Impervious, Inflow Depth = 4.66" for NOAA 25-yr event  
Inflow = 2.53 cfs @ 12.13 hrs, Volume= 0.175 af  
Outflow = 2.53 cfs @ 12.13 hrs, Volume= 0.175 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB4: EX-CB4**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 109

**Summary for Reach CB5: EX-CB5**

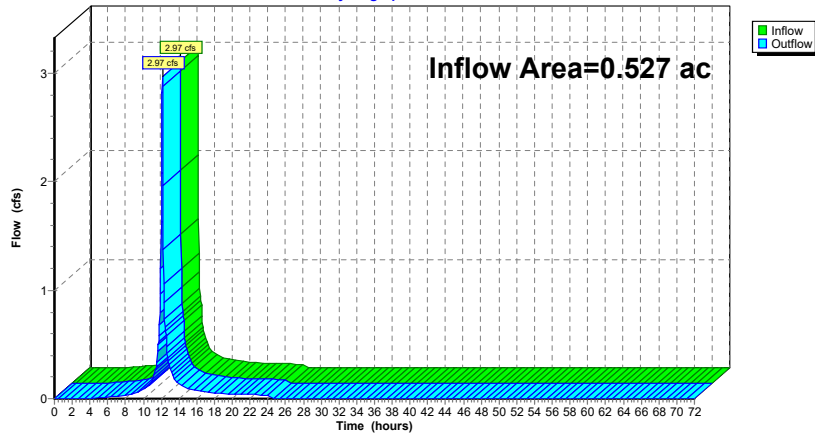
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.527 ac, 57.51% Impervious, Inflow Depth = 4.66" for NOAA 25-yr event  
Inflow = 2.97 cfs @ 12.13 hrs, Volume= 0.205 af  
Outflow = 2.97 cfs @ 12.13 hrs, Volume= 0.205 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB5: EX-CB5**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 110

**Summary for Reach CB6: EX-CB6**

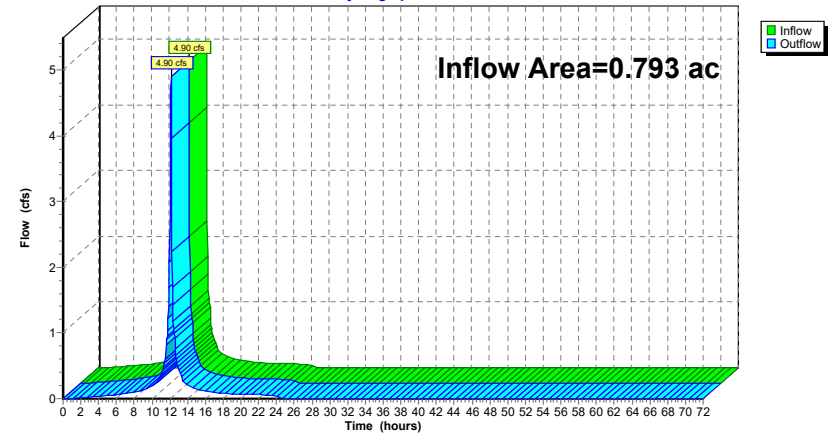
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.793 ac, 93.00% Impervious, Inflow Depth = 5.57" for NOAA 25-yr event  
Inflow = 4.90 cfs @ 12.13 hrs, Volume= 0.368 af  
Outflow = 4.90 cfs @ 12.13 hrs, Volume= 0.368 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB6: EX-CB6**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 111

**Summary for Reach CB7: EX-CB7**

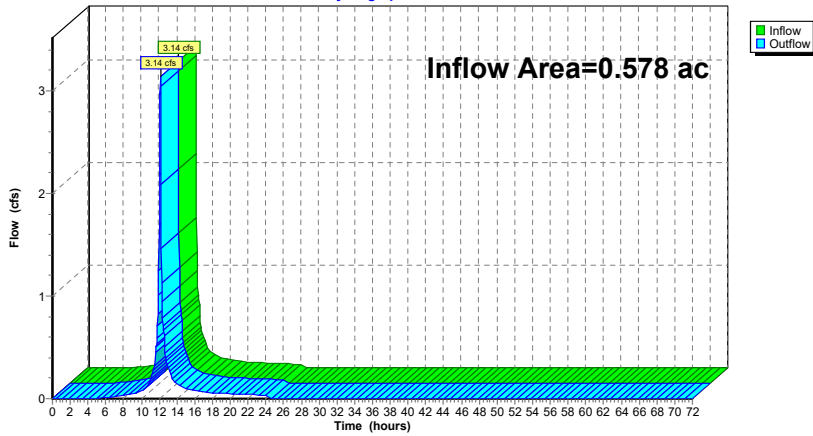
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.578 ac, 51.84% Impervious, Inflow Depth = 4.45" for NOAA 25-yr event  
Inflow = 3.14 cfs @ 12.13 hrs, Volume= 0.214 af  
Outflow = 3.14 cfs @ 12.13 hrs, Volume= 0.214 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB7: EX-CB7**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 112

**Summary for Reach CB8: EX-CB8**

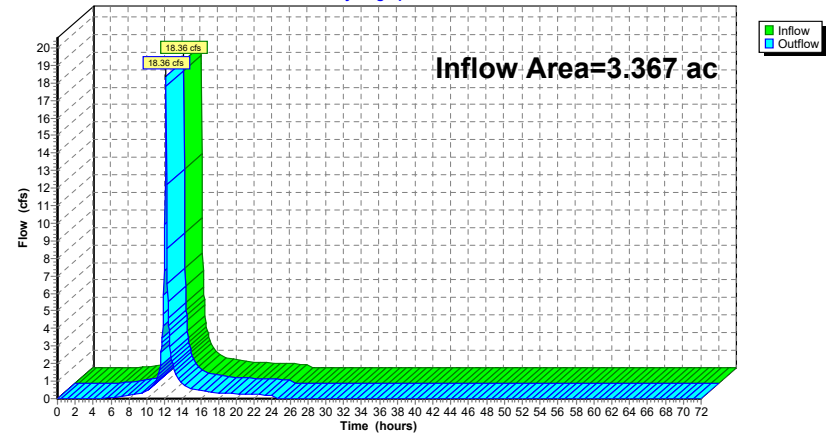
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.367 ac, 50.27% Impervious, Inflow Depth = 4.47" for NOAA 25-yr event  
Inflow = 18.36 cfs @ 12.13 hrs, Volume= 1.254 af  
Outflow = 18.36 cfs @ 12.13 hrs, Volume= 1.254 af, Atten= 0%, Lag= 0.0 min  
Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB8: EX-CB8**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 113

**Summary for Reach CB9: EX CB-9**

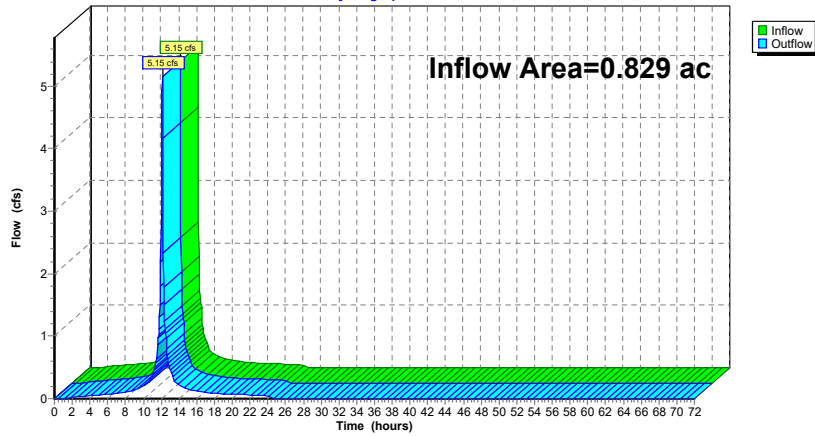
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.829 ac, 94.03% Impervious, Inflow Depth = 5.68" for NOAA 25-yr event  
 Inflow = 5.15 cfs @ 12.13 hrs, Volume= 0.393 af  
 Outflow = 5.15 cfs @ 12.13 hrs, Volume= 0.393 af, Atten= 0%, Lag= 0.0 min  
 Routed to Reach DP-2 : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach CB9: EX CB-9**

Hydrograph



**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 114

**Summary for Reach DP-1: French Rodney Blvd 14" Outfall**

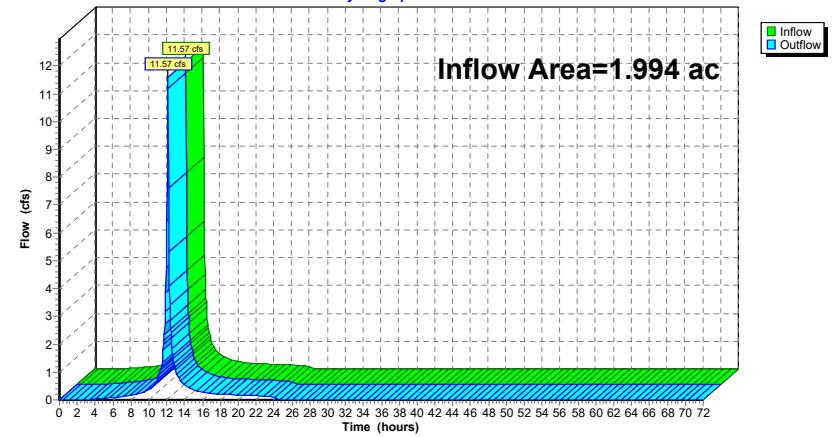
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.994 ac, 68.32% Impervious, Inflow Depth = 4.94" for NOAA 25-yr event  
 Inflow = 11.57 cfs @ 12.13 hrs, Volume= 0.822 af  
 Outflow = 11.57 cfs @ 12.13 hrs, Volume= 0.822 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-1: French Rodney Blvd 14" Outfall**

Hydrograph





**14850\_Existing-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 115

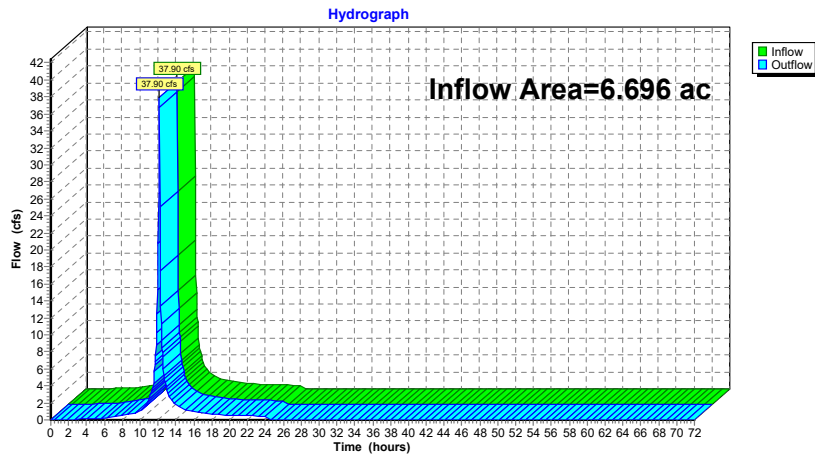
**Summary for Reach DP-2: PORTLAND STREET DRAINAGE**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.696 ac, 62.59% Impervious, Inflow Depth = 4.79" for NOAA 25-yr event  
Inflow = 37.90 cfs @ 12.13 hrs, Volume= 2.673 af  
Outflow = 37.90 cfs @ 12.13 hrs, Volume= 2.673 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-2: PORTLAND STREET DRAINAGE**

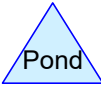
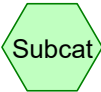
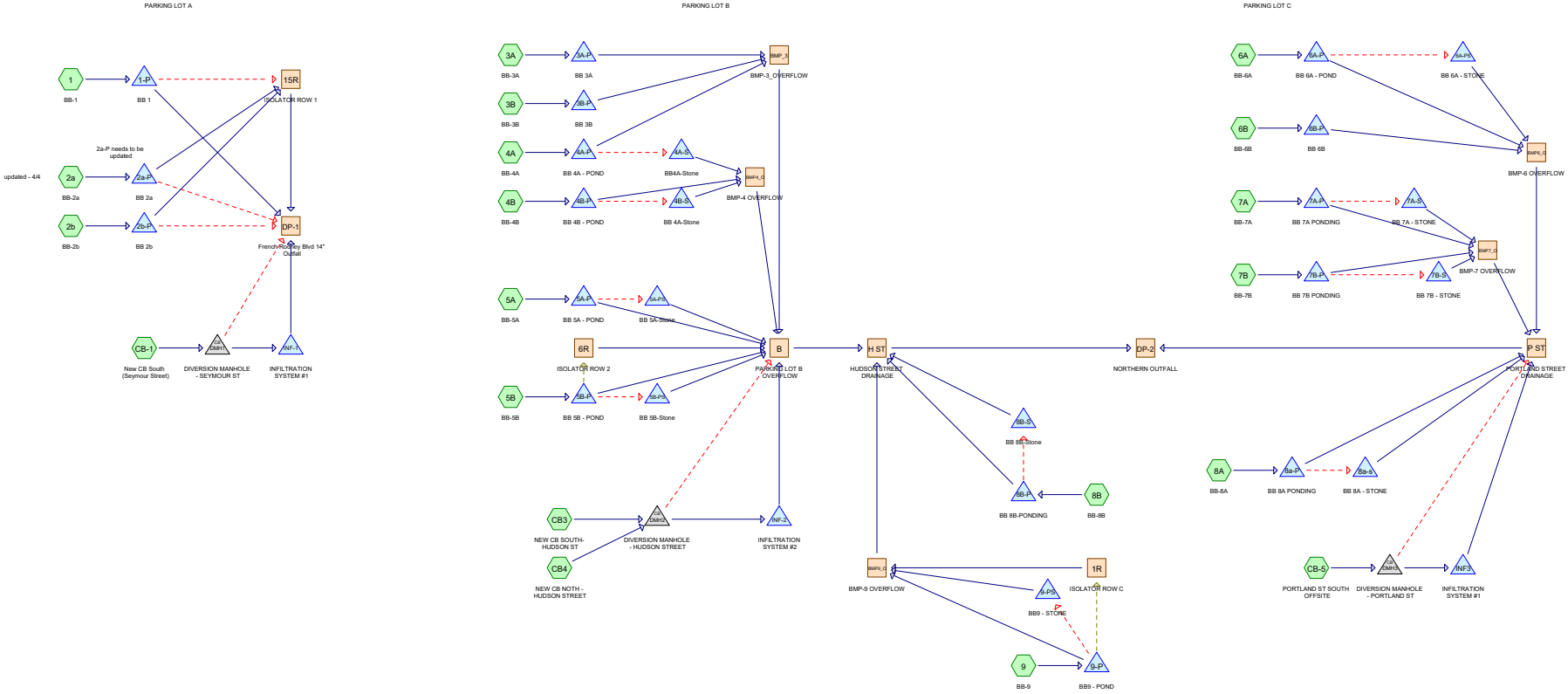


**APPENDIX D**

---

**Post-Development Conditions – HydroCAD Calculations**

# PROPOSED HYDROCAD ANALYSIS



**Routing Diagram for 14850\_Proposed-Drainage-Areas**  
 Prepared by {enter your company name here}, Printed 4/6/2022  
 HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

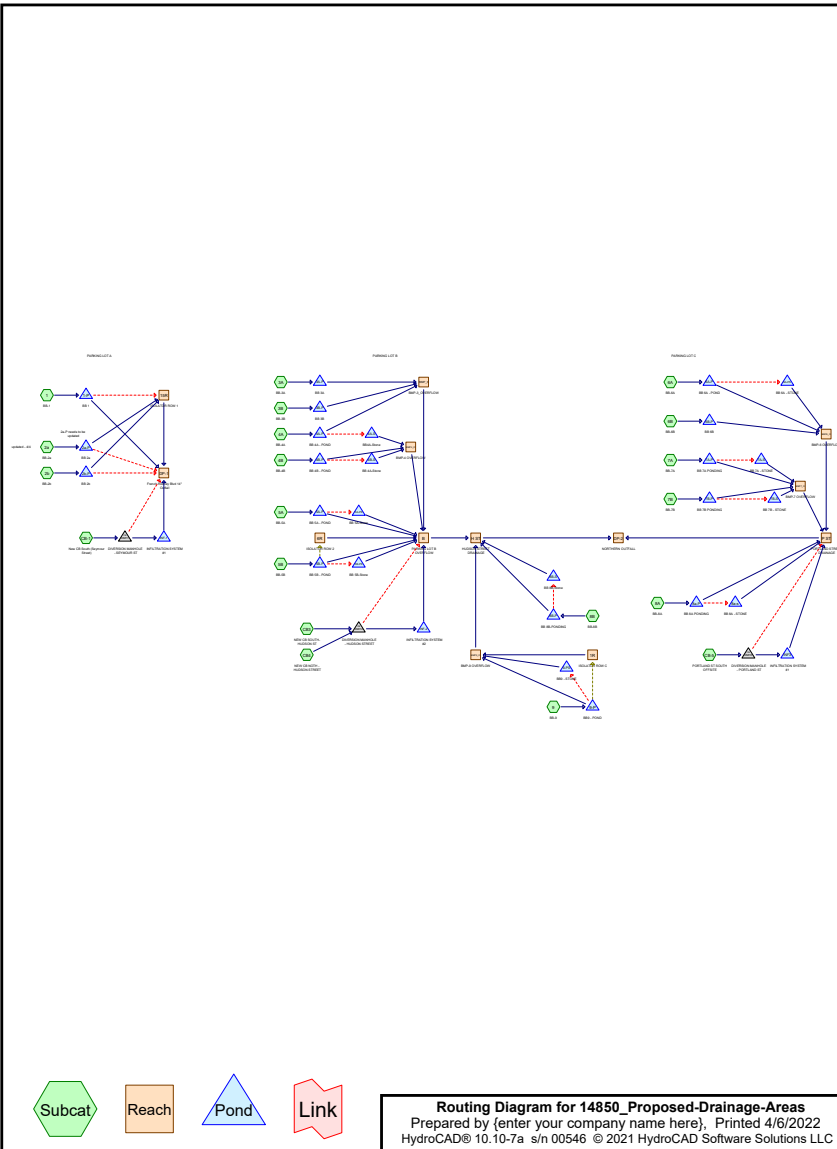
HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Printed 4/6/2022

Page 2

**Rainfall Events Listing (selected events)**

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	NOAA 10-yr	NOAA 24-hr	C	Default	24.00	1	5.02	2
2	NOAA 100-yr	NOAA 24-hr	C	Default	24.00	1	7.59	2
3	NOAA 2-yr	NOAA 24-hr	C	Default	24.00	1	3.40	2
4	NOAA 25-yr	NOAA 24-hr	C	Default	24.00	1	6.04	2



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Printed 4/6/2022

Page 3

**Area Listing (all nodes)**

Area (sq-ft)	CN	Description (subcatchment-numbers)
214,977	83	1/4 acre lots, 38% imp, HSG C (1, 2b, 3A, 5B, 6A, 6B, 9, CB-1, CB-5, CB3, CB4)
16,767	74	>75% Grass cover, Good, HSG C (1, 2a, 2b, 3A, 3B, 4A, 4B, 5A, 5B, 6A, 6B, 7A, 7B, 8A, 8B, 9)
107,061	98	Paved parking, HSG C (1, 2a, 2b, 3A, 3B, 4A, 4B, 5A, 5B, 6A, 6B, 7A, 7B, 8A, 8B, 9)
37,488	98	Roadway (CB-1, CB-5, CB3, CB4)
<b>376,293</b>	<b>88</b>	<b>TOTAL AREA</b>

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Printed 4/6/2022

Page 4

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: BB-1</b>	Runoff Area=38,826 sf 51.66% Impervious Runoff Depth=3.49" Tc=6.0 min CN=86 Runoff=3.85 cfs 11,278 cf
<b>Subcatchment2a: BB-2a</b>	Runoff Area=3,116 sf 92.62% Impervious Runoff Depth=4.55" Tc=6.0 min CN=96 Runoff=0.37 cfs 1,182 cf
<b>Subcatchment2b: BB-2b</b>	Runoff Area=21,490 sf 80.50% Impervious Runoff Depth=4.22" Tc=6.0 min CN=93 Runoff=2.43 cfs 7,533 cf
<b>Subcatchment3A: BB-3A</b>	Runoff Area=10,987 sf 58.16% Impervious Runoff Depth=3.69" Tc=6.0 min CN=88 Runoff=1.14 cfs 3,377 cf
<b>Subcatchment3B: BB-3B</b>	Runoff Area=4,545 sf 77.34% Impervious Runoff Depth=4.22" Tc=6.0 min CN=93 Runoff=0.51 cfs 1,597 cf
<b>Subcatchment4A: BB-4A</b>	Runoff Area=4,843 sf 86.37% Impervious Runoff Depth=4.44" Tc=6.0 min CN=95 Runoff=0.56 cfs 1,792 cf
<b>Subcatchment4B: BB-4B</b>	Runoff Area=3,048 sf 86.09% Impervious Runoff Depth=4.44" Tc=6.0 min CN=95 Runoff=0.35 cfs 1,128 cf
<b>Subcatchment5A: BB-5A</b>	Runoff Area=3,072 sf 73.44% Impervious Runoff Depth=4.11" Tc=6.0 min CN=92 Runoff=0.34 cfs 1,052 cf
<b>Subcatchment5B: BB-5B</b>	Runoff Area=34,755 sf 71.39% Impervious Runoff Depth=4.00" Tc=6.0 min CN=91 Runoff=3.81 cfs 11,589 cf
<b>Subcatchment6A: BB-6A</b>	Runoff Area=15,148 sf 46.97% Impervious Runoff Depth=3.39" Tc=6.0 min CN=85 Runoff=1.47 cfs 4,275 cf
<b>Subcatchment6B: BB-6B</b>	Runoff Area=6,495 sf 77.45% Impervious Runoff Depth=4.22" Tc=6.0 min CN=93 Runoff=0.73 cfs 2,283 cf
<b>Subcatchment7A: BB-7A</b>	Runoff Area=3,165 sf 87.74% Impervious Runoff Depth=4.44" Tc=6.0 min CN=95 Runoff=0.37 cfs 1,171 cf
<b>Subcatchment7B: BB-7B</b>	Runoff Area=4,942 sf 88.73% Impervious Runoff Depth=4.44" Tc=6.0 min CN=95 Runoff=0.57 cfs 1,828 cf
<b>Subcatchment8A: BB-8A</b>	Runoff Area=3,978 sf 79.99% Impervious Runoff Depth=4.22" Tc=6.0 min CN=93 Runoff=0.45 cfs 1,398 cf
<b>Subcatchment8B: BB-8B</b>	Runoff Area=5,598 sf 87.78% Impervious Runoff Depth=4.44" Tc=6.0 min CN=95 Runoff=0.65 cfs 2,071 cf
<b>Subcatchment9: BB-9</b>	Runoff Area=29,651 sf 74.77% Impervious Runoff Depth=4.11" Tc=6.0 min CN=92 Runoff=3.30 cfs 10,153 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 5

<b>SubcatchmentCB-1: New CB South</b>	Runoff Area=19,582 sf 58.17% Impervious Runoff Depth=3.69" Flow Length=512' Tc=6.0 min CN=88 Runoff=2.03 cfs 6,018 cf
<b>SubcatchmentCB-5: PORTLANDST</b>	Runoff Area=19,743 sf 50.83% Impervious Runoff Depth=3.49" Flow Length=574' Tc=6.0 min CN=86 Runoff=1.96 cfs 5,735 cf
<b>SubcatchmentCB3: NEW CB SOUTH-</b>	Runoff Area=25,183 sf 51.84% Impervious Runoff Depth=3.49" Flow Length=635' Tc=6.0 min CN=86 Runoff=2.49 cfs 7,315 cf
<b>SubcatchmentCB4: NEW CB NOTH -</b>	Runoff Area=118,126 sf 49.24% Impervious Runoff Depth=3.49" Flow Length=822' Tc=6.0 min CN=86 Runoff=11.70 cfs 34,312 cf
<b>Reach 1R: ISOLATORROW C</b>	Inflow=2.51 cfs 7,693 cf Outflow=2.51 cfs 7,693 cf
<b>Reach 6R: ISOLATORROW 2</b>	Inflow=2.18 cfs 7,000 cf Outflow=2.18 cfs 7,000 cf
<b>Reach 15R: ISOLATORROW 1</b>	Inflow=4.70 cfs 16,169 cf Outflow=4.70 cfs 16,169 cf
<b>Reach B: PARKINGLOT B OVERFLOW</b>	Inflow=20.02 cfs 50,822 cf Outflow=20.02 cfs 50,822 cf
<b>Reach BMP4_O: BMP-4 OVERFLOW</b>	Inflow=0.38 cfs 2,236 cf Outflow=0.38 cfs 2,236 cf
<b>Reach BMP6_O: BMP-6 OVERFLOW</b>	Inflow=2.12 cfs 5,487 cf Outflow=2.12 cfs 5,487 cf
<b>Reach BMP7_O: BMP-7 OVERFLOW</b>	Inflow=0.92 cfs 2,999 cf Outflow=0.92 cfs 2,999 cf
<b>Reach BMP9_O: BMP-9 OVERFLOW</b>	Inflow=3.22 cfs 10,153 cf Outflow=3.22 cfs 10,153 cf
<b>Reach BMP_3: BMP-3_OVERFLOW</b>	Inflow=2.06 cfs 3,545 cf Outflow=2.06 cfs 3,545 cf
<b>Reach DP-1: French Rodney Blvd 14" Outfall</b>	Inflow=7.60 cfs 19,210 cf Outflow=7.60 cfs 19,210 cf
<b>Reach DP-2: NORTHERNOUTFALL</b>	Inflow=29.18 cfs 75,762 cf Outflow=29.18 cfs 75,762 cf
<b>Reach H ST: HUDSON STREET DRAINAGE</b>	Inflow=23.87 cfs 63,045 cf Outflow=23.87 cfs 63,045 cf
<b>Reach P ST: PORTLANDSTREET DRAINAGE</b>	Inflow=5.33 cfs 12,717 cf Outflow=5.33 cfs 12,717 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 6

<b>Pond 1-P: BB 1</b>	Peak Elev=10.14' Storage=1,014 cf Inflow=3.85 cfs 11,278 cf Discarded=0.05 cfs 2,554 cf Primary=1.61 cfs 1,219 cf Secondary=2.06 cfs 7,505 cf Outflow=3.72 cfs 11,278 cf
<b>Pond 2a-P: BB 2a</b>	Peak Elev=8.19' Storage=76 cf Inflow=0.37 cfs 1,182 cf Primary=0.35 cfs 1,146 cf Secondary=0.00 cfs 0 cf Outflow=0.35 cfs 1,146 cf
<b>Pond 2b-P: BB 2b</b>	Peak Elev=8.46' Storage=228 cf Inflow=2.43 cfs 7,553 cf Primary=2.28 cfs 7,517 cf Secondary=0.00 cfs 0 cf Outflow=2.28 cfs 7,517 cf
<b>Pond 3A-P: BB 3A</b>	Peak Elev=11.07' Storage=449 cf Inflow=1.14 cfs 3,377 cf Discarded=0.02 cfs 1,314 cf Primary=1.07 cfs 2,063 cf Outflow=1.09 cfs 3,377 cf
<b>Pond 3B-P: BB 3B</b>	Peak Elev=12.89' Storage=254 cf Inflow=0.51 cfs 1,597 cf Discarded=0.01 cfs 799 cf Primary=0.49 cfs 798 cf Outflow=0.51 cfs 1,597 cf
<b>Pond 4A-P: BB 4A - POND</b>	Peak Elev=10.06' Storage=230 cf Inflow=0.56 cfs 1,792 cf Primary=0.51 cfs 683 cf Secondary=0.04 cfs 1,108 cf Outflow=0.54 cfs 1,792 cf
<b>Pond 4A-S: BB4A-Stone</b>	Peak Elev=6.12' Storage=9 cf Inflow=0.04 cfs 1,108 cf Outflow=0.04 cfs 1,108 cf
<b>Pond 4B-P: BB 4B - POND</b>	Peak Elev=11.03' Storage=131 cf Inflow=0.35 cfs 1,128 cf Primary=0.32 cfs 437 cf Secondary=0.02 cfs 690 cf Outflow=0.35 cfs 1,128 cf
<b>Pond 4B-S: BB 4A-Stone</b>	Peak Elev=6.10' Storage=4 cf Inflow=0.02 cfs 690 cf Outflow=0.02 cfs 690 cf
<b>Pond 5A-P: BB 5A - POND</b>	Peak Elev=9.43' Storage=369 cf Inflow=0.34 cfs 1,052 cf Primary=0.00 cfs 0 cf Secondary=0.04 cfs 1,052 cf Outflow=0.04 cfs 1,052 cf
<b>Pond 5A-PS: BB 5A-Stone</b>	Peak Elev=6.14' Storage=20 cf Inflow=0.04 cfs 1,052 cf Outflow=0.04 cfs 1,052 cf
<b>Pond 5B-P: BB 5B - POND</b>	Peak Elev=9.04' Storage=665 cf Inflow=3.81 cfs 11,589 cf Primary=1.48 cfs 934 cf Secondary=0.08 cfs 3,656 cf Tertiary=2.18 cfs 7,000 cf Outflow=3.74 cfs 11,589 cf
<b>Pond 5B-PS: BB 5B-Stone</b>	Peak Elev=6.00' Storage=1 cf Inflow=0.08 cfs 3,656 cf Outflow=0.08 cfs 3,656 cf
<b>Pond 6A-P: BB 6A - POND</b>	Peak Elev=11.00' Storage=420 cf Inflow=1.47 cfs 4,275 cf Primary=1.38 cfs 2,347 cf Secondary=0.05 cfs 1,928 cf Outflow=1.43 cfs 4,275 cf
<b>Pond 6A-PS: BB 6A - STONE</b>	Peak Elev=6.14' Storage=12 cf Inflow=0.05 cfs 1,928 cf Outflow=0.05 cfs 1,928 cf
<b>Pond 6B-P: BB 6B</b>	Peak Elev=12.08' Storage=380 cf Inflow=0.73 cfs 2,283 cf Discarded=0.02 cfs 1,071 cf Primary=0.69 cfs 1,212 cf Outflow=0.71 cfs 2,283 cf
<b>Pond 7A-P: BB 7A PONDING</b>	Peak Elev=9.98' Storage=148 cf Inflow=0.37 cfs 1,171 cf Primary=0.34 cfs 465 cf Secondary=0.02 cfs 706 cf Outflow=0.36 cfs 1,171 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 7

**Pond 7A-S: BB 7A - STONE** Peak Elev=5.19' Storage=4 cf Inflow=0.02 cfs 706 cf  
Outflow=0.02 cfs 706 cf

**Pond 7B-P: BB 7B PONDING** Peak Elev=10.71' Storage=274 cf Inflow=0.57 cfs 1,828 cf  
Primary=0.52 cfs 676 cf Secondary=0.03 cfs 1,153 cf Outflow=0.56 cfs 1,828 cf

**Pond 7B-S: BB 7B - STONE** Peak Elev=5.22' Storage=5 cf Inflow=0.03 cfs 1,153 cf  
Outflow=0.03 cfs 1,153 cf

**Pond 8a-P: BB 8A PONDING** Peak Elev=9.09' Storage=285 cf Inflow=0.45 cfs 1,398 cf  
Primary=0.38 cfs 357 cf Secondary=0.04 cfs 1,041 cf Outflow=0.42 cfs 1,398 cf

**Pond 8a-s: BB 8A - STONE** Peak Elev=4.53' Storage=12 cf Inflow=0.04 cfs 1,041 cf  
Outflow=0.04 cfs 1,041 cf

**Pond 8B-P: BB 8B-PONDING** Peak Elev=9.72' Storage=255 cf Inflow=0.65 cfs 2,071 cf  
Primary=0.60 cfs 805 cf Secondary=0.04 cfs 1,266 cf Outflow=0.64 cfs 2,071 cf

**Pond 8B-S: BB 8B-Stone** Peak Elev=4.53' Storage=12 cf Inflow=0.04 cfs 1,266 cf  
Outflow=0.04 cfs 1,266 cf

**Pond 9-P: BB9 - POND** Peak Elev=8.93' Storage=430 cf Inflow=3.30 cfs 10,153 cf  
Primary=0.67 cfs 245 cf Secondary=0.05 cfs 2,215 cf Tertiary=2.51 cfs 7,693 cf Outflow=3.22 cfs 10,153 cf

**Pond 9-PS: BB9 - STONE** Peak Elev=6.15' Storage=8 cf Inflow=0.05 cfs 2,215 cf  
Outflow=0.05 cfs 2,215 cf

**Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST** Peak Elev=9.81' Inflow=2.03 cfs 6,018 cf  
Primary=0.71 cfs 4,632 cf Secondary=1.32 cfs 1,386 cf Outflow=2.03 cfs 6,018 cf

**Pond DMH2: DIVERSION MANHOLE - HUDSON STREET** Peak Elev=20.37' Inflow=14.20 cfs 41,627 cf  
Primary=2.93 cfs 23,559 cf Secondary=11.27 cfs 18,068 cf Outflow=14.20 cfs 41,627 cf

**Pond DMH3: DIVERSION MANHOLE - PORTLAND ST** Peak Elev=11.73' Inflow=1.96 cfs 5,735 cf  
Primary=1.06 cfs 4,862 cf Secondary=0.90 cfs 872 cf Outflow=1.96 cfs 5,735 cf

**Pond INF-1: INFILTRATIONS SYSTEM #1** Peak Elev=9.44' Storage=1,658 cf Inflow=0.71 cfs 4,632 cf  
Discarded=0.08 cfs 4,195 cf Primary=0.15 cfs 437 cf Outflow=0.23 cfs 4,632 cf

**Pond INF-2: INFILTRATIONS SYSTEM #2** Peak Elev=9.80' Storage=3,428 cf Inflow=2.93 cfs 23,559 cf  
Discarded=0.12 cfs 9,228 cf Primary=2.66 cfs 14,332 cf Outflow=2.77 cfs 23,559 cf

**Pond INF3: INFILTRATIONS SYSTEM #1** Peak Elev=9.66' Storage=991 cf Inflow=1.06 cfs 4,862 cf  
Discarded=0.05 cfs 2,902 cf Primary=1.00 cfs 1,960 cf Outflow=1.05 cfs 4,862 cf

**Total Runoff Area = 376,293 sf Runoff Volume = 117,106 cf Average Runoff Depth = 3.73"**  
**39.88% Pervious = 150,053 sf 60.12% Impervious = 226,240 sf**

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 8

**Summary for Subcatchment 1: BB-1**

Runoff = 3.85 cfs @ 12.13 hrs, Volume= 11,278 cf, Depth= 3.49"  
Routed to Pond 1-P : BB 1

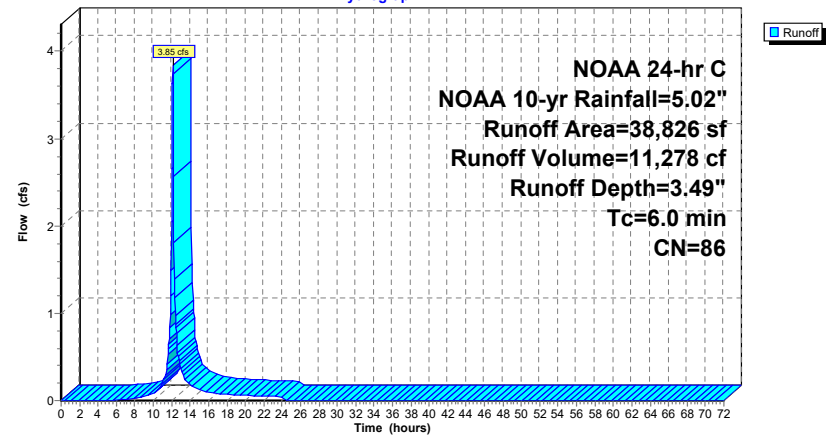
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
27,309	83	1/4 acre lots, 38% imp, HSG C
1,838	74	>75% Grass cover, Good, HSG C
9,679	98	Paved parking, HSG C
38,826	86	Weighted Average
18,770		48.34% Pervious Area
20,056		51.66% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 1: BB-1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 9

**Summary for Subcatchment 2a: BB-2a**

Runoff = 0.37 cfs @ 12.13 hrs, Volume= 1,182 cf, Depth= 4.55"  
Routed to Pond 2a-P : BB 2a

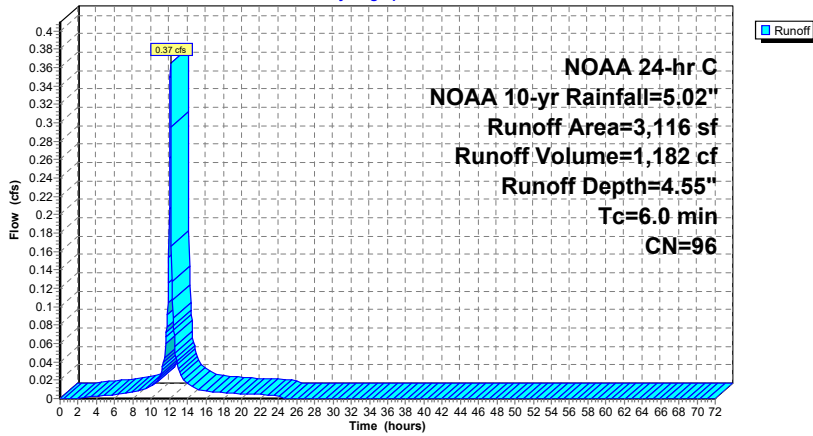
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
230	74	>75% Grass cover, Good, HSG C
2,886	98	Paved parking, HSG C
3,116	96	Weighted Average
230		7.38% Pervious Area
2,886		92.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2a: BB-2a**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 10

**Summary for Subcatchment 2b: BB-2b**

Runoff = 2.43 cfs @ 12.13 hrs, Volume= 7,553 cf, Depth= 4.22"  
Routed to Pond 2b-P : BB 2b

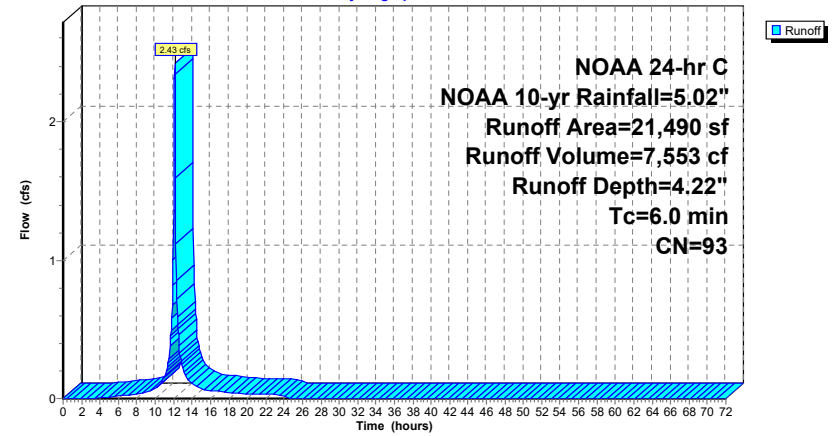
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
3,097	83	1/4 acre lots, 38% imp, HSG C
2,270	74	>75% Grass cover, Good, HSG C
16,123	98	Paved parking, HSG C
21,490	93	Weighted Average
4,190		19.50% Pervious Area
17,300		80.50% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2b: BB-2b**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 11

**Summary for Subcatchment 3A: BB-3A**

Runoff = 1.14 cfs @ 12.13 hrs, Volume= 3,377 cf, Depth= 3.69"  
Routed to Pond 3A-P : BB 3A

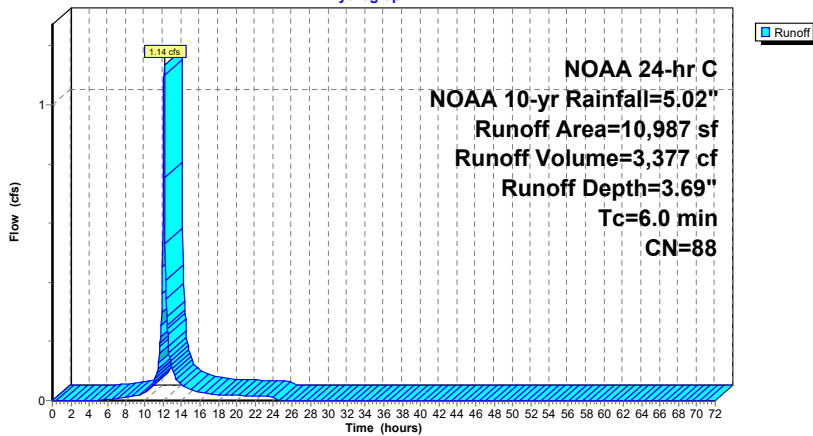
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
5,791	83	1/4 acre lots, 38% imp, HSG C
1,007	74	>75% Grass cover, Good, HSG C
4,189	98	Paved parking, HSG C
10,987	88	Weighted Average
4,597		41.84% Pervious Area
6,390		58.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3A: BB-3A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 12

**Summary for Subcatchment 3B: BB-3B**

Runoff = 0.51 cfs @ 12.13 hrs, Volume= 1,597 cf, Depth= 4.22"  
Routed to Pond 3B-P : BB 3B

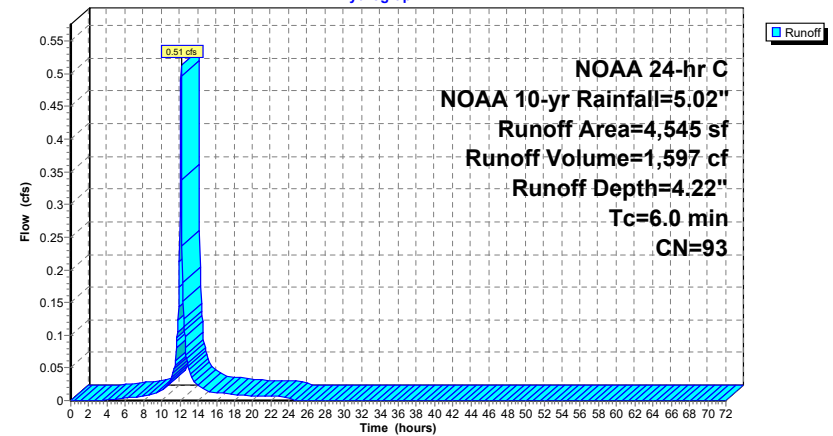
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
* 1,030	74	>75% Grass cover, Good, HSG C
3,515	98	Paved parking, HSG C
4,545	93	Weighted Average
1,030		22.66% Pervious Area
3,515		77.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3B: BB-3B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 13

**Summary for Subcatchment 4A: BB-4A**

Runoff = 0.56 cfs @ 12.13 hrs, Volume= 1,792 cf, Depth= 4.44"  
Routed to Pond 4A-P : BB 4A - POND

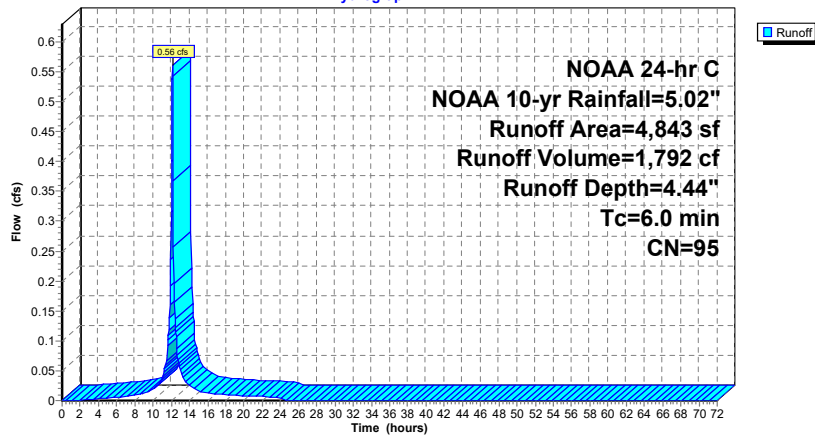
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
660	74	>75% Grass cover, Good, HSG C
4,183	98	Paved parking, HSG C
4,843	95	Weighted Average
660		13.63% Pervious Area
4,183		86.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 4A: BB-4A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 14

**Summary for Subcatchment 4B: BB-4B**

Runoff = 0.35 cfs @ 12.13 hrs, Volume= 1,128 cf, Depth= 4.44"  
Routed to Pond 4B-P : BB 4B - POND

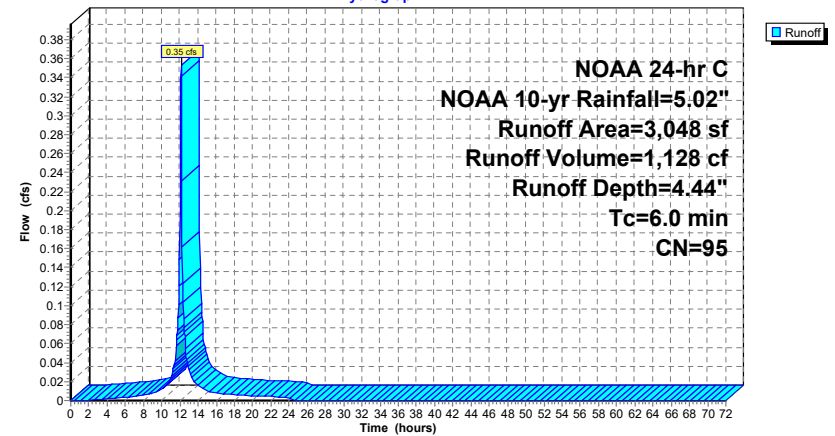
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
424	74	>75% Grass cover, Good, HSG C
2,624	98	Paved parking, HSG C
3,048	95	Weighted Average
424		13.91% Pervious Area
2,624		86.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 4B: BB-4B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 15

**Summary for Subcatchment 5A: BB-5A**

Runoff = 0.34 cfs @ 12.13 hrs, Volume= 1,052 cf, Depth= 4.11"  
Routed to Pond 5A-P : BB 5A - POND

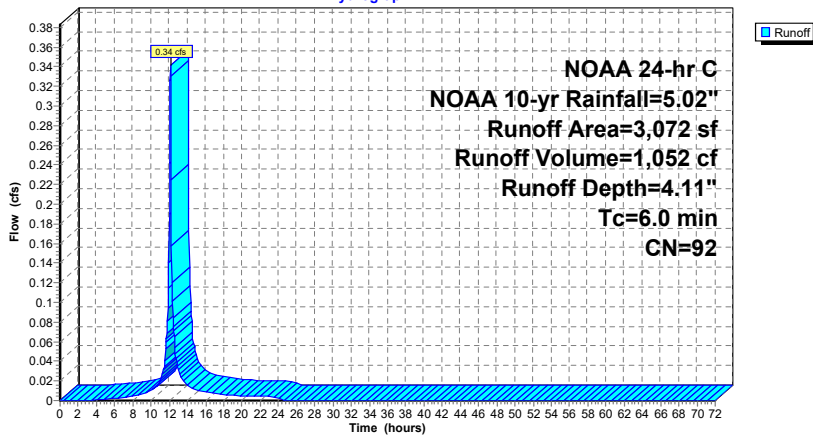
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
816	74	>75% Grass cover, Good, HSG C
2,256	98	Paved parking, HSG C
3,072	92	Weighted Average
816		26.56% Pervious Area
2,256		73.44% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5A: BB-5A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 16

**Summary for Subcatchment 5B: BB-5B**

Runoff = 3.81 cfs @ 12.13 hrs, Volume= 11,589 cf, Depth= 4.00"  
Routed to Pond 5B-P : BB 5B - POND

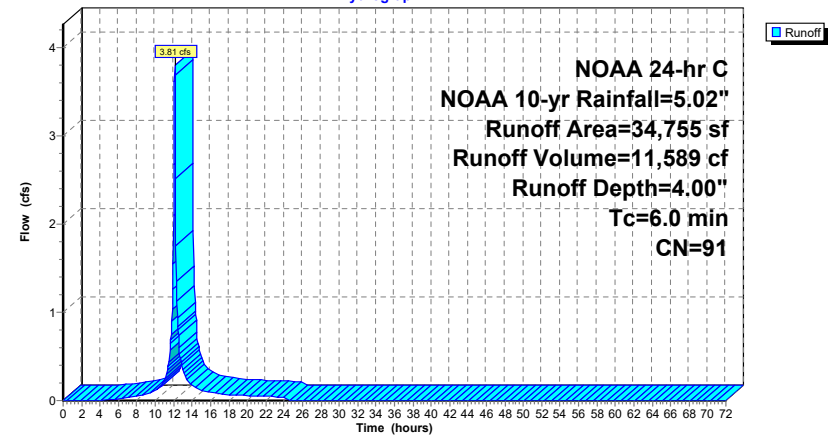
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
12,062	83	1/4 acre lots, 38% imp, HSG C
2,464	74	>75% Grass cover, Good, HSG C
20,229	98	Paved parking, HSG C
34,755	91	Weighted Average
9,942		28.61% Pervious Area
24,813		71.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5B: BB-5B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 17

**Summary for Subcatchment 6A: BB-6A**

Runoff = 1.47 cfs @ 12.13 hrs, Volume= 4,275 cf, Depth= 3.39"  
Routed to Pond 6A-P : BB 6A - POND

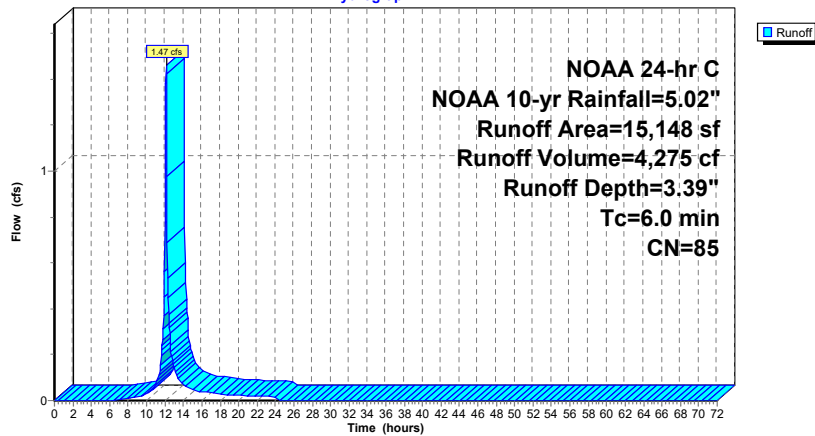
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
11,763	83	1/4 acre lots, 38% imp, HSG C
740	74	>75% Grass cover, Good, HSG C
2,645	98	Paved parking, HSG C
15,148	85	Weighted Average
8,033		53.03% Pervious Area
7,115		46.97% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6A: BB-6A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 18

**Summary for Subcatchment 6B: BB-6B**

Runoff = 0.73 cfs @ 12.13 hrs, Volume= 2,283 cf, Depth= 4.22"  
Routed to Pond 6B-P : BB 6B

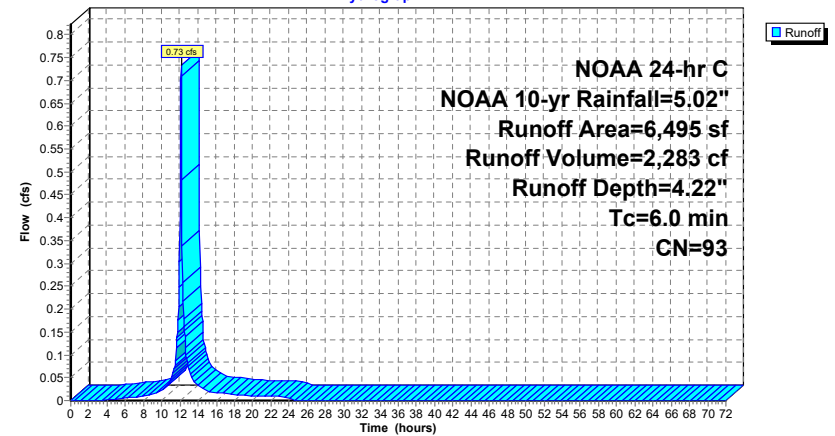
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
1,259	83	1/4 acre lots, 38% imp, HSG C
684	74	>75% Grass cover, Good, HSG C
4,552	98	Paved parking, HSG C
6,495	93	Weighted Average
1,465		22.55% Pervious Area
5,030		77.45% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6B: BB-6B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 19

**Summary for Subcatchment 7A: BB-7A**

Runoff = 0.37 cfs @ 12.13 hrs, Volume= 1,171 cf, Depth= 4.44"  
 Routed to Pond 7A-P : BB 7A PONDING

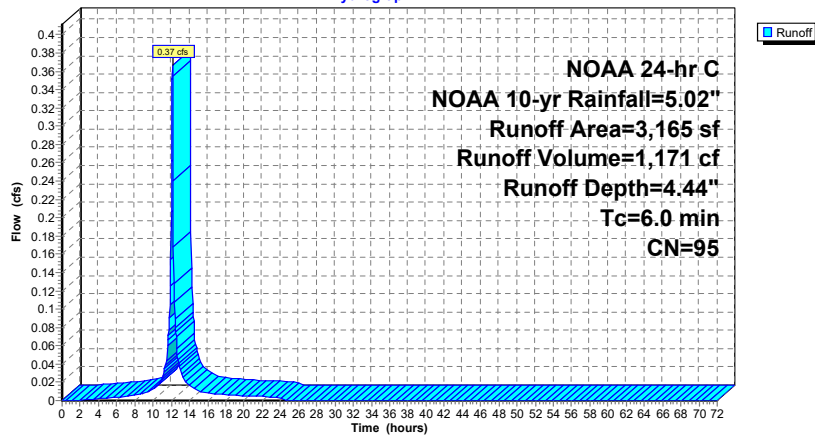
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
388	74	>75% Grass cover, Good, HSG C
2,777	98	Paved parking, HSG C
3,165	95	Weighted Average
388		12.26% Pervious Area
2,777		87.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 7A: BB-7A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 20

**Summary for Subcatchment 7B: BB-7B**

Runoff = 0.57 cfs @ 12.13 hrs, Volume= 1,828 cf, Depth= 4.44"  
 Routed to Pond 7B-P : BB 7B PONDING

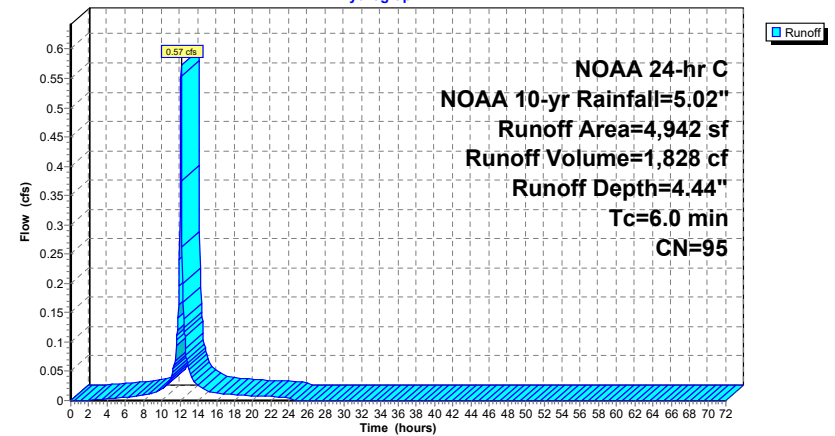
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
557	74	>75% Grass cover, Good, HSG C
4,385	98	Paved parking, HSG C
4,942	95	Weighted Average
557		11.27% Pervious Area
4,385		88.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 7B: BB-7B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 21

**Summary for Subcatchment 8A: BB-8A**

Runoff = 0.45 cfs @ 12.13 hrs, Volume= 1,398 cf, Depth= 4.22"  
Routed to Pond 8a-P : BB 8A PONDING

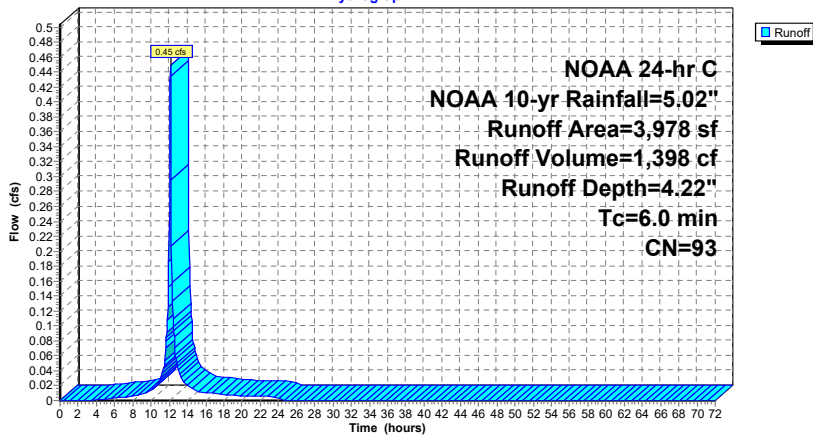
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
796	74	>75% Grass cover, Good, HSG C
3,182	98	Paved parking, HSG C
3,978	93	Weighted Average
796		20.01% Pervious Area
3,182		79.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8A: BB-8A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 22

**Summary for Subcatchment 8B: BB-8B**

Runoff = 0.65 cfs @ 12.13 hrs, Volume= 2,071 cf, Depth= 4.44"  
Routed to Pond 8B-P : BB 8B-PONDING

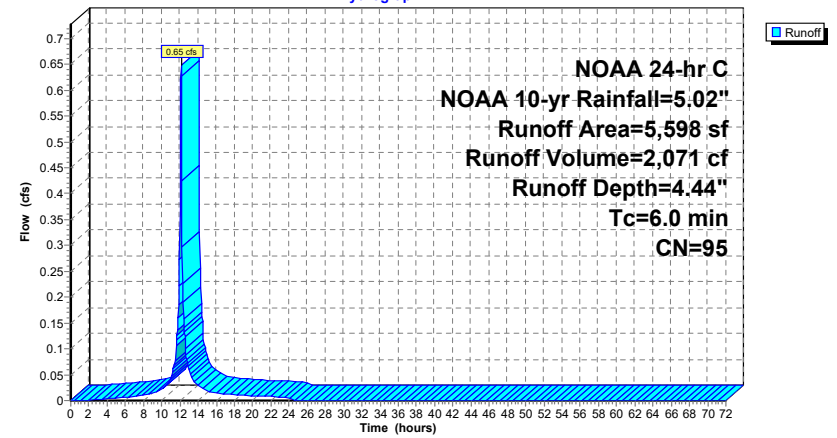
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
684	74	>75% Grass cover, Good, HSG C
4,914	98	Paved parking, HSG C
5,598	95	Weighted Average
684		12.22% Pervious Area
4,914		87.78% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8B: BB-8B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 23

**Summary for Subcatchment 9: BB-9**

Runoff = 3.30 cfs @ 12.13 hrs, Volume= 10,153 cf, Depth= 4.11"  
Routed to Pond 9-P : BB9 - POND

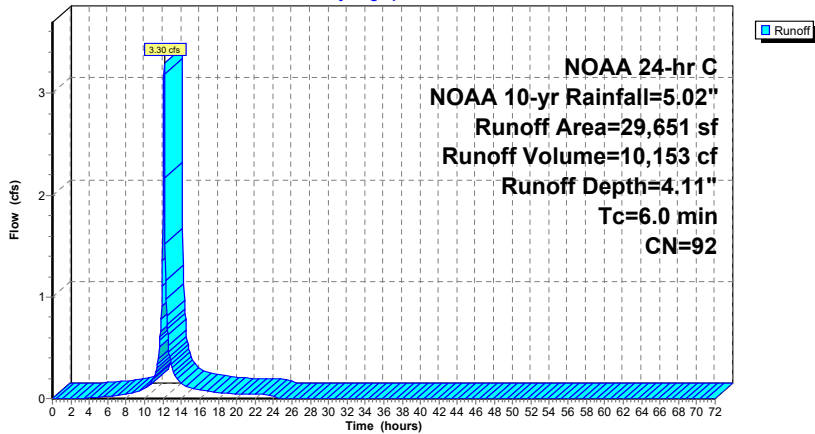
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
8,550	83	1/4 acre lots, 38% imp, HSG C
2,179	74	>75% Grass cover, Good, HSG C
18,922	98	Paved parking, HSG C
29,651	92	Weighted Average
7,480		25.23% Pervious Area
22,171		74.77% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 9: BB-9**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 24

**Summary for Subcatchment CB-1: New CB South (Seymour Street)**

Runoff = 2.03 cfs @ 12.13 hrs, Volume= 6,018 cf, Depth= 3.69"  
Routed to Pond DMH1 : DIVERSION MANHOLE - SEYMOUR ST

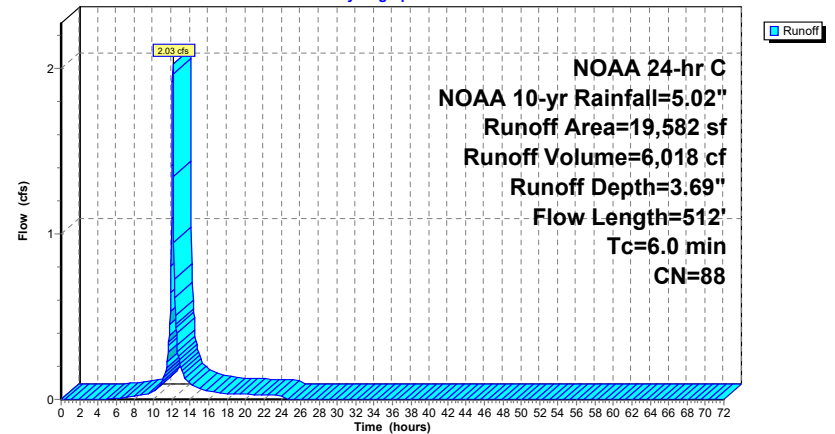
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
13,211	83	1/4 acre lots, 38% imp, HSG C
6,371	98	Roadway
19,582	88	Weighted Average
8,191		41.83% Pervious Area
11,391		58.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	50	0.0300	1.45		Sheet Flow, A-B Smooth surfaces n= 0.011 P2= 3.40"
2.4	462	0.0249	3.20		Shallow Concentrated Flow, Paved Paved Kv= 20.3 fps
3.0					Direct Entry, Direct entry to 6
6.0	512				Total

**Subcatchment CB-1: New CB South (Seymour Street)**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 25

**Summary for Subcatchment CB-5: PORTLAND ST SOUTH OFFSITE**

Runoff = 1.96 cfs @ 12.13 hrs, Volume= 5,735 cf, Depth= 3.49"  
 Routed to Pond DMH3 : DIVERSION MANHOLE - PORTLAND ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

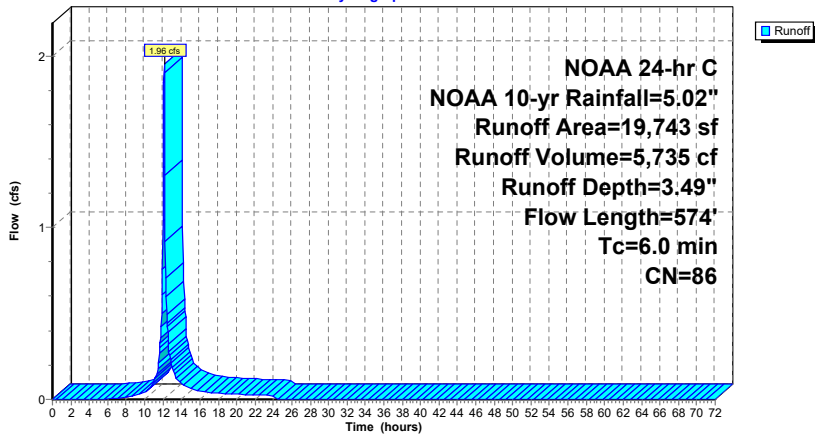
Area (sf)	CN	Description
15,657	83	1/4 acre lots, 38% imp, HSG C
* 4,086	98	Roadway
19,743	86	Weighted Average
9,707		49.17% Pervious Area
10,036		50.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
2.3	524	0.0346	3.78		Shallow Concentrated Flow, B-C (shallow conc.) Paved Kv= 20.3 fps
3.2					Direct Entry, direct to 6
6.0	574	Total			

**Subcatchment CB-5: PORTLAND ST SOUTH OFFSITE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 26

**Summary for Subcatchment CB3: NEW CB SOUTH- HUDSON ST**

Runoff = 2.49 cfs @ 12.13 hrs, Volume= 7,315 cf, Depth= 3.49"  
 Routed to Pond DMH2 : DIVERSION MANHOLE - HUDSON STREET

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

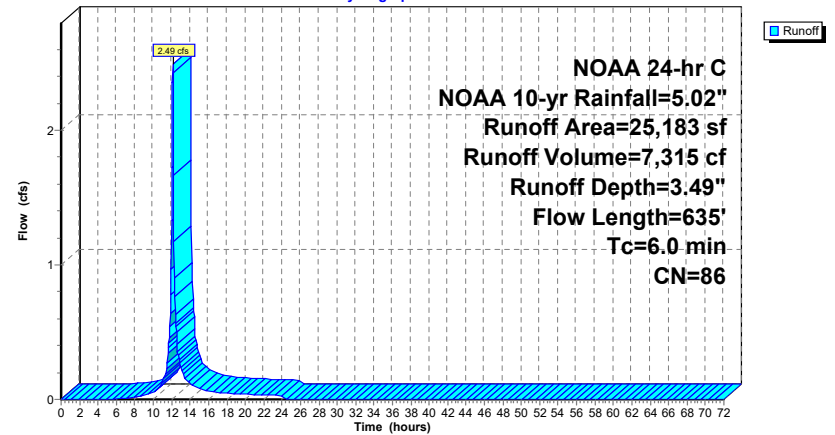
Area (sf)	CN	Description
19,562	83	1/4 acre lots, 38% imp, HSG C
* 5,621	98	Roadway
25,183	86	Weighted Average
12,128		48.16% Pervious Area
13,055		51.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0444	1.70		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
3.0	585	0.0256	3.25		Shallow Concentrated Flow, B-C Paved Kv= 20.3 fps
2.5					Direct Entry, direct entry to 6
6.0	635	Total			

**Subcatchment CB3: NEW CB SOUTH- HUDSON ST**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 27

**Summary for Subcatchment CB4: NEW CB NOTH - HUDSON STREET**

Runoff = 11.70 cfs @ 12.13 hrs, Volume= 34,312 cf, Depth= 3.49"  
 Routed to Pond DMH2 : DIVERSION MANHOLE - HUDSON STREET

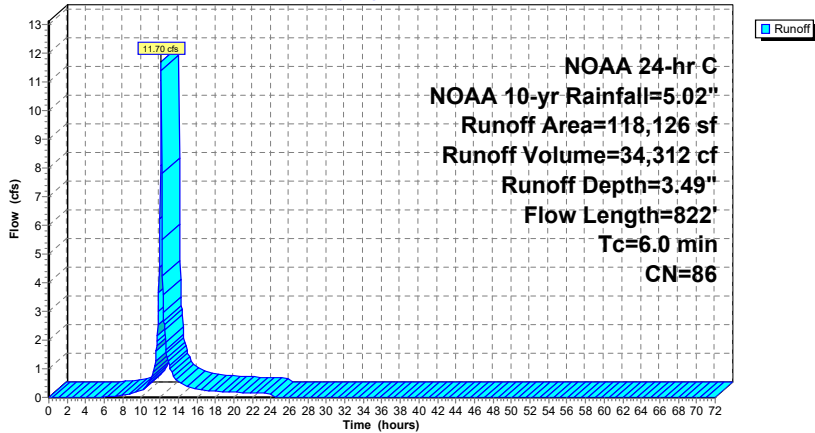
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Area (sf)	CN	Description
96,716	83	1/4 acre lots, 38% imp, HSG C
* 21,410	98	Roadway
118,126	86	Weighted Average
59,964		50.76% Pervious Area
58,162		49.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
4.0	772	0.0245	3.18		Shallow Concentrated Flow, B-C (shallow concentrated) Paved Kv= 20.3 fps
1.5					Direct Entry, direct entry to 6
6.0	822	Total			

**Subcatchment CB4: NEW CB NOTH - HUDSON STREET**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 28

**Summary for Reach 1R: ISOLATOR ROW C**

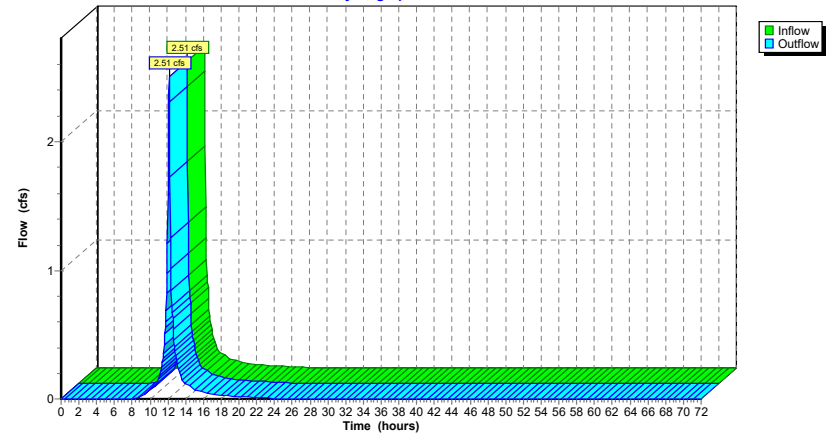
[40] Hint: Not Described (Outflow=Inflow)

Inflow = 2.51 cfs @ 12.15 hrs, Volume= 7,693 cf  
 Outflow = 2.51 cfs @ 12.15 hrs, Volume= 7,693 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 1R: ISOLATOR ROW C**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 29

**Summary for Reach 6R: ISOLATOR ROW 2**

[40] Hint: Not Described (Outflow=Inflow)

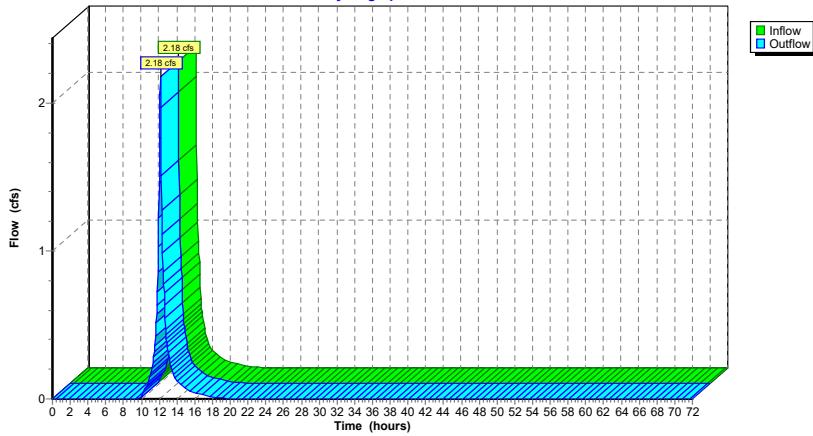
Inflow	=	2.18 cfs @ 12.14 hrs,	Volume=	7,000 cf
Outflow	=	2.18 cfs @ 12.14 hrs,	Volume=	7,000 cf, Atten= 0%, Lag= 0.0 min

Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 6R: ISOLATOR ROW 2**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 30

**Summary for Reach 15R: ISOLATOR ROW 1**

[40] Hint: Not Described (Outflow=Inflow)

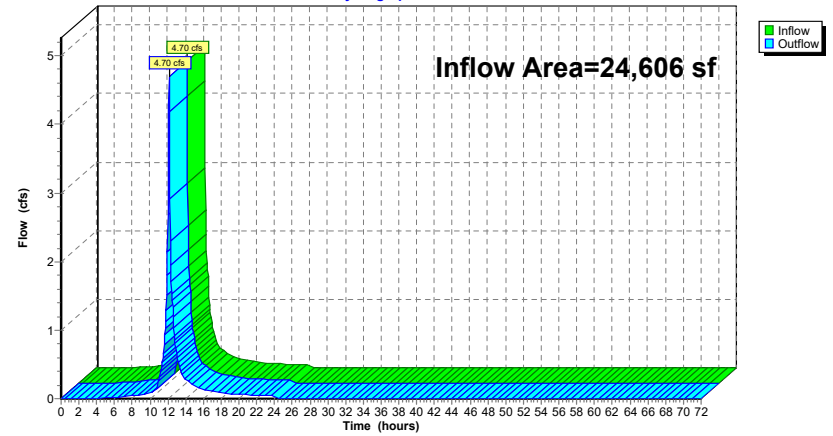
Inflow Area	=	24,606 sf, 82.04% Impervious,	Inflow Depth =	7.89" for NOAA 10-yr event
Inflow	=	4.70 cfs @ 12.15 hrs,	Volume=	16,169 cf
Outflow	=	4.70 cfs @ 12.15 hrs,	Volume=	16,169 cf, Atten= 0%, Lag= 0.0 min

Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 15R: ISOLATOR ROW 1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 31

**Summary for Reach B: PARKING LOT B OVERFLOW**

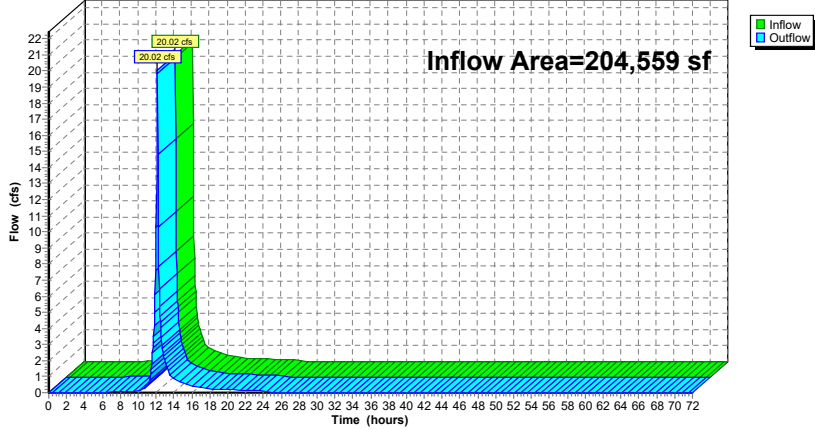
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 204,559 sf, 56.22% Impervious, Inflow Depth = 2.98" for NOAA 10-yr event  
 Inflow = 20.02 cfs @ 12.14 hrs, Volume= 50,822 cf  
 Outflow = 20.02 cfs @ 12.14 hrs, Volume= 50,822 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach B: PARKING LOT B OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 32

**Summary for Reach BMP4\_O: BMP-4 OVERFLOW**

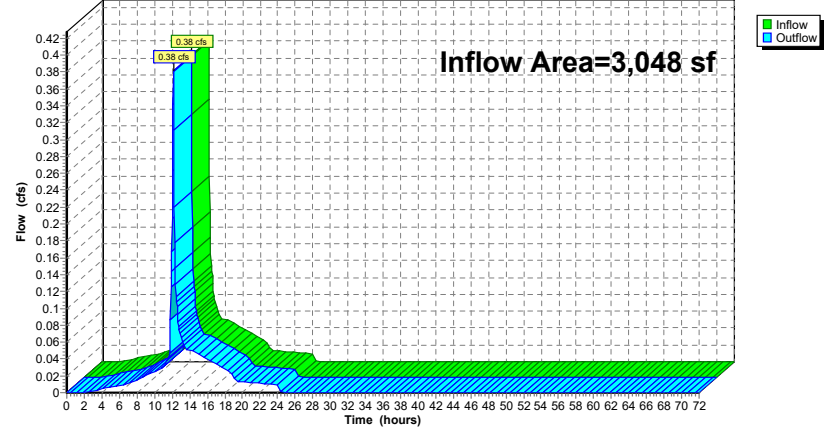
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3,048 sf, 86.09% Impervious, Inflow Depth = 8.80" for NOAA 10-yr event  
 Inflow = 0.38 cfs @ 12.14 hrs, Volume= 2,236 cf  
 Outflow = 0.38 cfs @ 12.14 hrs, Volume= 2,236 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP4\_O: BMP-4 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 33

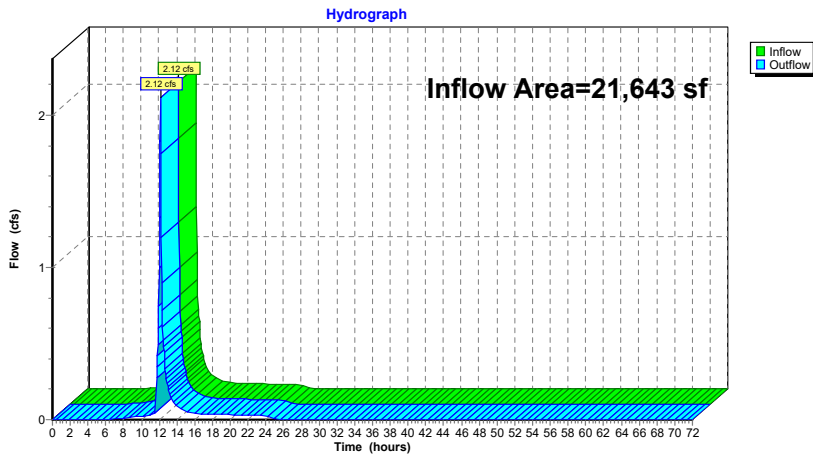
**Summary for Reach BMP6\_O: BMP-6 OVERFLOW**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 21,643 sf, 56.12% Impervious, Inflow Depth = 3.04" for NOAA 10-yr event  
 Inflow = 2.12 cfs @ 12.15 hrs, Volume= 5,487 cf  
 Outflow = 2.12 cfs @ 12.15 hrs, Volume= 5,487 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP6\_O: BMP-6 OVERFLOW**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 34

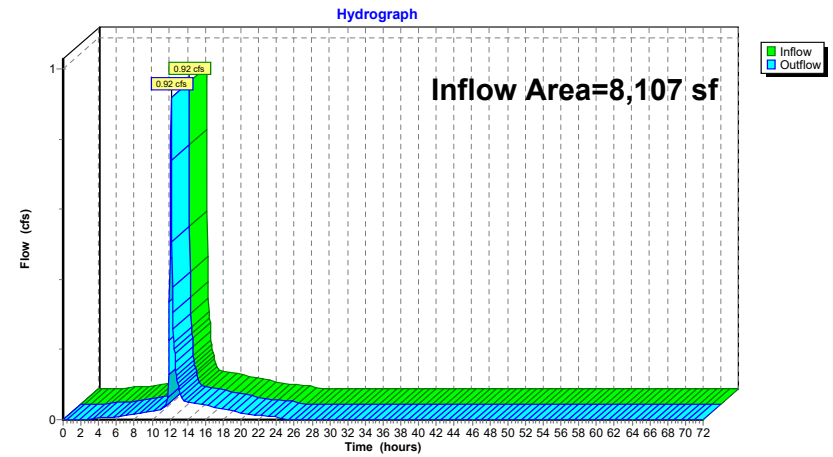
**Summary for Reach BMP7\_O: BMP-7 OVERFLOW**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8,107 sf, 88.34% Impervious, Inflow Depth = 4.44" for NOAA 10-yr event  
 Inflow = 0.92 cfs @ 12.15 hrs, Volume= 2,999 cf  
 Outflow = 0.92 cfs @ 12.15 hrs, Volume= 2,999 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP7\_O: BMP-7 OVERFLOW**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 35

**Summary for Reach BMP9\_O: BMP-9 OVERFLOW**

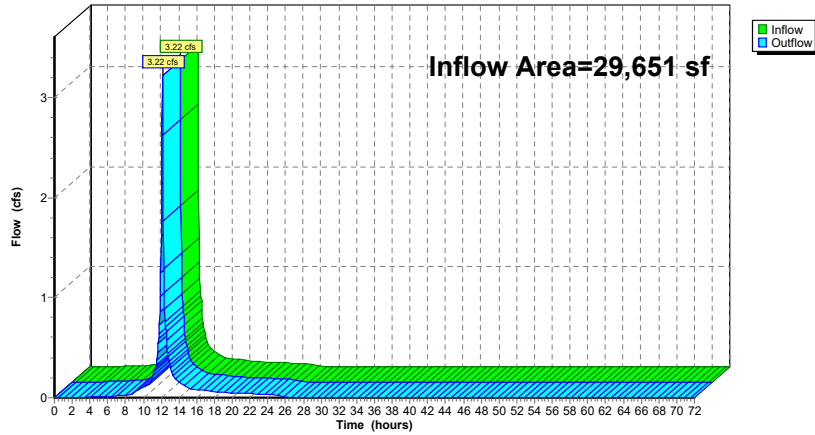
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 29,651 sf, 74.77% Impervious, Inflow Depth = 4.11" for NOAA 10-yr event  
 Inflow = 3.22 cfs @ 12.15 hrs, Volume= 10,153 cf  
 Outflow = 3.22 cfs @ 12.15 hrs, Volume= 10,153 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP9\_O: BMP-9 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 36

**Summary for Reach BMP\_3: BMP-3\_OVERFLOW**

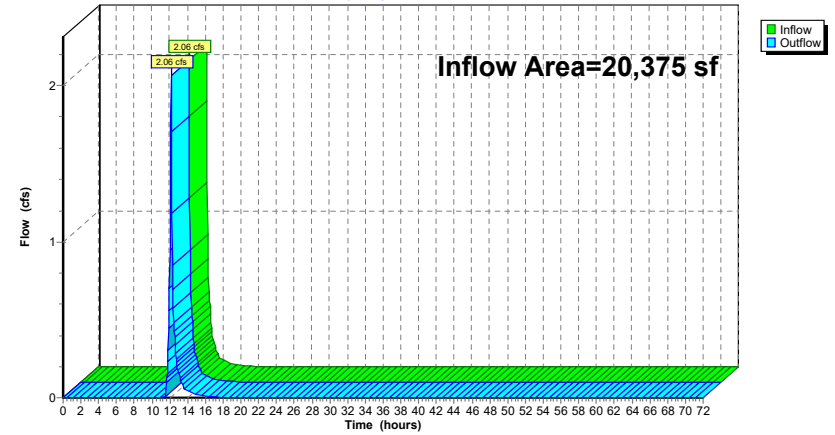
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 20,375 sf, 69.14% Impervious, Inflow Depth = 2.09" for NOAA 10-yr event  
 Inflow = 2.06 cfs @ 12.15 hrs, Volume= 3,545 cf  
 Outflow = 2.06 cfs @ 12.15 hrs, Volume= 3,545 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP\_3: BMP-3\_OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 37

**Summary for Reach DP-1: French Rodney Blvd 14" Outfall**

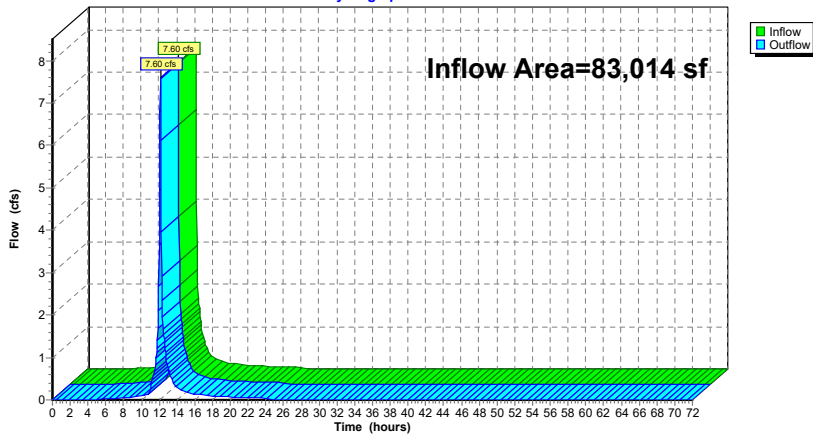
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 83,014 sf, 62.20% Impervious, Inflow Depth = 2.78" for NOAA 10-yr event  
Inflow = 7.60 cfs @ 12.14 hrs, Volume= 19,210 cf  
Outflow = 7.60 cfs @ 12.14 hrs, Volume= 19,210 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-1: French Rodney Blvd 14" Outfall**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 38

**Summary for Reach DP-2: NORTHERN OUTFALL**

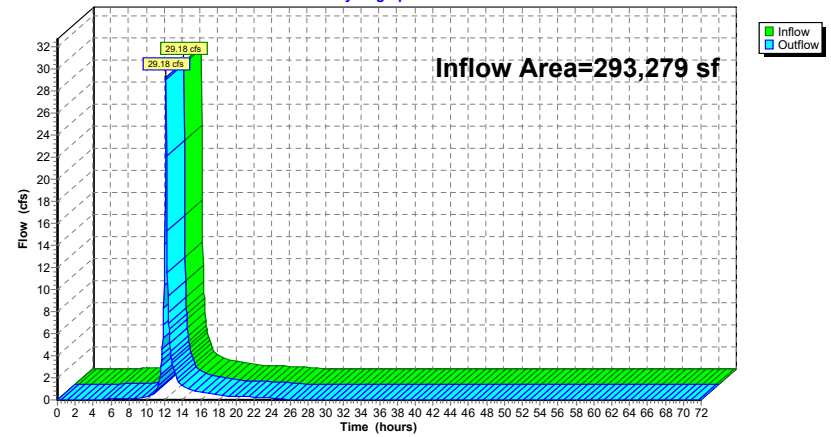
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 293,279 sf, 59.54% Impervious, Inflow Depth = 3.10" for NOAA 10-yr event  
Inflow = 29.18 cfs @ 12.14 hrs, Volume= 75,762 cf  
Outflow = 29.18 cfs @ 12.14 hrs, Volume= 75,762 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-2: NORTHERN OUTFALL**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 39

**Summary for Reach H ST: HUDSON STREET DRAINAGE**

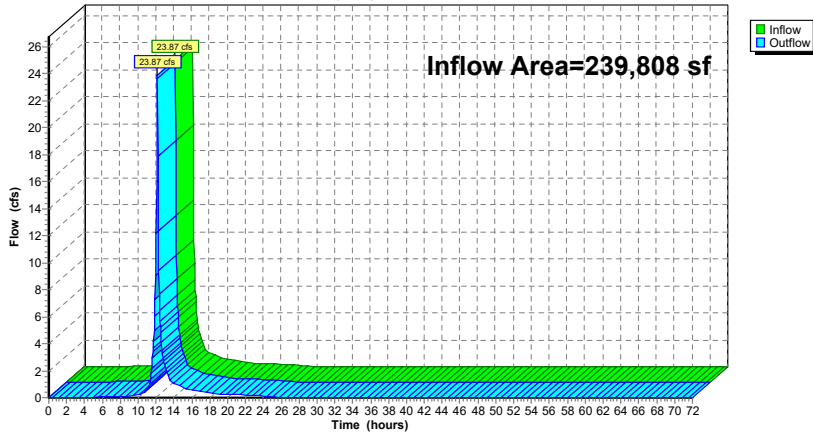
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 239,808 sf, 59.25% Impervious, Inflow Depth = 3.15" for NOAA 10-yr event  
 Inflow = 23.87 cfs @ 12.14 hrs, Volume= 63,045 cf  
 Outflow = 23.87 cfs @ 12.14 hrs, Volume= 63,045 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach DP-2 : NORTHERN OUTFALL

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach H ST: HUDSON STREET DRAINAGE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 40

**Summary for Reach P ST: PORTLAND STREET DRAINAGE**

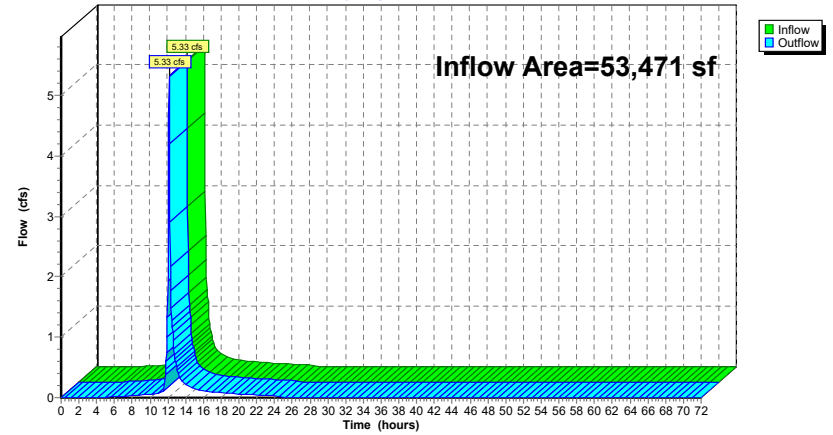
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 53,471 sf, 60.83% Impervious, Inflow Depth = 2.85" for NOAA 10-yr event  
 Inflow = 5.33 cfs @ 12.14 hrs, Volume= 12,717 cf  
 Outflow = 5.33 cfs @ 12.14 hrs, Volume= 12,717 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach DP-2 : NORTHERN OUTFALL

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach P ST: PORTLAND STREET DRAINAGE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 41

**Summary for Pond 1-P: BB 1**

Inflow Area = 38,826 sf, 51.66% Impervious, Inflow Depth = 3.49" for NOAA 10-yr event  
 Inflow = 3.85 cfs @ 12.13 hrs, Volume= 11,278 cf  
 Outflow = 3.72 cfs @ 12.15 hrs, Volume= 11,278 cf, Atten= 3%, Lag= 1.1 min  
 Discarded = 0.05 cfs @ 12.15 hrs, Volume= 2,554 cf  
 Primary = 1.61 cfs @ 12.15 hrs, Volume= 1,219 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall  
 Secondary = 2.06 cfs @ 12.15 hrs, Volume= 7,505 cf  
 Routed to Reach 15R : ISOLATOR ROW 1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.14' @ 12.15 hrs Surf.Area= 1,748 sf Storage= 1,014 cf

Plug-Flow detention time= 53.1 min calculated for 11,270 cf (100% of inflow)  
 Center-of-Mass det. time= 53.4 min ( 860.5 - 807.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.20'	1,114 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.20	490	0	0
9.50	800	194	194
10.20	1,830	920	1,114

Device	Routing	Invert	Outlet Devices
#1	Primary	8.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 ' S= 0.0100 ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Discarded	9.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.00'
#3	Device 1	10.00'	<b>24inch-Dome Grate Capacity X 2.00</b>
#4	Secondary	9.83'	<b>15inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.05 cfs @ 12.15 hrs HW=10.14' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.05 cfs)

**Primary OutFlow** Max=1.59 cfs @ 12.15 hrs HW=10.14' (Free Discharge)  
 ↳1=Culvert (Passes 1.59 cfs of 4.85 cfs potential flow)  
 ↳3=24inch-Dome Grate Capacity (Custom Controls 1.59 cfs)

**Secondary OutFlow** Max=2.05 cfs @ 12.15 hrs HW=10.14' (Free Discharge)  
 ↳4=15inch-Dome Grate Capacity (Custom Controls 2.05 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

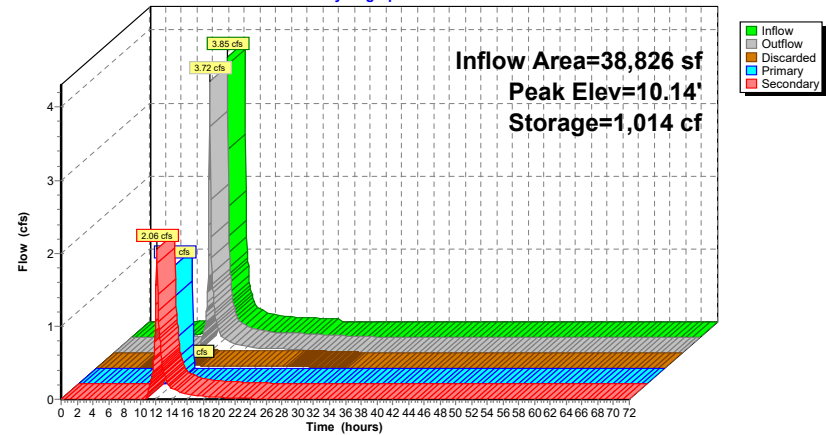
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 42

**Pond 1-P: BB 1**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 43

**Summary for Pond 2a-P: BB 2a**

Inflow Area = 3,116 sf, 92.62% Impervious, Inflow Depth = 4.55" for NOAA 10-yr event  
 Inflow = 0.37 cfs @ 12.13 hrs, Volume= 1,182 cf  
 Outflow = 0.35 cfs @ 12.15 hrs, Volume= 1,146 cf, Atten= 3%, Lag= 1.2 min  
 Primary = 0.35 cfs @ 12.15 hrs, Volume= 1,146 cf  
 Routed to Reach 15R : ISOLATOR ROW 1  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.19' @ 12.15 hrs Surf.Area= 469 sf Storage= 76 cf

Plug-Flow detention time= 36.9 min calculated for 1,145 cf (97% of inflow)  
 Center-of-Mass det. time= 18.0 min ( 781.7 - 763.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	710 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	320	0	0
9.00	1,100	710	710

Device	Routing	Invert	Outlet Devices
#1	Secondary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/ S= 0.0100 ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	8.50'	<b>24inch-Dome Grate Capacity X 2.00</b>
#3	Primary	8.10'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.35 cfs @ 12.15 hrs HW=8.19' (Free Discharge)  
 ↳3=15inch-Dome Grate Capacity (Custom Controls 0.35 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.00' (Free Discharge)  
 ↳1=Culvert (Passes 0.00 cfs of 2.27 cfs potential flow)  
 ↳2=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

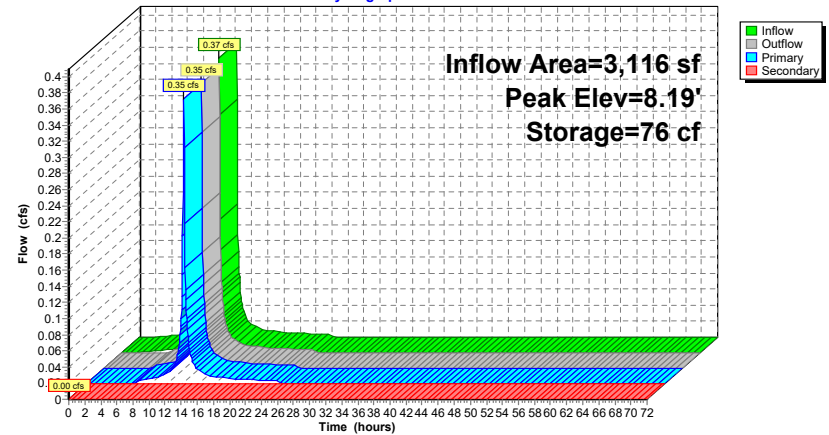
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 44

**Pond 2a-P: BB 2a**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 45

**Summary for Pond 2b-P: BB 2b**

Inflow Area = 21,490 sf, 80.50% Impervious, Inflow Depth = 4.22" for NOAA 10-yr event  
 Inflow = 2.43 cfs @ 12.13 hrs, Volume= 7,553 cf  
 Outflow = 2.28 cfs @ 12.15 hrs, Volume= 7,517 cf, Atten= 6%, Lag= 1.3 min  
 Primary = 2.28 cfs @ 12.15 hrs, Volume= 7,517 cf

Routed to Reach 15R : ISOLATOR ROW 1  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.46' @ 12.15 hrs Surf.Area= 677 sf Storage= 228 cf

Plug-Flow detention time= 7.8 min calculated for 7,512 cf (99% of inflow)  
 Center-of-Mass det. time= 4.7 min ( 784.9 - 780.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	710 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	320	0	0
9.00	1,100	710	710

Device	Routing	Invert	Outlet Devices
#1	Secondary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/ S= 0.0100 ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	8.50'	<b>24inch-Dome Grate Capacity X 2.00</b>
#3	Primary	8.10'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=2.28 cfs @ 12.15 hrs HW=8.46' (Free Discharge)  
 ↳3=15inch-Dome Grate Capacity (Custom Controls 2.28 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.00' (Free Discharge)  
 ↳1=Culvert (Passes 0.00 cfs of 2.27 cfs potential flow)  
 ↳2=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

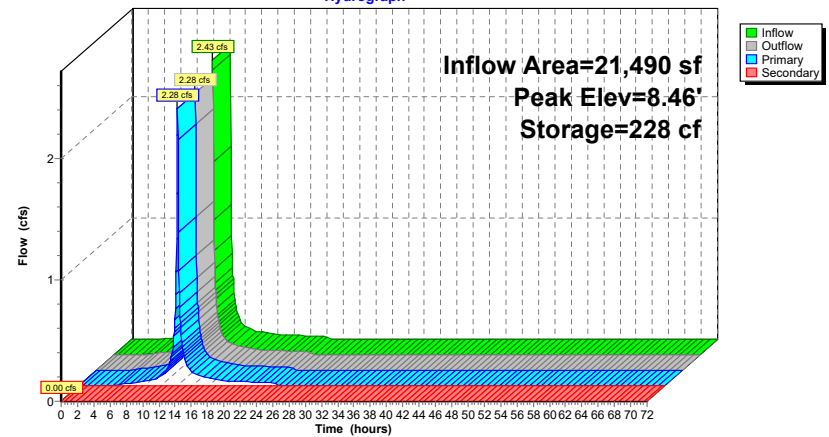
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 46

**Pond 2b-P: BB 2b**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 47

**Summary for Pond 3A-P: BB 3A**

Inflow Area = 10,987 sf, 58.16% Impervious, Inflow Depth = 3.69" for NOAA 10-yr event  
 Inflow = 1.14 cfs @ 12.13 hrs, Volume= 3,377 cf  
 Outflow = 1.09 cfs @ 12.15 hrs, Volume= 3,377 cf, Atten= 4%, Lag= 1.3 min  
 Discarded = 0.02 cfs @ 12.15 hrs, Volume= 1,314 cf  
 Primary = 1.07 cfs @ 12.15 hrs, Volume= 2,063 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.07' @ 12.15 hrs Surf.Area= 894 sf Storage= 449 cf

Plug-Flow detention time= 84.5 min calculated for 3,374 cf (100% of inflow)  
 Center-of-Mass det. time= 84.7 min ( 885.1 - 800.4 )

Volume #1	Invert 10.25'	Avail.Storage 622 cf	Storage Description Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.25	271	0	0
10.45	350	62	62
11.25	1,050	560	622

Device #1	Routing Primary	Invert 9.30'	Outlet Devices 10.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.30' / 9.20' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.55 sf
#2	Discarded	10.25'	1.020 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 7.30'
#3	Primary	11.15'	5.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Device 1	10.90'	24inch-Dome Grate Capacity

Discarded OutFlow Max=0.02 cfs @ 12.15 hrs HW=11.07' (Free Discharge)  
 2=Exfiltration ( Controls 0.02 cfs)

Primary OutFlow Max=1.07 cfs @ 12.15 hrs HW=11.07' (Free Discharge)  
 1=Culvert (Passes 1.07 cfs of 3.06 cfs potential flow)  
 4=24inch-Dome Grate Capacity (Custom Controls 1.07 cfs)  
 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

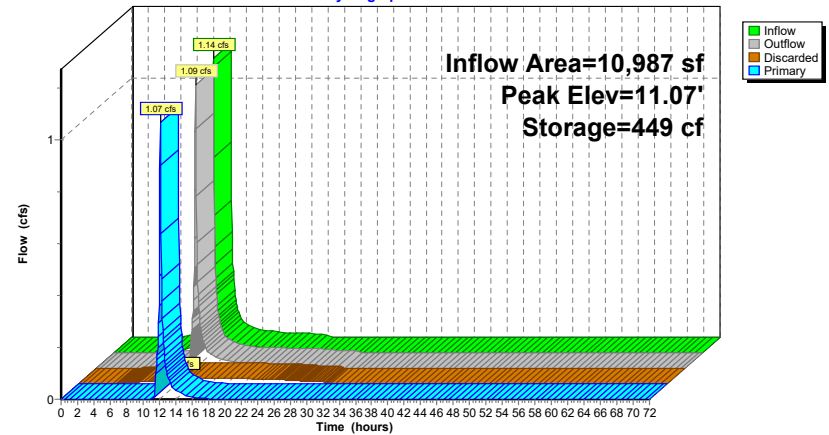
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 48

**Pond 3A-P: BB 3A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 49

**Summary for Pond 3B-P: BB 3B**

Inflow Area = 4,545 sf, 77.34% Impervious, Inflow Depth = 4.22" for NOAA 10-yr event  
 Inflow = 0.51 cfs @ 12.13 hrs, Volume= 1,597 cf  
 Outflow = 0.51 cfs @ 12.14 hrs, Volume= 1,597 cf, Atten= 1%, Lag= 1.0 min  
 Discarded = 0.01 cfs @ 12.14 hrs, Volume= 799 cf  
 Primary = 0.49 cfs @ 12.14 hrs, Volume= 798 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 12.89' @ 12.14 hrs Surf.Area= 562 sf Storage= 254 cf

Plug-Flow detention time= 96.8 min calculated for 1,597 cf (100% of inflow)  
 Center-of-Mass det. time= 96.7 min ( 876.9 - 780.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	12.20'	263 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
12.20	180	0	0
12.90	570	263	263

Device	Routing	Invert	Outlet Devices
#1	Primary	10.70'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.70' / 10.60' S= 0.0100 ' /' Cc= 0.900 n= 0.013, Flow Area= 0.55 sf
#2	Discarded	12.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 8.70'
#3	Device 1	12.80'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	12.85'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.01 cfs @ 12.14 hrs HW=12.88' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.01 cfs)

**Primary OutFlow** Max=0.48 cfs @ 12.14 hrs HW=12.88' (Free Discharge)  
 ↳1=Culvert (Passes 0.37 cfs of 3.49 cfs potential flow)  
 ↳3=24inch-Dome Grate Capacity (Custom Controls 0.37 cfs)  
 ↳4=Sharp-Crested Rectangular Weir (Weir Controls 0.11 cfs @ 0.61 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

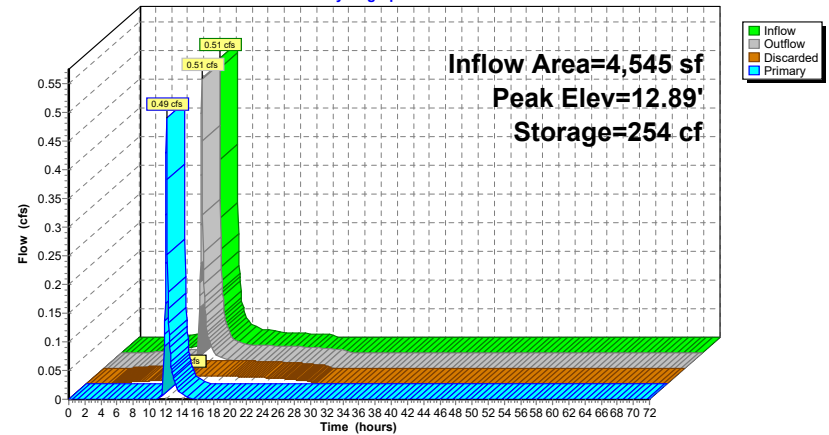
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 50

**Pond 3B-P: BB 3B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 51

**Summary for Pond 4A-P: BB 4A - POND**

Inflow Area = 4,843 sf, 86.37% Impervious, Inflow Depth = 4.44" for NOAA 10-yr event  
 Inflow = 0.56 cfs @ 12.13 hrs, Volume= 1,792 cf  
 Outflow = 0.54 cfs @ 12.15 hrs, Volume= 1,792 cf, Atten= 4%, Lag= 1.3 min  
 Primary = 0.51 cfs @ 12.15 hrs, Volume= 683 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW  
 Secondary = 0.04 cfs @ 12.15 hrs, Volume= 1,108 cf  
 Routed to Pond 4A-S : BB4A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.06' @ 12.15 hrs Surf.Area= 579 sf Storage= 230 cf

Plug-Flow detention time= 30.6 min calculated for 1,790 cf (100% of inflow)  
 Center-of-Mass det. time= 30.6 min ( 800.4 - 769.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.50'	320 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.50	250	0	0
10.20	664	320	320

Device	Routing	Invert	Outlet Devices
#1	Primary	8.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.00'
#3	Primary	10.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	9.95'	<b>24inchDome Grate Capacity</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950

**Primary OutFlow** Max=0.50 cfs @ 12.15 hrs HW=10.06' (Free Discharge)  
 1=Culvert (Passes 0.50 cfs of 4.72 cfs potential flow)  
 4=24inchDome Grate Capacity (Custom Controls 0.50 cfs)  
 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.15 hrs HW=10.06' (Free Discharge)  
 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

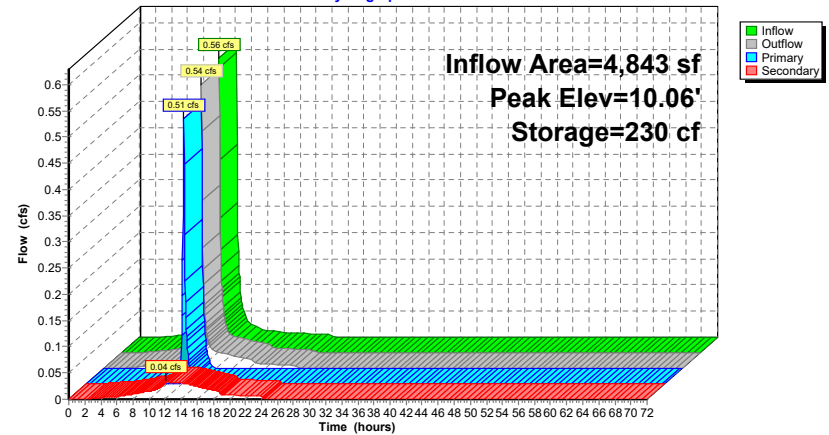
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 52

**Pond 4A-P: BB 4A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 53

**Summary for Pond 4A-S: BB4A-Stone**

Inflow = 0.04 cfs @ 12.15 hrs, Volume= 1,108 cf  
 Outflow = 0.04 cfs @ 12.18 hrs, Volume= 1,108 cf, Atten= 1%, Lag= 1.9 min  
 Primary = 0.04 cfs @ 12.18 hrs, Volume= 1,108 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.12' @ 12.18 hrs Surf.Area= 230 sf Storage= 9 cf

Plug-Flow detention time= 5.8 min calculated for 1,107 cf (100% of inflow)  
 Center-of-Mass det. time= 5.8 min ( 846.5 - 840.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	138 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 460 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	230	0	0
8.00	230	460	460

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.18 hrs HW=6.12' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.20 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

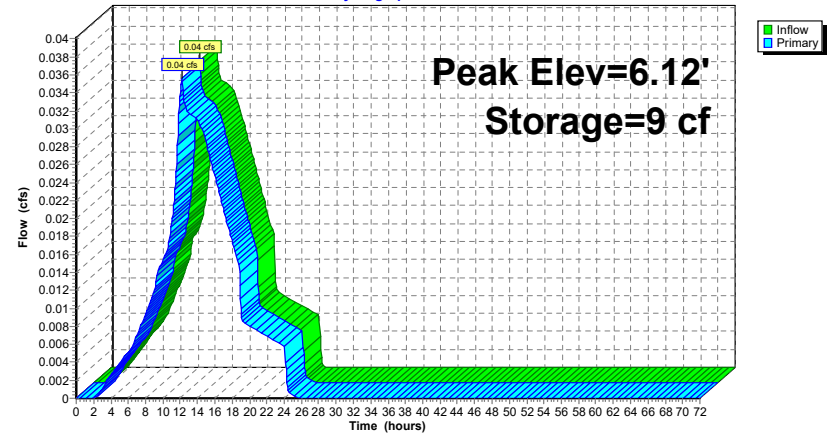
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 54

**Pond 4A-S: BB4A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 55

**Summary for Pond 4B-P: BB 4B - POND**

Inflow Area = 3,048 sf, 86.09% Impervious, Inflow Depth = 4.44" for NOAA 10-yr event  
 Inflow = 0.35 cfs @ 12.13 hrs, Volume= 1,128 cf  
 Outflow = 0.35 cfs @ 12.14 hrs, Volume= 1,128 cf, Atten= 2%, Lag= 1.0 min  
 Primary = 0.32 cfs @ 12.14 hrs, Volume= 437 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW  
 Secondary = 0.02 cfs @ 12.14 hrs, Volume= 690 cf  
 Routed to Pond 4B-S : BB 4A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.03' @ 12.14 hrs Surf.Area= 354 sf Storage= 131 cf

Plug-Flow detention time= 29.9 min calculated for 1,128 cf (100% of inflow)  
 Center-of-Mass det. time= 29.9 min ( 799.6 - 769.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.50'	199 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.50	144	0	0
11.20	424	199	199

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.00' / 8.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.00'
#3	Primary	11.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	10.95'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.32 cfs @ 12.14 hrs HW=11.02' (Free Discharge)

- 1=Culvert (Passes 0.32 cfs of 4.67 cfs potential flow)
- 4=24inch-Dome Grate Capacity (Custom Controls 0.32 cfs)
- 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.02 cfs @ 12.14 hrs HW=11.02' (Free Discharge)

- 2=Exfiltration ( Controls 0.02 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

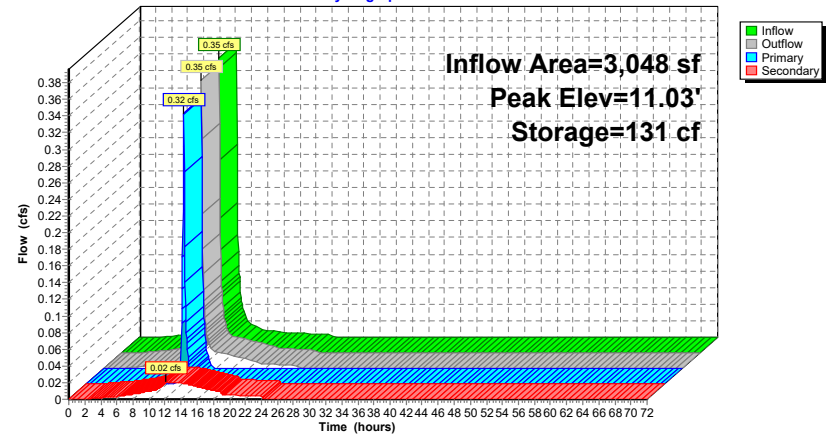
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 56

**Pond 4B-P: BB 4B - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 57

**Summary for Pond 4B-S: BB 4A-Stone**

Inflow = 0.02 cfs @ 12.14 hrs, Volume= 690 cf  
 Outflow = 0.02 cfs @ 12.17 hrs, Volume= 690 cf, Atten= 0%, Lag= 1.5 min  
 Primary = 0.02 cfs @ 12.17 hrs, Volume= 690 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.10' @ 12.17 hrs Surf.Area= 145 sf Storage= 4 cf

Plug-Flow detention time= 4.6 min calculated for 690 cf (100% of inflow)  
 Center-of-Mass det. time= 4.6 min ( 845.9 - 841.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	87 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 290 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	145	0	0
8.00	145	290	290

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.17 hrs HW=6.10' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.02 cfs @ 1.05 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

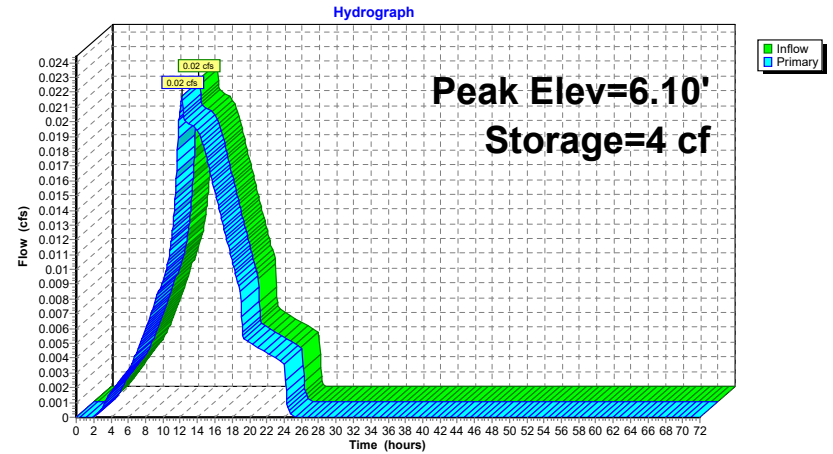
Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 58

**Pond 4B-S: BB 4A-Stone**





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 59

**Summary for Pond 5A-P: BB 5A - POND**

Inflow Area = 3,072 sf, 73.44% Impervious, Inflow Depth = 4.11" for NOAA 10-yr event  
 Inflow = 0.34 cfs @ 12.13 hrs, Volume= 1,052 cf  
 Outflow = 0.04 cfs @ 12.69 hrs, Volume= 1,052 cf, Atten= 87%, Lag= 33.5 min  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW  
 Secondary = 0.04 cfs @ 12.69 hrs, Volume= 1,052 cf  
 Routed to Pond 5A-PS : BB 5A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.43' @ 12.69 hrs Surf.Area= 688 sf Storage= 369 cf

Plug-Flow detention time= 65.3 min calculated for 1,052 cf (100% of inflow)  
 Center-of-Mass det. time= 65.2 min ( 850.0 - 784.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.80'	645 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.80	480	0	0
9.80	810	645	645

Device	Routing	Invert	Outlet Devices
#1	Primary	7.60'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.60' / 7.50' S= 0.0100 '/ S Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.80'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.60'
#3	Device 1	9.50'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.80' (Free Discharge)  
 ↑1=Culvert (Passes 0.00 cfs of 2.86 cfs potential flow)  
 ↑3=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.69 hrs HW=9.43' (Free Discharge)  
 ↑2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

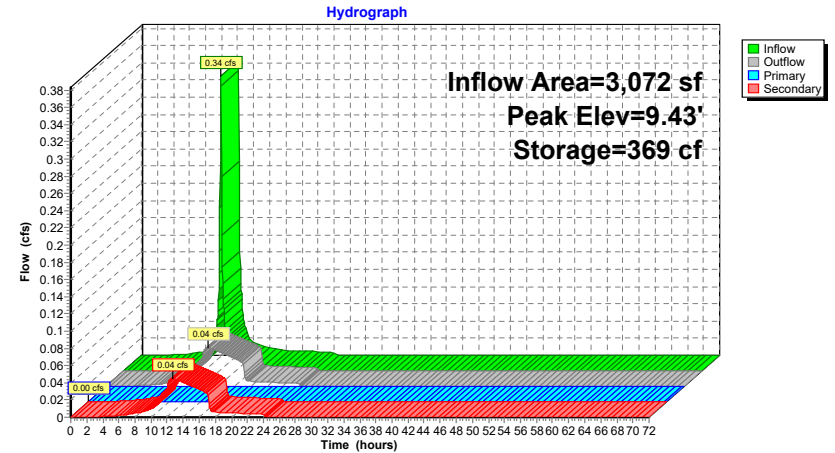
Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 60

**Pond 5A-P: BB 5A - POND**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 61

**Summary for Pond 5A-PS: BB 5A-Stone**

Inflow = 0.04 cfs @ 12.69 hrs, Volume= 1,052 cf  
 Outflow = 0.04 cfs @ 12.78 hrs, Volume= 1,052 cf, Atten= 0%, Lag= 5.8 min  
 Primary = 0.04 cfs @ 12.78 hrs, Volume= 1,052 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.14' @ 12.78 hrs Surf.Area= 480 sf Storage= 20 cf

Plug-Flow detention time= 11.2 min calculated for 1,051 cf (100% of inflow)  
 Center-of-Mass det. time= 11.3 min ( 861.3 - 850.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	288 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 960 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	480	0	0
8.00	480	960	960

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.78 hrs HW=6.14' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.28 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

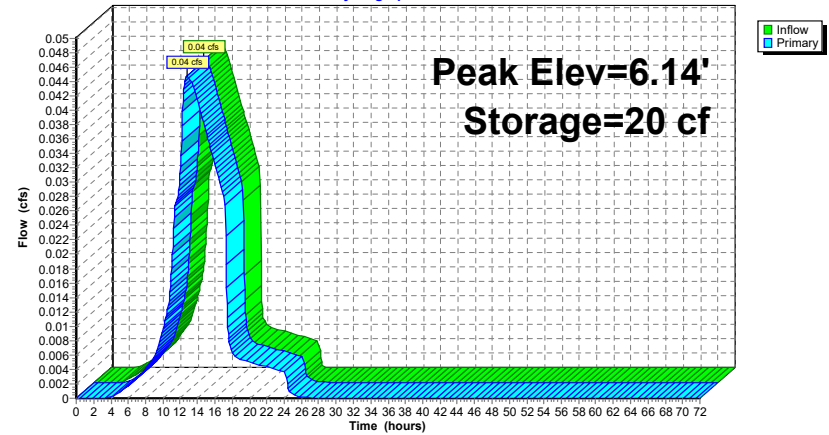
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 62

**Pond 5A-PS: BB 5A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 63

**Summary for Pond 5B-P: BB 5B - POND**

Inflow Area = 34,755 sf, 71.39% Impervious, Inflow Depth = 4.00" for NOAA 10-yr event  
 Inflow = 3.81 cfs @ 12.13 hrs, Volume= 11,589 cf  
 Outflow = 3.74 cfs @ 12.14 hrs, Volume= 11,589 cf, Atten= 2%, Lag= 0.9 min  
 Primary = 1.48 cfs @ 12.14 hrs, Volume= 934 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW  
 Secondary = 0.08 cfs @ 12.14 hrs, Volume= 3,656 cf  
 Routed to Pond 5B-PS : BB 5B-Stone  
 Tertiary = 2.18 cfs @ 12.14 hrs, Volume= 7,000 cf  
 Routed to Reach 6R : ISOLATOR ROW 2

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.04' @ 12.14 hrs Surf.Area= 1,265 sf Storage= 665 cf

Plug-Flow detention time= 27.0 min calculated for 11,581 cf (100% of inflow)  
 Center-of-Mass det. time= 27.0 min ( 816.1 - 789.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.20'	889 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.20	327	0	0
9.20	1,450	889	889

Device	Routing	Invert	Outlet Devices
#1	Primary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.20'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.00'
#3	Device 1	8.90'	<b>24inchDome Grate Capacity X 2.00</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Tertiary	8.70'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=1.43 cfs @ 12.14 hrs HW=9.03' (Free Discharge)

↳ **1=Culvert** (Passes 1.43 cfs of 4.68 cfs potential flow)

↳ **3=24inchDome Grate Capacity** (Custom Controls 1.43 cfs)

**Secondary OutFlow** Max=0.08 cfs @ 12.14 hrs HW=9.03' (Free Discharge)

↳ **2=Exfiltration** ( Controls 0.08 cfs)

**Tertiary OutFlow** Max=2.16 cfs @ 12.14 hrs HW=9.03' (Free Discharge)

↳ **4=15inch-Dome Grate Capacity** (Custom Controls 2.16 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

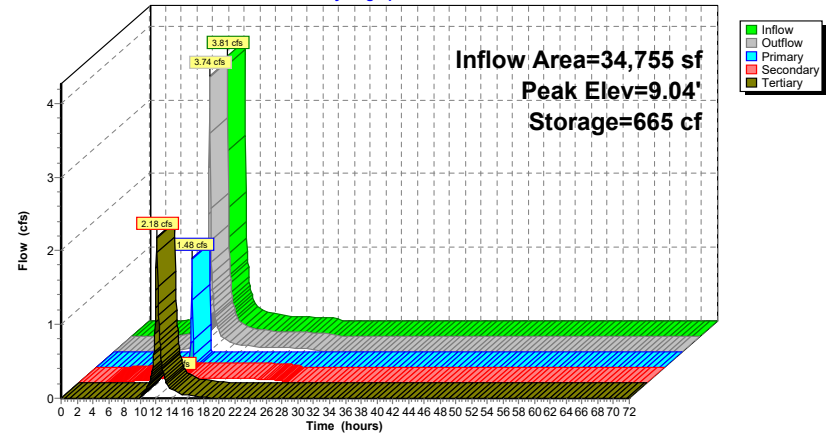
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 64

**Pond 5B-P: BB 5B - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 65

**Summary for Pond 5B-PS: BB 5B-Stone**

[44] Hint: Outlet device #1 is below defined storage

Inflow = 0.08 cfs @ 12.14 hrs, Volume= 3,656 cf  
 Outflow = 0.08 cfs @ 12.15 hrs, Volume= 3,656 cf, Atten= 0%, Lag= 0.1 min  
 Primary = 0.08 cfs @ 12.15 hrs, Volume= 3,656 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.00' @ 12.15 hrs Surf.Area= 690 sf Storage= 1 cf

Plug-Flow detention time= 0.1 min calculated for 3,653 cf (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 959.6 - 959.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	414 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 1,380 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	690	0	0
8.00	690	1,380	1,380

Device	Routing	Invert	Outlet Devices
#1	Primary	4.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.57 cfs @ 12.15 hrs HW=6.00' (Free Discharge)  
 1=Orifice/Grate (Orifice Controls 0.57 cfs @ 6.52 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

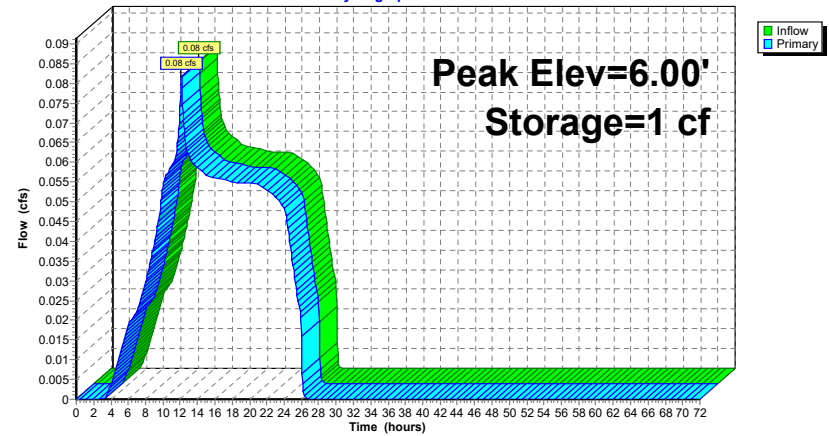
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 66

**Pond 5B-PS: BB 5B-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 67

**Summary for Pond 6A-P: BB 6A - POND**

Inflow Area = 15,148 sf, 46.97% Impervious, Inflow Depth = 3.39" for NOAA 10-yr event  
 Inflow = 1.47 cfs @ 12.13 hrs, Volume= 4,275 cf  
 Outflow = 1.43 cfs @ 12.15 hrs, Volume= 4,275 cf, Atten= 3%, Lag= 1.0 min  
 Primary = 1.38 cfs @ 12.15 hrs, Volume= 2,347 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW  
 Secondary = 0.05 cfs @ 12.15 hrs, Volume= 1,928 cf  
 Routed to Pond 6A-PS : BB 6A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.00' @ 12.15 hrs Surf.Area= 698 sf Storage= 420 cf

Plug-Flow detention time= 41.9 min calculated for 4,272 cf (100% of inflow)  
 Center-of-Mass det. time= 41.9 min ( 852.2 - 810.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.20'	491 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.20	350	0	0
11.10	740	491	491

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.00' / 8.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.20'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.00'
#3	Device 1	10.80'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	11.00'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=1.36 cfs @ 12.15 hrs HW=11.00' (Free Discharge)

- 1=Culvert (Passes 1.36 cfs of 4.63 cfs potential flow)
- 3=24inch-Dome Grate Capacity (Custom Controls 1.36 cfs)
- 4=Sharp-Crested Rectangular Weir (Weir Controls 0.00 cfs @ 0.07 fps)

**Secondary OutFlow** Max=0.05 cfs @ 12.15 hrs HW=11.00' (Free Discharge)

- 2=Exfiltration ( Controls 0.05 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

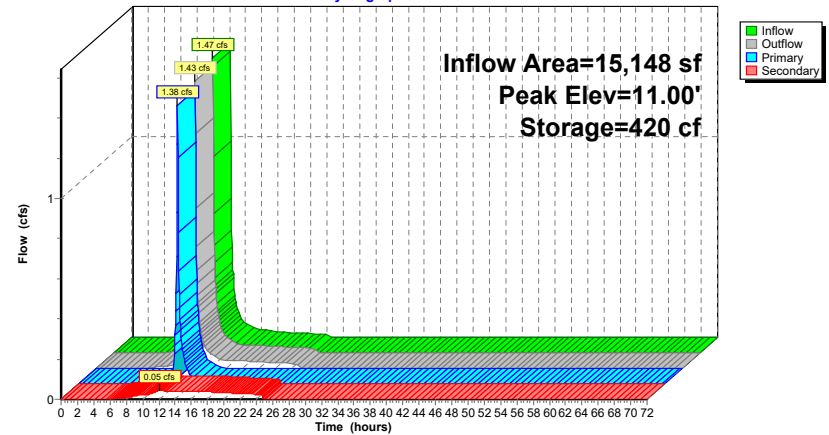
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 68

**Pond 6A-P: BB 6A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 69

**Summary for Pond 6A-PS: BB 6A - STONE**

Inflow = 0.05 cfs @ 12.15 hrs, Volume= 1,928 cf  
 Outflow = 0.05 cfs @ 12.18 hrs, Volume= 1,928 cf, Atten= 1%, Lag= 2.2 min  
 Primary = 0.05 cfs @ 12.18 hrs, Volume= 1,928 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.14' @ 12.18 hrs Surf.Area= 290 sf Storage= 12 cf

Plug-Flow detention time= 5.4 min calculated for 1,926 cf (100% of inflow)  
 Center-of-Mass det. time= 5.5 min ( 987.9 - 982.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	174 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 580 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	290	0	0
8.00	290	580	580

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.05 cfs @ 12.18 hrs HW=6.14' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.05 cfs @ 1.28 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

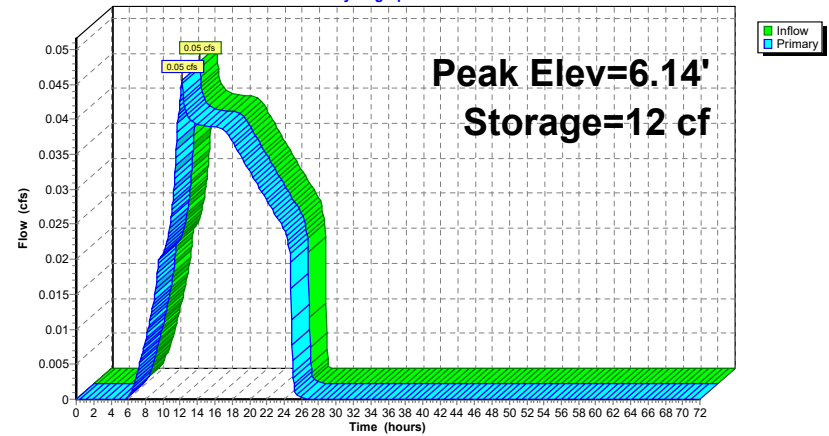
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 70

**Pond 6A-PS: BB 6A - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 71

**Summary for Pond 6B-P: BB 6B**

Inflow Area = 6,495 sf, 77.45% Impervious, Inflow Depth = 4.22" for NOAA 10-yr event  
 Inflow = 0.73 cfs @ 12.13 hrs, Volume= 2,283 cf  
 Outflow = 0.71 cfs @ 12.15 hrs, Volume= 2,283 cf, Atten= 4%, Lag= 1.3 min  
 Discarded = 0.02 cfs @ 12.15 hrs, Volume= 1,071 cf  
 Primary = 0.69 cfs @ 12.15 hrs, Volume= 1,212 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 12.08' @ 12.15 hrs Surf.Area= 678 sf Storage= 380 cf

Plug-Flow detention time= 110.1 min calculated for 2,281 cf (100% of inflow)  
 Center-of-Mass det. time= 110.3 min ( 890.5 - 780.2 )

Volume #1	Invert 11.20'	Avail.Storage 394 cf	Storage Description Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
11.20	185	0	0
12.10	690	394	394

Device #1	Routing Primary	Invert 10.10'	Outlet Devices 12.0" Round Culvert
#2	Discarded	11.20'	1.020 in/hr Exfiltration over Surface area
#3	Device 1	11.95'	24inch-Dome Grate Capacity

Discarded OutFlow Max=0.02 cfs @ 12.15 hrs HW=12.08' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.02 cfs)

Primary OutFlow Max=0.68 cfs @ 12.15 hrs HW=12.08' (Free Discharge)  
 ↳1=Culvert (Passes 0.68 cfs of 4.60 cfs potential flow)  
 ↳3=24inch-Dome Grate Capacity (Custom Controls 0.68 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

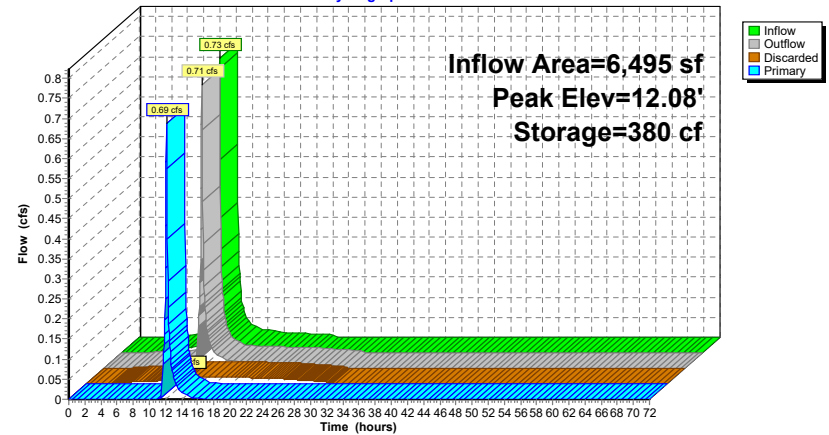
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 72

**Pond 6B-P: BB 6B**

**Hydrograph**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 73

**Summary for Pond 7A-P: BB 7A PONDING**

Inflow Area = 3,165 sf, 87.74% Impervious, Inflow Depth = 4.44" for NOAA 10-yr event  
 Inflow = 0.37 cfs @ 12.13 hrs, Volume= 1,171 cf  
 Outflow = 0.36 cfs @ 12.14 hrs, Volume= 1,171 cf, Atten= 2%, Lag= 0.9 min  
 Primary = 0.34 cfs @ 12.14 hrs, Volume= 465 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW  
 Secondary = 0.02 cfs @ 12.14 hrs, Volume= 706 cf  
 Routed to Pond 7A-S : BB 7A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.98' @ 12.14 hrs Surf.Area= 322 sf Storage= 148 cf

Plug-Flow detention time= 39.4 min calculated for 1,171 cf (100% of inflow)  
 Center-of-Mass det. time= 39.4 min ( 809.1 - 769.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.30'	227 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.30	115	0	0
10.20	390	227	227

Device	Routing	Invert	Outlet Devices
#1	Primary	8.10'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.10' / 8.00' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.30'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.10'
#3	Device 1	9.90'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	10.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.33 cfs @ 12.14 hrs HW=9.98' (Free Discharge)

- 1=Culvert (Passes 0.33 cfs of 4.44 cfs potential flow)
- 3=24inch-Dome Grate Capacity (Custom Controls 0.33 cfs)
- 4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.02 cfs @ 12.14 hrs HW=9.98' (Free Discharge)

- 2=Exfiltration ( Controls 0.02 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

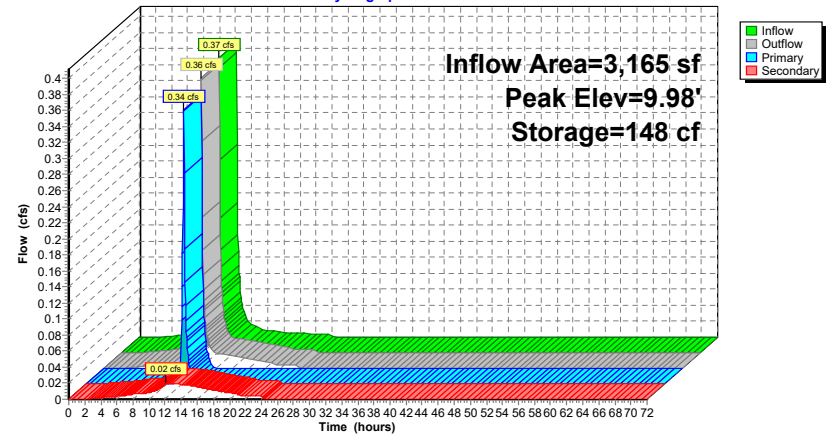
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 74

**Pond 7A-P: BB 7A PONDING**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 75

**Summary for Pond 7A-S: BB 7A - STONE**

Inflow = 0.02 cfs @ 12.14 hrs, Volume= 706 cf  
 Outflow = 0.02 cfs @ 12.17 hrs, Volume= 706 cf, Atten= 0%, Lag= 1.5 min  
 Primary = 0.02 cfs @ 12.17 hrs, Volume= 706 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5.19' @ 12.17 hrs Surf.Area= 150 sf Storage= 4 cf

Plug-Flow detention time= 4.8 min calculated for 706 cf (100% of inflow)  
 Center-of-Mass det. time= 4.7 min ( 863.0 - 858.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	5.10'	90 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 300 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
5.10	150	0	0
7.10	150	300	300

Device	Routing	Invert	Outlet Devices
#1	Primary	5.10'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.17 hrs HW=5.19' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.02 cfs @ 1.03 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

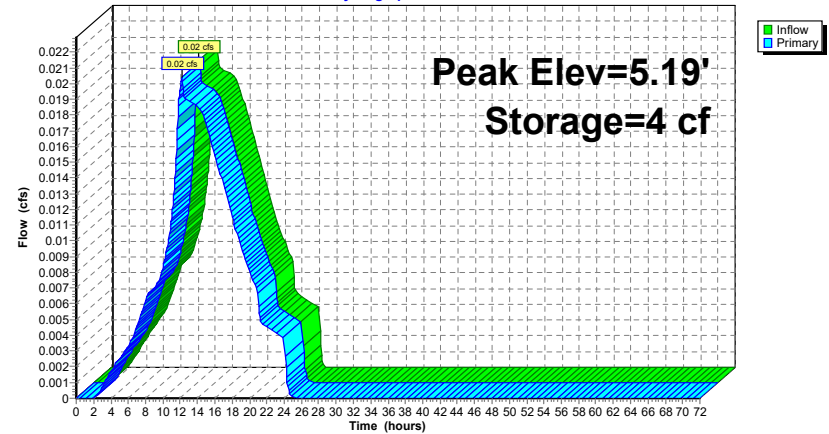
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 76

**Pond 7A-S: BB 7A - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 77

**Summary for Pond 7B-P: BB 7B PONDING**

Inflow Area = 4,942 sf, 88.73% Impervious, Inflow Depth = 4.44" for NOAA 10-yr event  
 Inflow = 0.57 cfs @ 12.13 hrs, Volume= 1,828 cf  
 Outflow = 0.56 cfs @ 12.15 hrs, Volume= 1,828 cf, Atten= 3%, Lag= 1.2 min  
 Primary = 0.52 cfs @ 12.15 hrs, Volume= 676 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW  
 Secondary = 0.03 cfs @ 12.15 hrs, Volume= 1,153 cf  
 Routed to Pond 7B-S : BB 7B - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.71' @ 12.15 hrs Surf.Area= 524 sf Storage= 274 cf

Plug-Flow detention time= 41.6 min calculated for 1,828 cf (100% of inflow)  
 Center-of-Mass det. time= 41.6 min ( 811.3 - 769.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	324 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	250	0	0
10.80	560	324	324

Device	Routing	Invert	Outlet Devices
#1	Primary	8.90'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.90' / 8.80' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.00'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.90'
#3	Device 1	10.60'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.52 cfs @ 12.15 hrs HW=10.71' (Free Discharge)  
 1=Culvert (Passes 0.52 cfs of 4.32 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 0.52 cfs)

**Secondary OutFlow** Max=0.03 cfs @ 12.15 hrs HW=10.71' (Free Discharge)  
 2=Exfiltration ( Controls 0.03 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

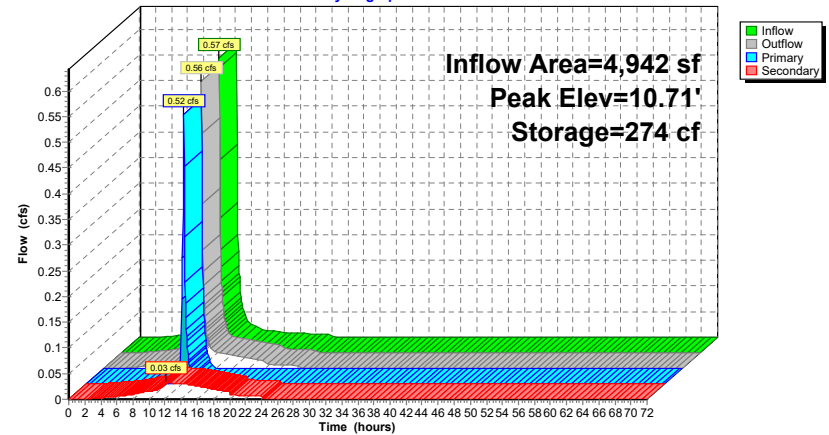
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 78

**Pond 7B-P: BB 7B PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 79

**Summary for Pond 7B-S: BB 7B - STONE**

Inflow = 0.03 cfs @ 12.15 hrs, Volume= 1,153 cf  
 Outflow = 0.03 cfs @ 12.17 hrs, Volume= 1,153 cf, Atten= 0%, Lag= 1.3 min  
 Primary = 0.03 cfs @ 12.17 hrs, Volume= 1,153 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5.22' @ 12.17 hrs Surf.Area= 150 sf Storage= 5 cf

Plug-Flow detention time= 3.7 min calculated for 1,152 cf (100% of inflow)  
 Center-of-Mass det. time= 3.7 min ( 859.1 - 855.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	5.10'	90 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 300 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
5.10	150	0	0
7.10	150	300	300

Device	Routing	Invert	Outlet Devices
#1	Primary	5.10'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.03 cfs @ 12.17 hrs HW=5.22' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.18 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

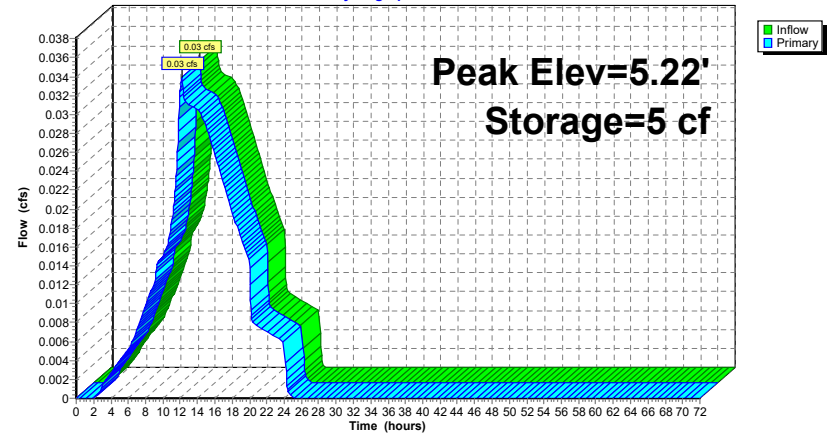
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 80

**Pond 7B-S: BB 7B - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 81

**Summary for Pond 8a-P: BB 8A PONDING**

Inflow Area = 3,978 sf, 79.99% Impervious, Inflow Depth = 4.22" for NOAA 10-yr event  
 Inflow = 0.45 cfs @ 12.13 hrs, Volume= 1,398 cf  
 Outflow = 0.42 cfs @ 12.16 hrs, Volume= 1,398 cf, Atten= 6%, Lag= 2.0 min  
 Primary = 0.38 cfs @ 12.16 hrs, Volume= 357 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE  
 Secondary = 0.04 cfs @ 12.16 hrs, Volume= 1,041 cf  
 Routed to Pond 8a-s : BB 8A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.09' @ 12.16 hrs Surf.Area= 612 sf Storage= 285 cf

Plug-Flow detention time= 39.7 min calculated for 1,398 cf (100% of inflow)  
 Center-of-Mass det. time= 39.7 min ( 819.9 - 780.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.50'	575 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.50	360	0	0
9.50	790	575	575

Device	Routing	Invert	Outlet Devices
#1	Primary	7.40'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.40' / 7.30' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.40'
#3	Device 1	9.00'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	9.40'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.37 cfs @ 12.16 hrs HW=9.08' (Free Discharge)

- 1=Culvert (Passes 0.37 cfs of 4.11 cfs potential flow)
- 3=24inch-Dome Grate Capacity (Custom Controls 0.37 cfs)
- 4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.16 hrs HW=9.08' (Free Discharge)

- 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

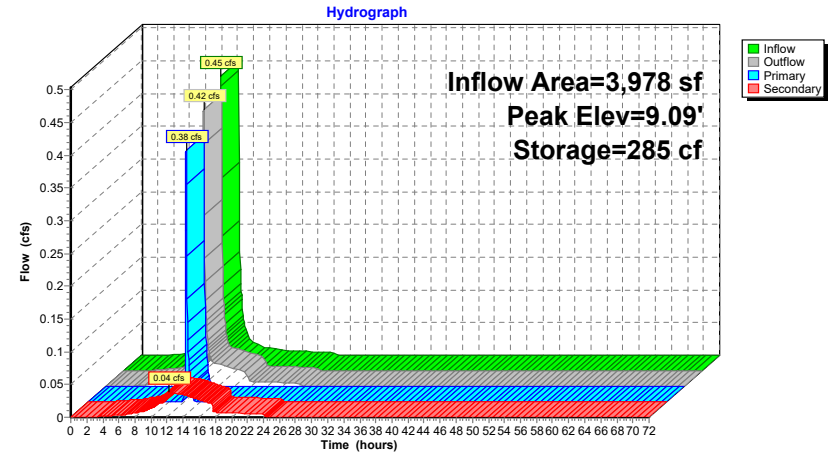
Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 82

**Pond 8a-P: BB 8A PONDING**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 83

**Summary for Pond 8a-s: BB 8A - STONE**

Inflow = 0.04 cfs @ 12.16 hrs, Volume= 1,041 cf  
 Outflow = 0.04 cfs @ 12.22 hrs, Volume= 1,041 cf, Atten= 1%, Lag= 3.3 min  
 Primary = 0.04 cfs @ 12.22 hrs, Volume= 1,041 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 4.53' @ 12.22 hrs Surf.Area= 300 sf Storage= 12 cf

Plug-Flow detention time= 7.4 min calculated for 1,041 cf (100% of inflow)  
 Center-of-Mass det. time= 7.4 min ( 855.4 - 848.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	4.40'	180 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 600 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
4.40	300	0	0
6.40	300	600	600

Device	Routing	Invert	Outlet Devices
#1	Primary	4.40'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.22 hrs HW=4.53' (Free Discharge)  
 ↳=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.23 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

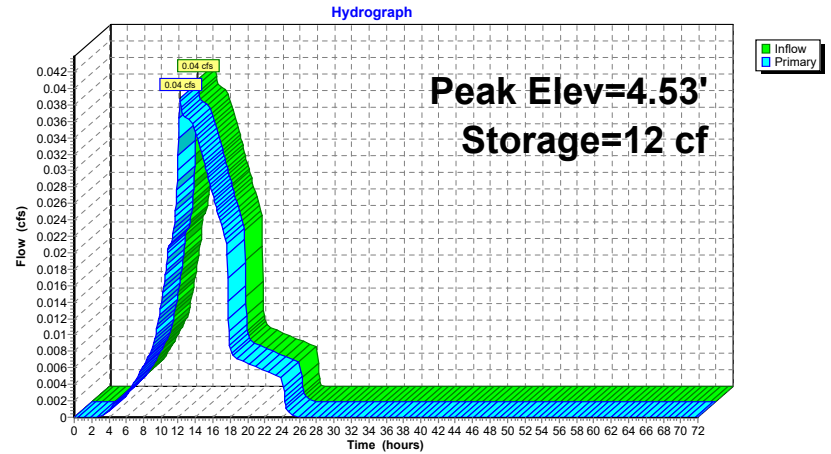
Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 84

**Pond 8a-s: BB 8A - STONE**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 85

**Summary for Pond 8B-P: BB 8B-PONDING**

Inflow Area = 5,598 sf, 87.78% Impervious, Inflow Depth = 4.44" for NOAA 10-yr event  
 Inflow = 0.65 cfs @ 12.13 hrs, Volume= 2,071 cf  
 Outflow = 0.64 cfs @ 12.14 hrs, Volume= 2,071 cf, Atten=2%, Lag= 0.9 min  
 Primary = 0.60 cfs @ 12.14 hrs, Volume= 805 cf  
 Routed to Reach H ST : HUDSON STREET DRAINAGE  
 Secondary = 0.04 cfs @ 12.14 hrs, Volume= 1,266 cf  
 Routed to Pond 8B-S : BB 8B-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.72' @ 12.14 hrs Surf.Area= 629 sf Storage= 255 cf

Plug-Flow detention time= 35.5 min calculated for 2,071 cf (100% of inflow)  
 Center-of-Mass det. time= 35.5 min ( 805.2 - 769.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.10'	306 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.10	190	0	0
9.80	685	306	306

Device	Routing	Invert	Outlet Devices
#1	Primary	7.90'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.90' / 7.80' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.10'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.90'
#3	Device 1	9.65'	<b>24inch-Dome Grate Capacity X 2.00</b>

**Primary OutFlow** Max=0.59 cfs @ 12.14 hrs HW=9.72' (Free Discharge)  
 1=Culvert (Passes 0.59 cfs of 4.34 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 0.59 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.14 hrs HW=9.72' (Free Discharge)  
 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

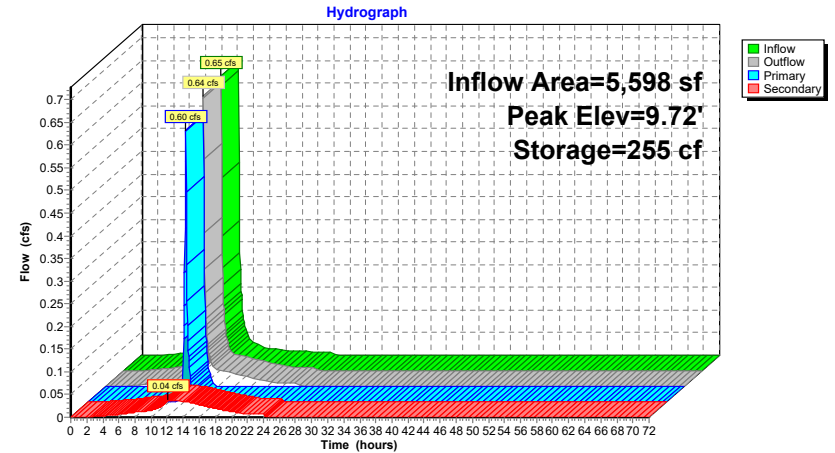
Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 86

**Pond 8B-P: BB 8B-PONDING**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 87

**Summary for Pond 8B-S: BB 8B-Stone**

Inflow = 0.04 cfs @ 12.14 hrs, Volume= 1,266 cf  
 Outflow = 0.04 cfs @ 12.18 hrs, Volume= 1,266 cf, Atten= 1%, Lag= 2.2 min  
 Primary = 0.04 cfs @ 12.18 hrs, Volume= 1,266 cf  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 4.53' @ 12.18 hrs Surf.Area= 300 sf Storage= 12 cf

Plug-Flow detention time= 7.4 min calculated for 1,266 cf (100% of inflow)  
 Center-of-Mass det. time= 7.1 min ( 857.7 - 850.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	4.40'	180 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 600 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
4.40	300	0	0
6.40	300	600	600

Device	Routing	Invert	Outlet Devices
#1	Primary	4.40'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.18 hrs HW=4.53' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.23 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

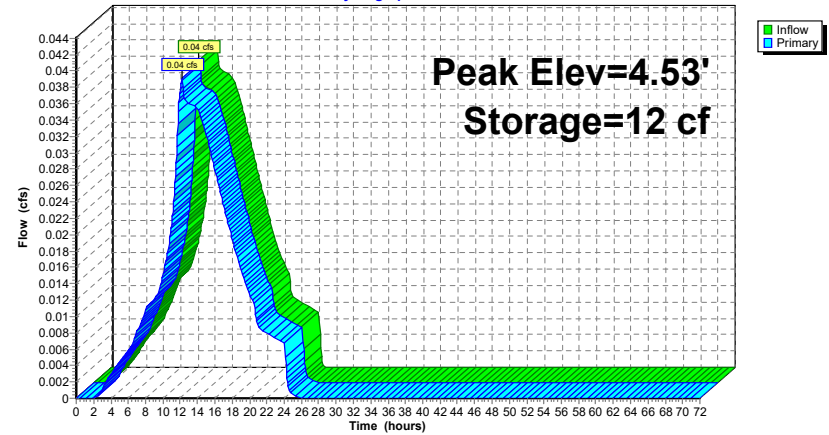
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 88

**Pond 8B-S: BB 8B-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 89

**Summary for Pond 9-P: BB9 - POND**

Inflow Area = 29,651 sf, 74.77% Impervious, Inflow Depth = 4.11" for NOAA 10-yr event  
 Inflow = 3.30 cfs @ 12.13 hrs, Volume= 10,153 cf  
 Outflow = 3.22 cfs @ 12.15 hrs, Volume= 10,153 cf, Atten=2%, Lag= 1.1 min  
 Primary = 0.67 cfs @ 12.15 hrs, Volume= 245 cf  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW  
 Secondary = 0.05 cfs @ 12.15 hrs, Volume= 2,215 cf  
 Routed to Pond 9-PS : BB9 - STONE  
 Tertiary = 2.51 cfs @ 12.15 hrs, Volume= 7,693 cf  
 Routed to Reach 1R : ISOLATOR ROW C

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.93' @ 12.15 hrs Surf.Area= 737 sf Storage= 430 cf

Plug-Flow detention time= 19.4 min calculated for 10,153 cf (100% of inflow)  
 Center-of-Mass det. time= 19.4 min ( 804.2 - 784.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	485 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	190	0	0
9.00	780	485	485

Device	Routing	Invert	Outlet Devices
#1	Primary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.00'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.00'
#3	Device 1	8.80'	<b>24inchDome Grate Capacity</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Tertiary	8.50'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.64 cfs @ 12.15 hrs HW=8.92' (Free Discharge)

↳ **1=Culvert** (Passes 0.64 cfs of 4.51 cfs potential flow)

↳ **3=24inchDome Grate Capacity** (Custom Controls 0.64 cfs)

**Secondary OutFlow** Max=0.05 cfs @ 12.15 hrs HW=8.92' (Free Discharge)

↳ **2=Exfiltration** ( Controls 0.05 cfs)

**Tertiary OutFlow** Max=2.49 cfs @ 12.15 hrs HW=8.92' (Free Discharge)

↳ **4=15inch-Dome Grate Capacity** (Custom Controls 2.49 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

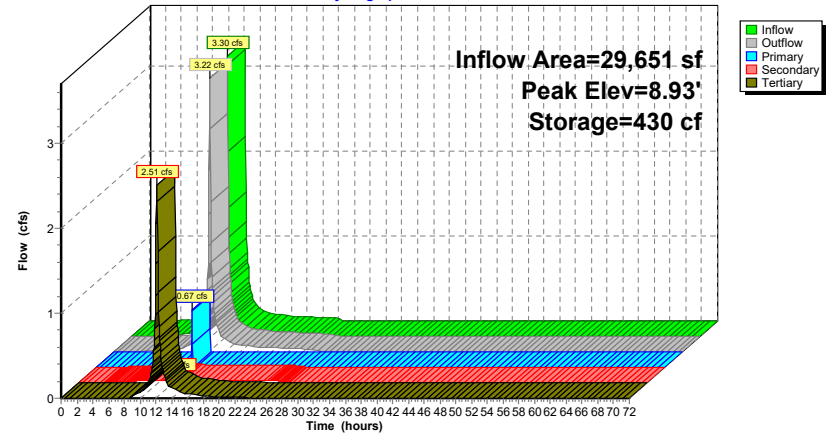
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 90

**Pond 9-P: BB9 - POND**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 91

**Summary for Pond 9-PS: BB9 - STONE**

Inflow = 0.05 cfs @ 12.15 hrs, Volume= 2,215 cf  
 Outflow = 0.05 cfs @ 12.17 hrs, Volume= 2,215 cf, Atten= 2%, Lag= 1.4 min  
 Primary = 0.05 cfs @ 12.17 hrs, Volume= 2,215 cf  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.15' @ 12.17 hrs Surf.Area= 190 sf Storage= 8 cf

Plug-Flow detention time= 3.7 min calculated for 2,213 cf (100% of inflow)  
 Center-of-Mass det. time= 3.7 min ( 944.6 - 940.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	114 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 380 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	190	0	0
8.00	190	380	380

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.05 cfs @ 12.17 hrs HW=6.15' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.05 cfs @ 1.30 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

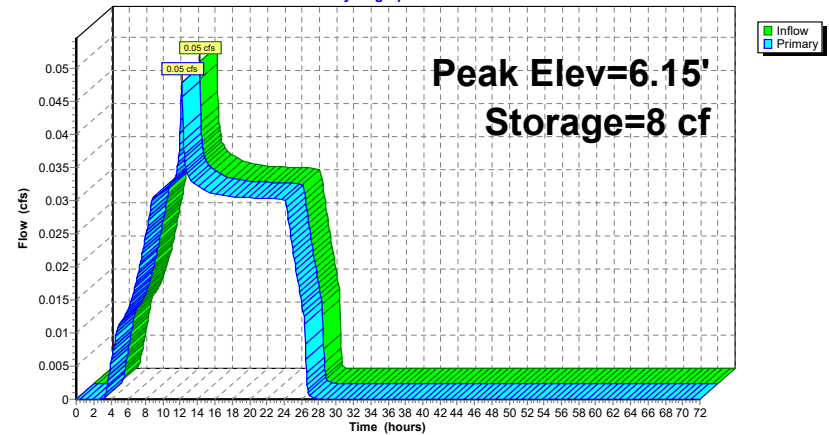
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 92

**Pond 9-PS: BB9 - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 93

**Summary for Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST**

[57] Hint: Peaked at 9.81' (Flood elevation advised)

Inflow Area = 19,582 sf, 58.17% Impervious, Inflow Depth = 3.69" for NOAA 10-yr event  
 Inflow = 2.03 cfs @ 12.13 hrs, Volume= 6,018 cf  
 Outflow = 2.03 cfs @ 12.13 hrs, Volume= 6,018 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 0.71 cfs @ 12.13 hrs, Volume= 4,632 cf  
 Routed to Pond INF-1 : INFILTRATION SYSTEM #1  
 Secondary = 1.32 cfs @ 12.13 hrs, Volume= 1,386 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.81' @ 12.13 hrs

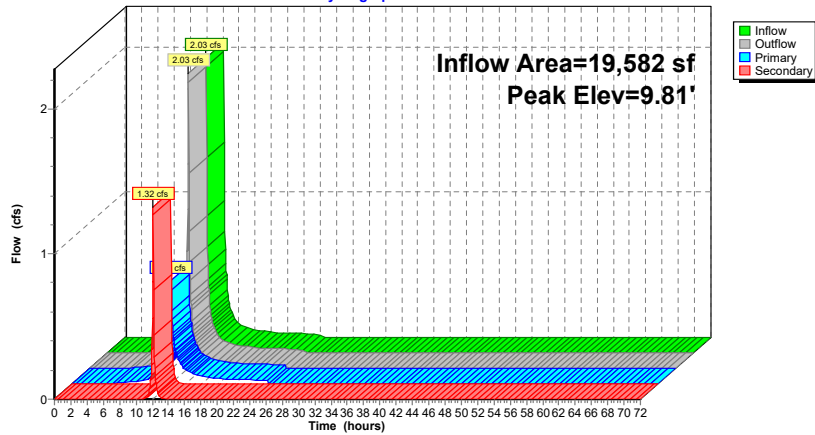
Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	6.0" Vert. WATER QUALITY STORM DIVERSION C= 0.600 Limited to weir flow at low heads
#2	Secondary	9.20'	12.0" Vert. LARGE STORM OVEFLOW C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.69 cfs @ 12.13 hrs HW=9.79' (Free Discharge)  
 ↳1=WATER QUALITY STORM DIVERSION(Orifice Controls 0.69 cfs @ 3.53 fps)

Secondary OutFlow Max=1.25 cfs @ 12.13 hrs HW=9.79' (Free Discharge)  
 ↳2=LARGE STORM OVEFLOW(Orifice Controls 1.25 cfs @ 2.61 fps)

**Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 94

**Summary for Pond DMH2: DIVERSION MANHOLE - HUDSON STREET**

[57] Hint: Peaked at 20.37' (Flood elevation advised)

Inflow Area = 143,309 sf, 49.69% Impervious, Inflow Depth = 3.49" for NOAA 10-yr event  
 Inflow = 14.20 cfs @ 12.13 hrs, Volume= 41,627 cf  
 Outflow = 14.20 cfs @ 12.13 hrs, Volume= 41,627 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 2.93 cfs @ 12.13 hrs, Volume= 23,559 cf  
 Routed to Pond INF-2 : INFILTRATION SYSTEM #2  
 Secondary = 11.27 cfs @ 12.13 hrs, Volume= 18,068 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 20.37' @ 12.13 hrs

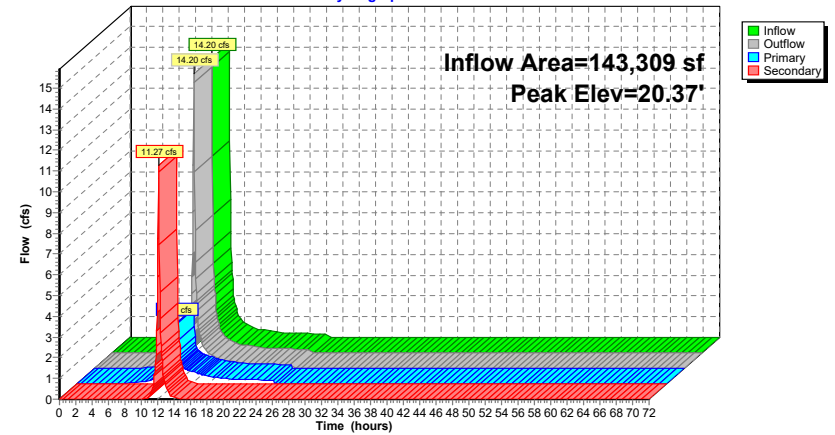
Device	Routing	Invert	Outlet Devices
#1	Primary	10.60'	6.0" Vert. WATER QUALITY STORM DIVERSION C= 0.600 Limited to weir flow at low heads
#2	Secondary	11.10'	12.0" Vert. LARGE STORM OVEFLOW C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.82 cfs @ 12.13 hrs HW=19.76' (Free Discharge)  
 ↳1=WATER QUALITY STORM DIVERSION(Orifice Controls 2.82 cfs @ 14.37 fps)

Secondary OutFlow Max=10.80 cfs @ 12.13 hrs HW=19.76' (Free Discharge)  
 ↳2=LARGE STORM OVERFLOW(Orifice Controls 10.80 cfs @ 13.76 fps)

**Pond DMH2: DIVERSION MANHOLE - HUDSON STREET**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 95

**Summary for Pond DMH3: DIVERSION MANHOLE - PORTLAND ST**

[57] Hint: Peaked at 11.73' (Flood elevation advised)

Inflow Area = 19,743 sf, 50.83% Impervious, Inflow Depth = 3.49" for NOAA 10-yr event  
 Inflow = 1.96 cfs @ 12.13 hrs, Volume= 5,735 cf  
 Outflow = 1.96 cfs @ 12.13 hrs, Volume= 5,735 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 1.06 cfs @ 12.13 hrs, Volume= 4,862 cf  
 Routed to Pond INF3 : INFILTRATION SYSTEM #1  
 Secondary = 0.90 cfs @ 12.13 hrs, Volume= 872 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

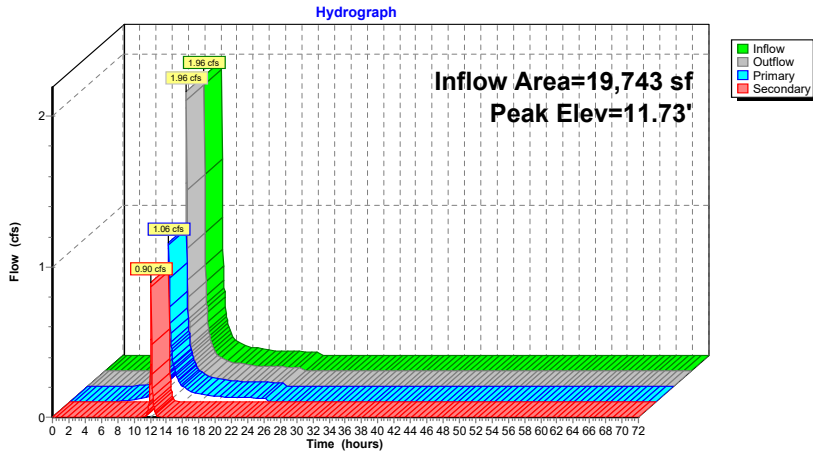
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.73' @ 12.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	11.00'	<b>8.0" Vert. WATER QUALITY DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	11.20'	<b>10.0" Vert. LARGE STORM OVERFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=1.03 cfs @ 12.13 hrs HW=11.71' (Free Discharge)  
 ↳1=WATER QUALITY DIVERSION(Orifice Controls 1.03 cfs @ 2.95 fps)

**Secondary OutFlow** Max=0.85 cfs @ 12.13 hrs HW=11.71' (Free Discharge)  
 ↳2=LARGE STORM OVERFLOW(Orifice Controls 0.85 cfs @ 2.43 fps)

**Pond DMH3: DIVERSION MANHOLE - PORTLAND ST**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 96

**Summary for Pond INF-1: INFILTRATION SYSTEM #1**

[81] Warning: Exceeded Pond DMH1 by 0.24' @ 14.90 hrs

Inflow Area = 19,582 sf, 58.17% Impervious, Inflow Depth = 2.84" for NOAA 10-yr event  
 Inflow = 0.71 cfs @ 12.13 hrs, Volume= 4,632 cf  
 Outflow = 0.23 cfs @ 12.69 hrs, Volume= 4,632 cf, Atten= 67%, Lag= 33.4 min  
 Discarded = 0.08 cfs @ 12.69 hrs, Volume= 4,195 cf  
 Primary = 0.15 cfs @ 12.69 hrs, Volume= 437 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.44' @ 12.69 hrs Surf.Area= 1,772 sf Storage= 1,658 cf

Plug-Flow detention time= 216.5 min calculated for 4,629 cf (100% of inflow)  
 Center-of-Mass det. time= 216.5 min ( 1,038.5 - 822.0 )

Volume	Invert	Avail.Storage	Storage Description
#1A	7.80'	1,091 cf	<b>21.50'W x 81.52'L x 2.33'H Field A</b> 4,090 cf Overall - 973 cf Embedded = 3,117 cf x 35.0% Voids
#2A	8.30'	973 cf	<b>ADS StormTech SC-310 +Cap</b> x 66 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 66 Chambers in 6 Rows
#3	7.80'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		2,201 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	7.80'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.80'
#2	Primary	8.10'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.10' / 8.00' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.40'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.08 cfs @ 12.69 hrs HW=9.44' (Free Discharge)  
 ↳1=Exfiltration ( Controls 0.08 cfs)

**Primary OutFlow** Max=0.13 cfs @ 12.69 hrs HW=9.44' (Free Discharge)  
 ↳2=Culvert ( Passes 0.13 cfs of 2.51 cfs potential flow)  
 ↳3=Sharp-Crested Rectangular Weir(Weir Controls 0.13 cfs @ 0.65 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 97

**Pond INF-1: INFILTRATION SYSTEM #1 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-310+Cap (ADS StormTech®SC-310 with cap length)**

Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf

Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap

34.0" Wide + 6.0" Spacing = 40.0" C-C Row Spacing

11 Chambers/Row x 7.12' Long +0.60' Cap Length x 2 = 79.52' Row Length +12.0" End Stone x 2 = 81.52' Base Length

6 Rows x 34.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 21.50' Base Width

6.0" Stone Base + 16.0" Chamber Height + 6.0" Stone Cover = 2.33' Field Height

66 Chambers x 14.7 cf = 973.0 cf Chamber Storage

4,089.6 cf Field - 973.0 cf Chambers = 3,116.6 cf Stone x 35.0% Voids = 1,090.8 cf Stone Storage

Chamber Storage + Stone Storage = 2,063.8 cf = 0.047 af

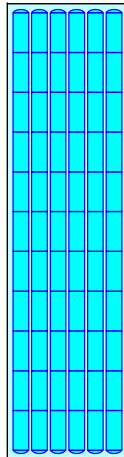
Overall Storage Efficiency = 50.5%

Overall System Size = 81.52' x 21.50' x 2.33'

66 Chambers

151.5 cy Field

115.4 cy Stone



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

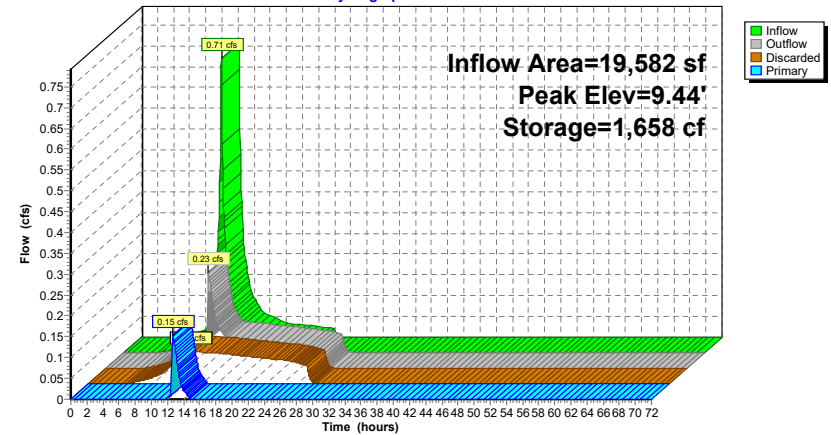
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 98

**Pond INF-1: INFILTRATION SYSTEM #1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 99

**Summary for Pond INF-2: INFILTRATION SYSTEM #2**

Inflow Area = 143,309 sf, 49.69% Impervious, Inflow Depth = 1.97" for NOAA 10-yr event  
 Inflow = 2.93 cfs @ 12.13 hrs, Volume= 23,559 cf  
 Outflow = 2.77 cfs @ 12.15 hrs, Volume= 23,559 cf, Atten= 5%, Lag= 1.5 min  
 Discarded = 0.12 cfs @ 12.15 hrs, Volume= 9,228 cf  
 Primary = 2.66 cfs @ 12.15 hrs, Volume= 14,332 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.80' @ 12.15 hrs Surf.Area= 2,268 sf Storage= 3,428 cf

Plug-Flow detention time= 144.9 min calculated for 23,559 cf (100% of inflow)  
 Center-of-Mass det. time= 144.8 min ( 1,009.1 - 864.3 )

Volume	Invert	Avail.Storage	Storage Description
#1A	7.50'	1,790 cf	<b>25.25'W x 89.06'L x 3.50'H Field A</b> 7,870 cf Overall - 2,756 cf Embedded = 5,114 cf x 35.0% Voids
#2A	8.00'	2,756 cf	<b>ADS_StormTech SC-740 +Cap</b> x 60 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 60 Chambers in 5 Rows
#3	7.50'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		4,684 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	7.50'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.50'
#2	Primary	8.00'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.50'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.12 cfs @ 12.15 hrs HW=9.80' (Free Discharge)  
 ↳1=Exfiltration ( Controls 0.12 cfs)

**Primary OutFlow** Max=2.62 cfs @ 12.15 hrs HW=9.80' (Free Discharge)  
 ↳2=Culvert (Passes 2.62 cfs of 3.09 cfs potential flow)  
 ↳3=Sharp-Crested Rectangular Weir(Weir Controls 2.62 cfs @ 1.78 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 100

**Pond INF-2: INFILTRATION SYSTEM #2 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-740 +Cap (ADS StormTech®SC-740 with cap length)**

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf

Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

12 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 87.06' Row Length +12.0" End Stone x 2 = 89.06'

Base Length

5 Rows x 51.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.25' Base Width

6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

60 Chambers x 45.9 cf = 2,756.4 cf Chamber Storage

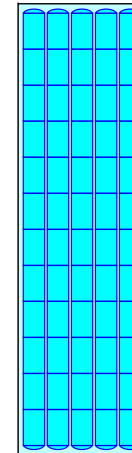
7,870.4 cf Field - 2,756.4 cf Chambers = 5,114.0 cf Stone x 35.0% Voids = 1,789.9 cf Stone Storage

Chamber Storage + Stone Storage = 4,546.3 cf = 0.104 af

Overall Storage Efficiency = 57.8%

Overall System Size = 89.06' x 25.25' x 3.50'

60 Chambers  
 291.5 cy Field  
 189.4 cy Stone



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

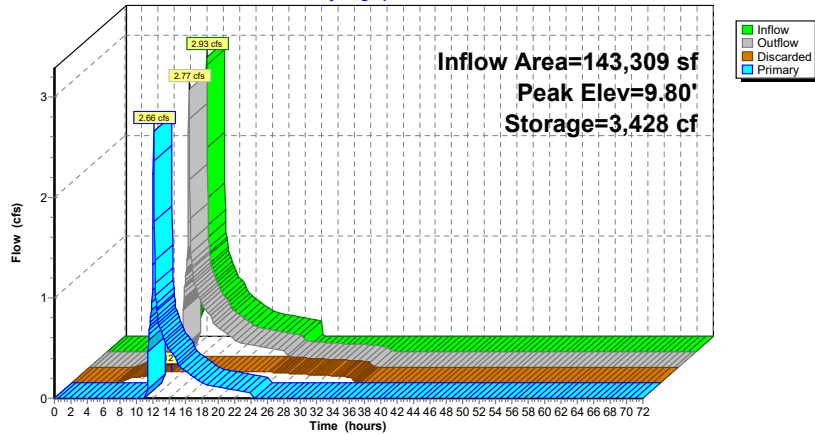
NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Printed 4/6/2022

Page 101

**Pond INF-2: INFILTRATION SYSTEM #2**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Printed 4/6/2022

Page 102

**Summary for Pond INF3: INFILTRATION SYSTEM #1**

Inflow Area = 19,743 sf, 50.83% Impervious, Inflow Depth = 2.96" for NOAA 10-yr event  
 Inflow = 1.06 cfs @ 12.13 hrs, Volume= 4,862 cf  
 Outflow = 1.05 cfs @ 12.14 hrs, Volume= 4,862 cf, Atten= 1%, Lag= 0.9 min  
 Discarded = 0.05 cfs @ 12.14 hrs, Volume= 2,902 cf  
 Primary = 1.00 cfs @ 12.14 hrs, Volume= 1,960 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.66' @ 12.14 hrs Surf.Area= 1,113 sf Storage= 991 cf

Plug-Flow detention time= 146.9 min calculated for 4,862 cf (100% of inflow)  
 Center-of-Mass det. time= 146.7 min ( 968.0 - 821.3 )

Volume	Invert	Avail.Storage	Storage Description
#1A	8.10'	686 cf	<b>18.17'W x 60.16'L x 2.33'H Field A</b> 2,550 cf Overall - 590 of Embedded = 1,960 cf x 35.0% Voids
#2A	8.60'	590 cf	<b>ADS StormTech SC-310 +Cap x 40 Inside #1</b> Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 40 Chambers in 5 Rows
#3	8.10'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		1,413 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	8.10'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.10'
#2	Primary	8.40'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.40' / 8.30' S= 0.0100'/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.50'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.05 cfs @ 12.14 hrs HW=9.65' (Free Discharge)  
 ↳1=Exfiltration ( Controls 0.05 cfs)

**Primary OutFlow** Max=0.98 cfs @ 12.14 hrs HW=9.65' (Free Discharge)  
 ↳2=Culvert (Passes 0.98 cfs of 2.33 cfs potential flow)  
 ↳3=Sharp-Crested Rectangular Weir(Weir Controls 0.98 cfs @ 1.28 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 103

**Pond INF3: INFILTRATION SYSTEM #1 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-310+Cap (ADS StormTech®SC-310 with cap length)**

Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf

Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap

34.0" Wide + 6.0" Spacing = 40.0" C-C Row Spacing

8 Chambers/Row x 7.12' Long +0.60' Cap Length x 2 = 58.16' Row Length +12.0" End Stone x 2 = 60.16' Base Length

5 Rows x 34.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 18.17' Base Width

6.0" Stone Base + 16.0" Chamber Height + 6.0" Stone Cover = 2.33' Field Height

40 Chambers x 14.7 cf = 589.7 cf Chamber Storage

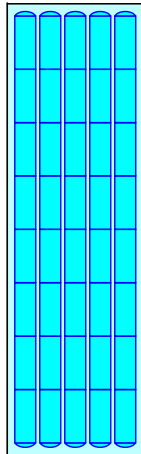
2,550.1 cf Field - 589.7 cf Chambers = 1,960.4 cf Stone x 35.0% Voids = 686.2 cf Stone Storage

Chamber Storage + Stone Storage = 1,275.8 cf = 0.029 af

Overall Storage Efficiency = 50.0%

Overall System Size = 60.16' x 18.17' x 2.33'

40 Chambers  
94.4 cy Field  
72.6 cy Stone



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 10-yr Rainfall=5.02"

Prepared by {enter your company name here}

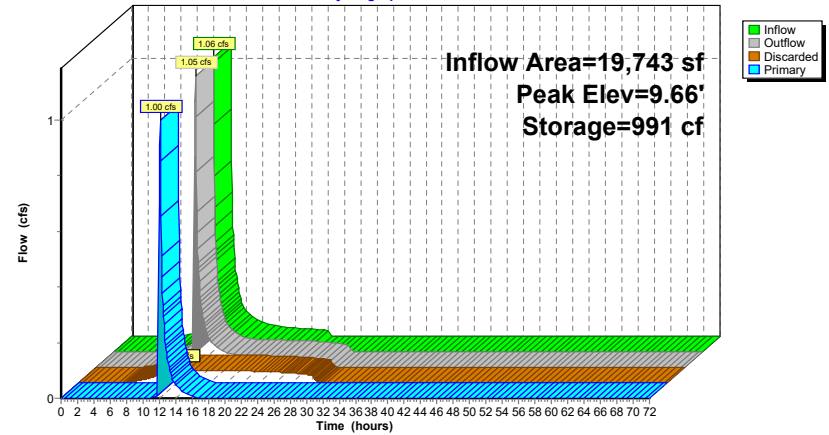
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 104

**Pond INF3: INFILTRATION SYSTEM #1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 105

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: BB-1</b>	Runoff Area=38,826 sf 51.66% Impervious Runoff Depth=5.93" Tc=6.0 min CN=86 Runoff=6.35 cfs 19,201 cf
<b>Subcatchment2a: BB-2a</b>	Runoff Area=3,116 sf 92.62% Impervious Runoff Depth=7.11" Tc=6.0 min CN=96 Runoff=0.56 cfs 1,847 cf
<b>Subcatchment2b: BB-2b</b>	Runoff Area=21,490 sf 80.50% Impervious Runoff Depth=6.76" Tc=6.0 min CN=93 Runoff=3.78 cfs 12,099 cf
<b>Subcatchment3A: BB-3A</b>	Runoff Area=10,987 sf 58.16% Impervious Runoff Depth=6.17" Tc=6.0 min CN=88 Runoff=1.84 cfs 5,647 cf
<b>Subcatchment3B: BB-3B</b>	Runoff Area=4,545 sf 77.34% Impervious Runoff Depth=6.76" Tc=6.0 min CN=93 Runoff=0.80 cfs 2,559 cf
<b>Subcatchment4A: BB-4A</b>	Runoff Area=4,843 sf 86.37% Impervious Runoff Depth=6.99" Tc=6.0 min CN=95 Runoff=0.86 cfs 2,822 cf
<b>Subcatchment4B: BB-4B</b>	Runoff Area=3,048 sf 86.09% Impervious Runoff Depth=6.99" Tc=6.0 min CN=95 Runoff=0.54 cfs 1,776 cf
<b>Subcatchment5A: BB-5A</b>	Runoff Area=3,072 sf 73.44% Impervious Runoff Depth=6.64" Tc=6.0 min CN=92 Runoff=0.54 cfs 1,699 cf
<b>Subcatchment5B: BB-5B</b>	Runoff Area=34,755 sf 71.39% Impervious Runoff Depth=6.52" Tc=6.0 min CN=91 Runoff=6.01 cfs 18,883 cf
<b>Subcatchment6A: BB-6A</b>	Runoff Area=15,148 sf 46.97% Impervious Runoff Depth=5.82" Tc=6.0 min CN=85 Runoff=2.44 cfs 7,345 cf
<b>Subcatchment6B: BB-6B</b>	Runoff Area=6,495 sf 77.45% Impervious Runoff Depth=6.76" Tc=6.0 min CN=93 Runoff=1.14 cfs 3,657 cf
<b>Subcatchment7A: BB-7A</b>	Runoff Area=3,165 sf 87.74% Impervious Runoff Depth=6.99" Tc=6.0 min CN=95 Runoff=0.56 cfs 1,844 cf
<b>Subcatchment7B: BB-7B</b>	Runoff Area=4,942 sf 88.73% Impervious Runoff Depth=6.99" Tc=6.0 min CN=95 Runoff=0.88 cfs 2,880 cf
<b>Subcatchment8A: BB-8A</b>	Runoff Area=3,978 sf 79.99% Impervious Runoff Depth=6.76" Tc=6.0 min CN=93 Runoff=0.70 cfs 2,240 cf
<b>Subcatchment8B: BB-8B</b>	Runoff Area=5,598 sf 87.78% Impervious Runoff Depth=6.99" Tc=6.0 min CN=95 Runoff=1.00 cfs 3,262 cf
<b>Subcatchment9: BB-9</b>	Runoff Area=29,651 sf 74.77% Impervious Runoff Depth=6.64" Tc=6.0 min CN=92 Runoff=5.18 cfs 16,401 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 106

<b>SubcatchmentCB-1: New CB South</b>	Runoff Area=19,582 sf 58.17% Impervious Runoff Depth=6.17" Flow Length=512' Tc=6.0 min CN=88 Runoff=3.28 cfs 10,065 cf
<b>SubcatchmentCB-5: PORTLANDST</b>	Runoff Area=19,743 sf 50.83% Impervious Runoff Depth=5.93" Flow Length=574' Tc=6.0 min CN=86 Runoff=3.23 cfs 9,764 cf
<b>SubcatchmentCB3: NEW CB SOUTH-</b>	Runoff Area=25,183 sf 51.84% Impervious Runoff Depth=5.93" Flow Length=635' Tc=6.0 min CN=86 Runoff=4.12 cfs 12,454 cf
<b>SubcatchmentCB4: NEW CB NOTH -</b>	Runoff Area=118,126 sf 49.24% Impervious Runoff Depth=5.93" Flow Length=822' Tc=6.0 min CN=86 Runoff=19.32 cfs 58,419 cf
<b>Reach 1R: ISOLATORROW C</b>	Inflow=2.93 cfs 12,777 cf Outflow=2.93 cfs 12,777 cf
<b>Reach 6R: ISOLATORROW 2</b>	Inflow=2.52 cfs 12,054 cf Outflow=2.52 cfs 12,054 cf
<b>Reach 15R: ISOLATORROW 1</b>	Inflow=5.80 cfs 26,550 cf Outflow=5.80 cfs 26,550 cf
<b>Reach B: PARKING LOT B OVERFLOW</b>	Inflow=31.84 cfs 91,717 cf Outflow=31.84 cfs 91,717 cf
<b>Reach BMP4_O: BMP-4 OVERFLOW</b>	Inflow=0.57 cfs 3,249 cf Outflow=0.57 cfs 3,249 cf
<b>Reach BMP6_O: BMP-6 OVERFLOW</b>	Inflow=3.58 cfs 9,708 cf Outflow=3.58 cfs 9,708 cf
<b>Reach BMP7_O: BMP-7 OVERFLOW</b>	Inflow=1.42 cfs 4,724 cf Outflow=1.42 cfs 4,724 cf
<b>Reach BMP9_O: BMP-9 OVERFLOW</b>	Inflow=5.28 cfs 16,401 cf Outflow=5.28 cfs 16,401 cf
<b>Reach BMP_3: BMP-3_OVERFLOW</b>	Inflow=3.33 cfs 7,035 cf Outflow=3.33 cfs 7,035 cf
<b>Reach DP-1: French Rodney Blvd 14" Outfall</b>	Inflow=13.97 cfs 35,096 cf Outflow=13.97 cfs 35,096 cf
<b>Reach DP-2: NORTHERN OUTFALL</b>	Inflow=46.92 cfs 134,321 cf Outflow=46.92 cfs 134,321 cf
<b>Reach H ST: HUDSON STREET DRAINAGE</b>	Inflow=38.10 cfs 111,381 cf Outflow=38.10 cfs 111,381 cf
<b>Reach P ST: PORTLAND STREET DRAINAGE</b>	Inflow=8.83 cfs 22,940 cf Outflow=8.83 cfs 22,940 cf



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 107

**Pond 1-P: BB 1** Peak Elev=10.27' Storage=1,114 cf Inflow=6.35 cfs 19,201 cf  
Discarded=0.05 cfs 2,860 cf Primary=4.04 cfs 3,321 cf Secondary=2.54 cfs 13,021 cf Outflow=6.64 cfs 19,201 cf

**Pond 2a-P: BB 2a** Peak Elev=8.22' Storage=91 cf Inflow=0.56 cfs 1,847 cf  
Primary=0.54 cfs 1,811 cf Secondary=0.00 cfs 0 cf Outflow=0.54 cfs 1,811 cf

**Pond 2b-P: BB 2b** Peak Elev=8.61' Storage=338 cf Inflow=3.78 cfs 12,099 cf  
Primary=2.72 cfs 11,718 cf Secondary=1.02 cfs 344 cf Outflow=3.74 cfs 12,063 cf

**Pond 3A-P: BB 3A** Peak Elev=11.14' Storage=511 cf Inflow=1.84 cfs 5,647 cf  
Discarded=0.03 cfs 1,537 cf Primary=1.73 cfs 4,110 cf Outflow=1.75 cfs 5,647 cf

**Pond 3B-P: BB 3B** Peak Elev=12.91' Storage=263 cf Inflow=0.80 cfs 2,559 cf  
Discarded=0.02 cfs 983 cf Primary=0.80 cfs 1,576 cf Outflow=0.82 cfs 2,559 cf

**Pond 4A-P: BB 4A - POND** Peak Elev=10.09' Storage=253 cf Inflow=0.86 cfs 2,822 cf  
Primary=0.80 cfs 1,349 cf Secondary=0.04 cfs 1,473 cf Outflow=0.84 cfs 2,822 cf

**Pond 4A-S: BB4A-Stone** Peak Elev=6.13' Storage=9 cf Inflow=0.04 cfs 1,473 cf  
Outflow=0.04 cfs 1,473 cf

**Pond 4B-P: BB 4B - POND** Peak Elev=11.06' Storage=142 cf Inflow=0.54 cfs 1,776 cf  
Primary=0.52 cfs 858 cf Secondary=0.02 cfs 919 cf Outflow=0.54 cfs 1,776 cf

**Pond 4B-S: BB 4A-Stone** Peak Elev=6.10' Storage=4 cf Inflow=0.02 cfs 919 cf  
Outflow=0.02 cfs 919 cf

**Pond 5A-P: BB 5A - POND** Peak Elev=9.58' Storage=472 cf Inflow=0.54 cfs 1,699 cf  
Primary=0.32 cfs 278 cf Secondary=0.05 cfs 1,422 cf Outflow=0.37 cfs 1,699 cf

**Pond 5A-PS: BB 5A-Stone** Peak Elev=6.15' Storage=21 cf Inflow=0.05 cfs 1,422 cf  
Outflow=0.05 cfs 1,422 cf

**Pond 5B-P: BB 5B - POND** Peak Elev=9.13' Storage=790 cf Inflow=6.01 cfs 18,883 cf  
Primary=3.29 cfs 2,579 cf Secondary=0.09 cfs 4,250 cf Tertiary=2.52 cfs 12,054 cf Outflow=5.90 cfs 18,883 cf

**Pond 5B-PS: BB 5B-Stone** Peak Elev=6.00' Storage=1 cf Inflow=0.09 cfs 4,250 cf  
Outflow=0.09 cfs 4,250 cf

**Pond 6A-P: BB 6A - POND** Peak Elev=11.07' Storage=469 cf Inflow=2.44 cfs 7,345 cf  
Primary=2.38 cfs 4,859 cf Secondary=0.05 cfs 2,486 cf Outflow=2.43 cfs 7,345 cf

**Pond 6A-PS: BB 6A - STONE** Peak Elev=6.15' Storage=13 cf Inflow=0.05 cfs 2,486 cf  
Outflow=0.05 cfs 2,486 cf

**Pond 6B-P: BB 6B** Peak Elev=12.13' Storage=394 cf Inflow=1.14 cfs 3,657 cf  
Discarded=0.02 cfs 1,294 cf Primary=1.15 cfs 2,363 cf Outflow=1.17 cfs 3,657 cf

**Pond 7A-P: BB 7A PONDING** Peak Elev=10.01' Storage=159 cf Inflow=0.56 cfs 1,844 cf  
Primary=0.54 cfs 916 cf Secondary=0.02 cfs 928 cf Outflow=0.56 cfs 1,844 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 108

**Pond 7A-S: BB 7A - STONE** Peak Elev=5.19' Storage=4 cf Inflow=0.02 cfs 928 cf  
Outflow=0.02 cfs 928 cf

**Pond 7B-P: BB 7B PONDING** Peak Elev=10.75' Storage=295 cf Inflow=0.88 cfs 2,880 cf  
Primary=0.82 cfs 1,369 cf Secondary=0.04 cfs 1,511 cf Outflow=0.86 cfs 2,880 cf

**Pond 7B-S: BB 7B - STONE** Peak Elev=5.22' Storage=6 cf Inflow=0.04 cfs 1,511 cf  
Outflow=0.04 cfs 1,511 cf

**Pond 8a-P: BB 8A PONDING** Peak Elev=9.12' Storage=308 cf Inflow=0.70 cfs 2,240 cf  
Primary=0.64 cfs 855 cf Secondary=0.04 cfs 1,385 cf Outflow=0.68 cfs 2,240 cf

**Pond 8a-s: BB 8A - STONE** Peak Elev=4.53' Storage=12 cf Inflow=0.04 cfs 1,385 cf  
Outflow=0.04 cfs 1,385 cf

**Pond 8B-P: BB 8B-PONDING** Peak Elev=9.75' Storage=274 cf Inflow=1.00 cfs 3,262 cf  
Primary=0.94 cfs 1,582 cf Secondary=0.04 cfs 1,680 cf Outflow=0.98 cfs 3,262 cf

**Pond 8B-S: BB 8B-Stone** Peak Elev=4.53' Storage=12 cf Inflow=0.04 cfs 1,680 cf  
Outflow=0.04 cfs 1,680 cf

**Pond 9-P: BB9 - POND** Peak Elev=9.09' Storage=485 cf Inflow=5.18 cfs 16,401 cf  
Primary=2.30 cfs 1,149 cf Secondary=0.05 cfs 2,475 cf Tertiary=2.93 cfs 12,777 cf Outflow=5.28 cfs 16,401 cf

**Pond 9-PS: BB9 - STONE** Peak Elev=6.15' Storage=9 cf Inflow=0.05 cfs 2,475 cf  
Outflow=0.05 cfs 2,475 cf

**Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST** Peak Elev=10.10' Inflow=3.28 cfs 10,065 cf  
Primary=0.87 cfs 7,068 cf Secondary=2.41 cfs 2,996 cf Outflow=3.28 cfs 10,065 cf

**Pond DMH2: DIVERSION MANHOLE - HUDSON STREET** Peak Elev=35.73' Inflow=23.44 cfs 70,873 cf  
Primary=4.74 cfs 35,221 cf Secondary=18.70 cfs 35,651 cf Outflow=23.44 cfs 70,873 cf

**Pond DMH3: DIVERSION MANHOLE - PORTLAND ST** Peak Elev=12.07' Inflow=3.23 cfs 9,764 cf  
Primary=1.44 cfs 7,738 cf Secondary=1.78 cfs 2,026 cf Outflow=3.23 cfs 9,764 cf

**Pond INF-1: INFILTRATIONSYSTEM#1** Peak Elev=9.53' Storage=1,727 cf Inflow=0.87 cfs 7,068 cf  
Discarded=0.08 cfs 5,184 cf Primary=0.82 cfs 1,885 cf Outflow=0.89 cfs 7,068 cf

**Pond INF-2: INFILTRATIONSYSTEM#2** Peak Elev=10.13' Storage=3,861 cf Inflow=4.74 cfs 35,221 cf  
Discarded=0.12 cfs 10,022 cf Primary=3.44 cfs 25,199 cf Outflow=3.57 cfs 35,221 cf

**Pond INF3: INFILTRATIONSYSTEM#1** Peak Elev=9.69' Storage=1,012 cf Inflow=1.44 cfs 7,738 cf  
Discarded=0.05 cfs 3,496 cf Primary=1.39 cfs 4,242 cf Outflow=1.43 cfs 7,738 cf

**Total Runoff Area = 376,293 sf Runoff Volume = 194,864 cf Average Runoff Depth = 6.21"**  
**39.88% Pervious = 150,053 sf 60.12% Impervious = 226,240 sf**

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 109

**Summary for Subcatchment 1: BB-1**

Runoff = 6.35 cfs @ 12.13 hrs, Volume= 19,201 cf, Depth= 5.93"  
Routed to Pond 1-P : BB 1

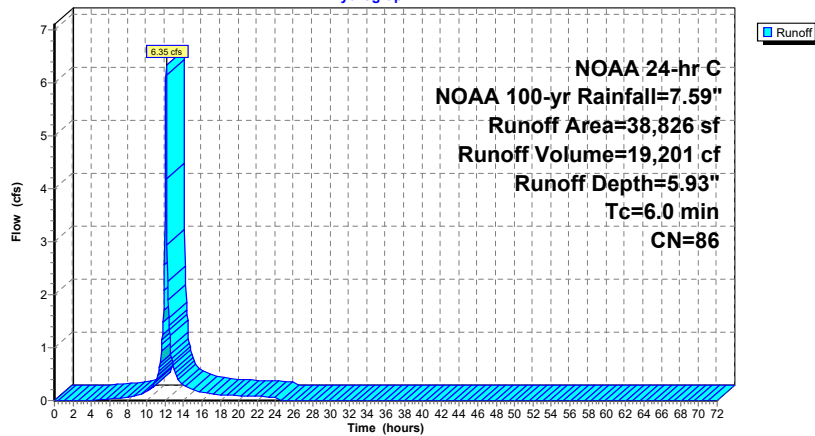
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
27,309	83	1/4 acre lots, 38% imp, HSG C
1,838	74	>75% Grass cover, Good, HSG C
9,679	98	Paved parking, HSG C
38,826	86	Weighted Average
18,770		48.34% Pervious Area
20,056		51.66% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 1: BB-1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 110

**Summary for Subcatchment 2a: BB-2a**

Runoff = 0.56 cfs @ 12.13 hrs, Volume= 1,847 cf, Depth= 7.11"  
Routed to Pond 2a-P : BB 2a

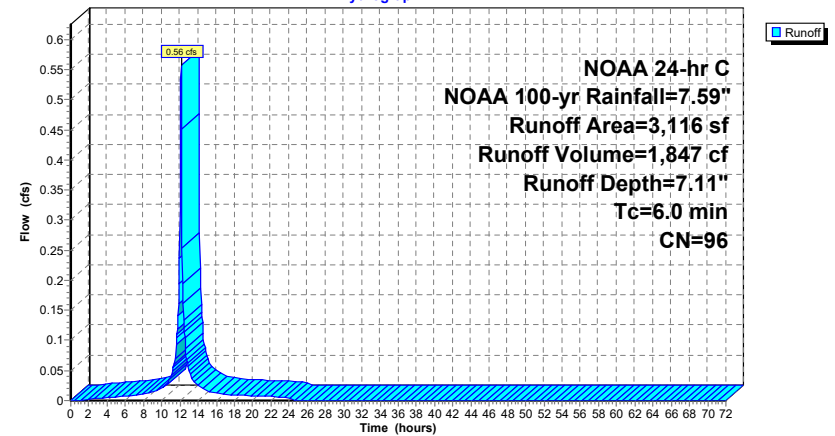
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
230	74	>75% Grass cover, Good, HSG C
2,886	98	Paved parking, HSG C
3,116	96	Weighted Average
230		7.38% Pervious Area
2,886		92.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2a: BB-2a**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 111

**Summary for Subcatchment 2b: BB-2b**

Runoff = 3.78 cfs @ 12.13 hrs, Volume= 12,099 cf, Depth= 6.76"  
Routed to Pond 2b-P : BB 2b

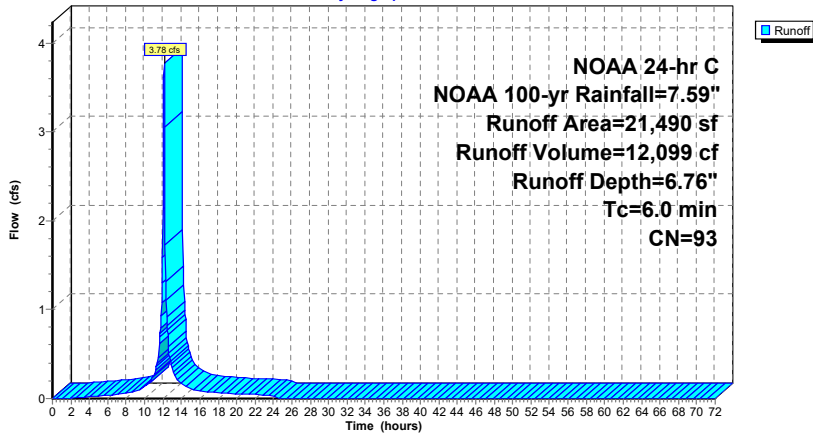
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
3,097	83	1/4 acre lots, 38% imp, HSG C
2,270	74	>75% Grass cover, Good, HSG C
16,123	98	Paved parking, HSG C
21,490	93	Weighted Average
4,190		19.50% Pervious Area
17,300		80.50% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2b: BB-2b**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 112

**Summary for Subcatchment 3A: BB-3A**

Runoff = 1.84 cfs @ 12.13 hrs, Volume= 5,647 cf, Depth= 6.17"  
Routed to Pond 3A-P : BB 3A

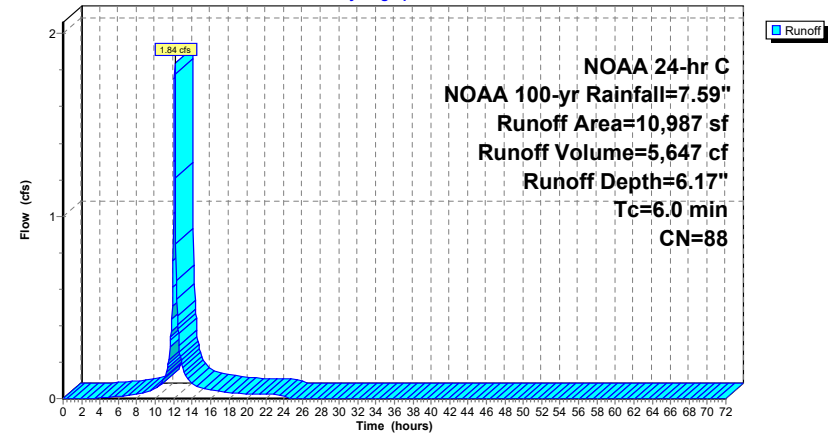
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
5,791	83	1/4 acre lots, 38% imp, HSG C
1,007	74	>75% Grass cover, Good, HSG C
4,189	98	Paved parking, HSG C
10,987	88	Weighted Average
4,597		41.84% Pervious Area
6,390		58.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3A: BB-3A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 113

**Summary for Subcatchment 3B: BB-3B**

Runoff = 0.80 cfs @ 12.13 hrs, Volume= 2,559 cf, Depth= 6.76"  
Routed to Pond 3B-P : BB 3B

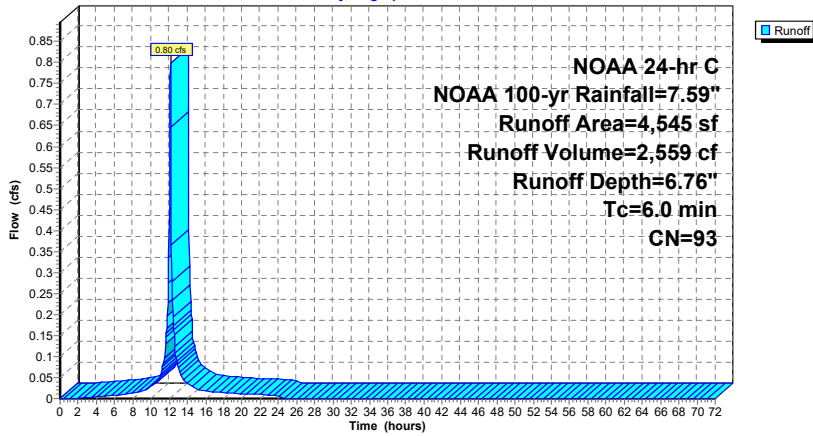
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
* 1,030	74	>75% Grass cover, Good, HSG C
3,515	98	Paved parking, HSG C
4,545	93	Weighted Average
1,030		22.66% Pervious Area
3,515		77.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3B: BB-3B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 114

**Summary for Subcatchment 4A: BB-4A**

Runoff = 0.86 cfs @ 12.13 hrs, Volume= 2,822 cf, Depth= 6.99"  
Routed to Pond 4A-P : BB 4A - POND

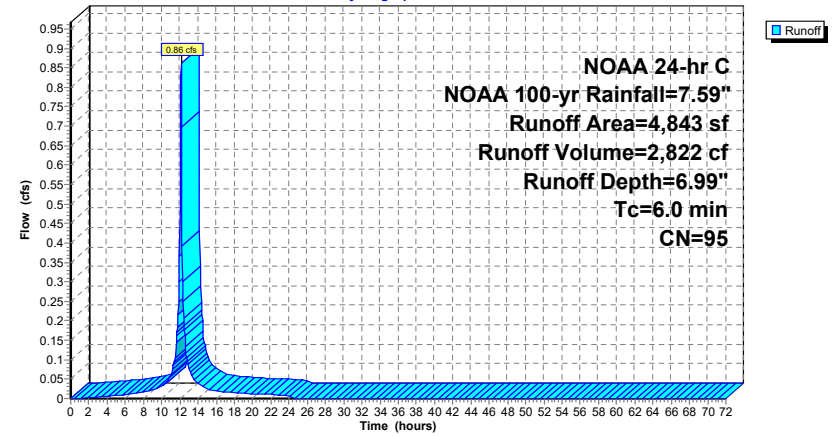
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
660	74	>75% Grass cover, Good, HSG C
4,183	98	Paved parking, HSG C
4,843	95	Weighted Average
660		13.63% Pervious Area
4,183		86.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 4A: BB-4A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 115

**Summary for Subcatchment 4B: BB-4B**

Runoff = 0.54 cfs @ 12.13 hrs, Volume= 1,776 cf, Depth= 6.99"  
Routed to Pond 4B-P : BB 4B - POND

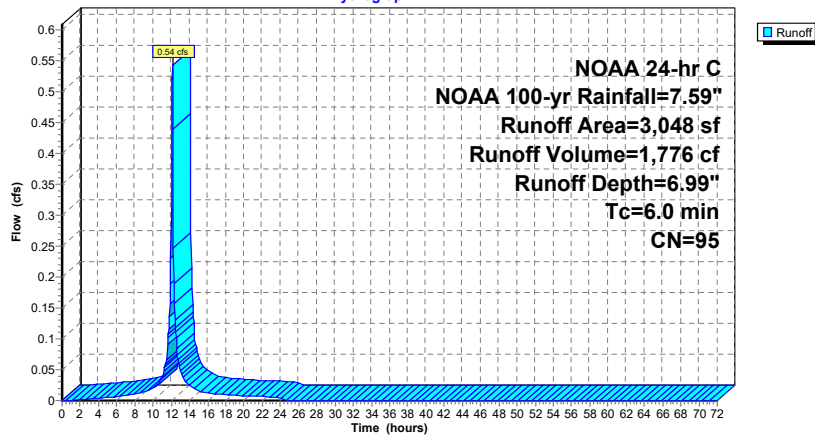
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
424	74	>75% Grass cover, Good, HSG C
2,624	98	Paved parking, HSG C
3,048	95	Weighted Average
424		13.91% Pervious Area
2,624		86.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 4B: BB-4B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 116

**Summary for Subcatchment 5A: BB-5A**

Runoff = 0.54 cfs @ 12.13 hrs, Volume= 1,699 cf, Depth= 6.64"  
Routed to Pond 5A-P : BB 5A - POND

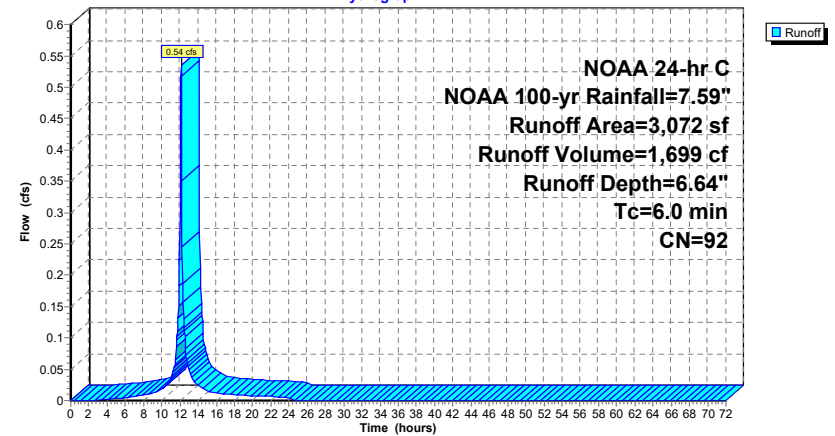
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
816	74	>75% Grass cover, Good, HSG C
2,256	98	Paved parking, HSG C
3,072	92	Weighted Average
816		26.56% Pervious Area
2,256		73.44% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5A: BB-5A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 117

**Summary for Subcatchment 5B: BB-5B**

Runoff = 6.01 cfs @ 12.13 hrs, Volume= 18,883 cf, Depth= 6.52"  
 Routed to Pond 5B-P : BB 5B - POND

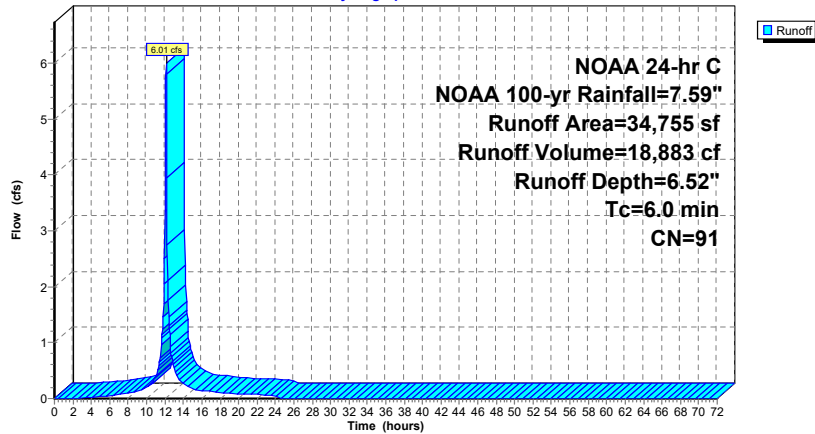
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
12,062	83	1/4 acre lots, 38% imp, HSG C
2,464	74	>75% Grass cover, Good, HSG C
20,229	98	Paved parking, HSG C
34,755	91	Weighted Average
9,942		28.61% Pervious Area
24,813		71.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5B: BB-5B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 118

**Summary for Subcatchment 6A: BB-6A**

Runoff = 2.44 cfs @ 12.13 hrs, Volume= 7,345 cf, Depth= 5.82"  
 Routed to Pond 6A-P : BB 6A - POND

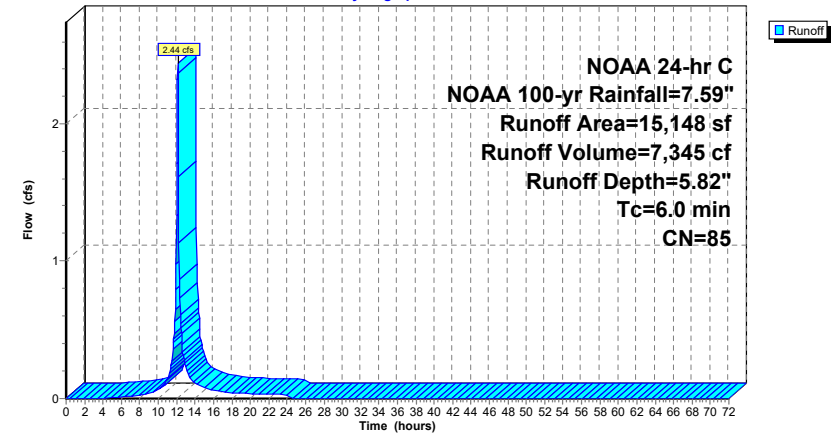
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
11,763	83	1/4 acre lots, 38% imp, HSG C
740	74	>75% Grass cover, Good, HSG C
2,645	98	Paved parking, HSG C
15,148	85	Weighted Average
8,033		53.03% Pervious Area
7,115		46.97% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6A: BB-6A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 119

**Summary for Subcatchment 6B: BB-6B**

Runoff = 1.14 cfs @ 12.13 hrs, Volume= 3,657 cf, Depth= 6.76"  
Routed to Pond 6B-P : BB 6B

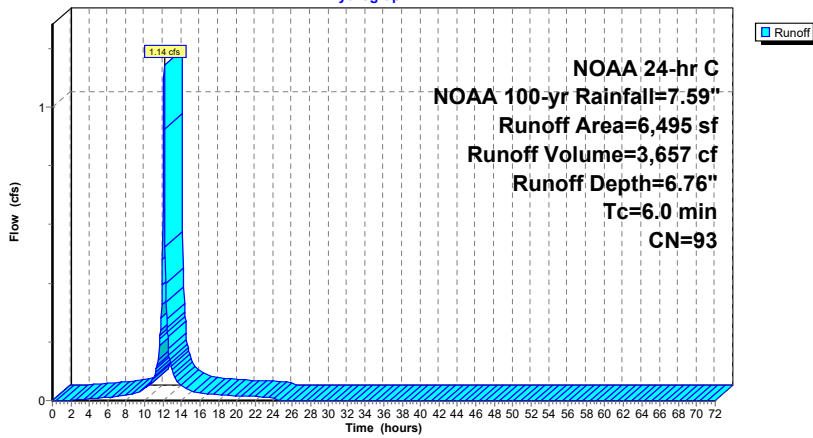
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
1,259	83	1/4 acre lots, 38% imp, HSG C
684	74	>75% Grass cover, Good, HSG C
4,552	98	Paved parking, HSG C
6,495	93	Weighted Average
1,465		22.55% Pervious Area
5,030		77.45% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6B: BB-6B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 120

**Summary for Subcatchment 7A: BB-7A**

Runoff = 0.56 cfs @ 12.13 hrs, Volume= 1,844 cf, Depth= 6.99"  
Routed to Pond 7A-P : BB 7A PONDING

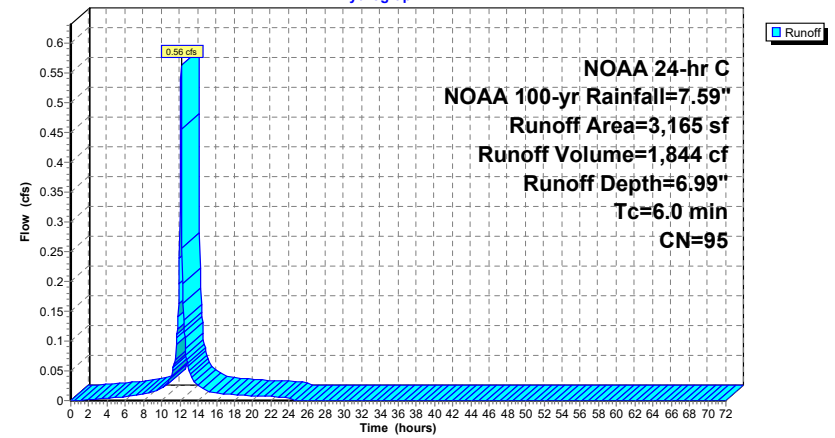
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
388	74	>75% Grass cover, Good, HSG C
2,777	98	Paved parking, HSG C
3,165	95	Weighted Average
388		12.26% Pervious Area
2,777		87.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 7A: BB-7A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 121

**Summary for Subcatchment 7B: BB-7B**

Runoff = 0.88 cfs @ 12.13 hrs, Volume= 2,880 cf, Depth= 6.99"  
 Routed to Pond 7B-P : BB 7B PONDING

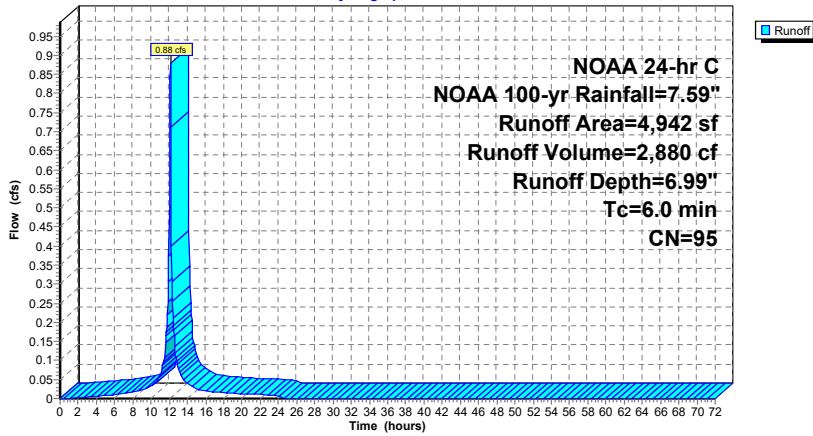
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
557	74	>75% Grass cover, Good, HSG C
4,385	98	Paved parking, HSG C
4,942	95	Weighted Average
557		11.27% Pervious Area
4,385		88.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 7B: BB-7B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 122

**Summary for Subcatchment 8A: BB-8A**

Runoff = 0.70 cfs @ 12.13 hrs, Volume= 2,240 cf, Depth= 6.76"  
 Routed to Pond 8a-P : BB 8A PONDING

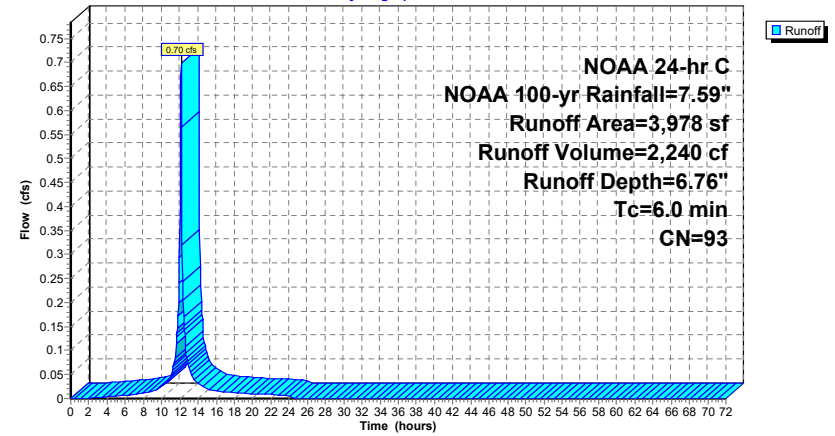
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
796	74	>75% Grass cover, Good, HSG C
3,182	98	Paved parking, HSG C
3,978	93	Weighted Average
796		20.01% Pervious Area
3,182		79.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8A: BB-8A**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 123

**Summary for Subcatchment 8B: BB-8B**

Runoff = 1.00 cfs @ 12.13 hrs, Volume= 3,262 cf, Depth= 6.99"  
 Routed to Pond 8B-P : BB 8B-PONDING

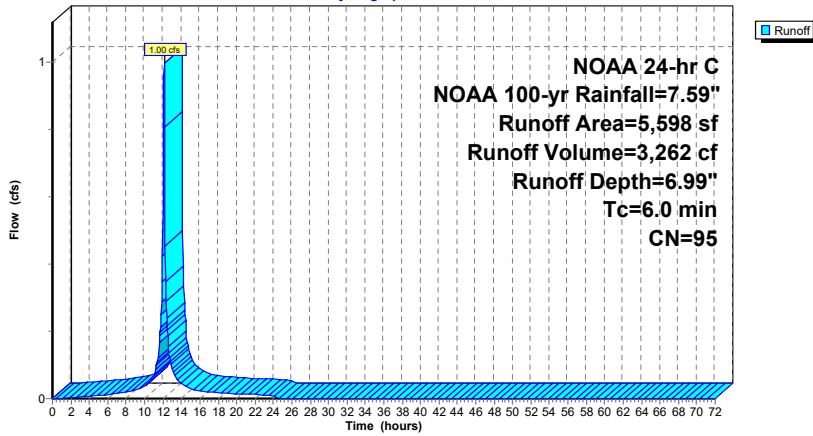
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
684	74	>75% Grass cover, Good, HSG C
4,914	98	Paved parking, HSG C
5,598	95	Weighted Average
684		12.22% Pervious Area
4,914		87.78% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8B: BB-8B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 124

**Summary for Subcatchment 9: BB-9**

Runoff = 5.18 cfs @ 12.13 hrs, Volume= 16,401 cf, Depth= 6.64"  
 Routed to Pond 9-P : BB9 - POND

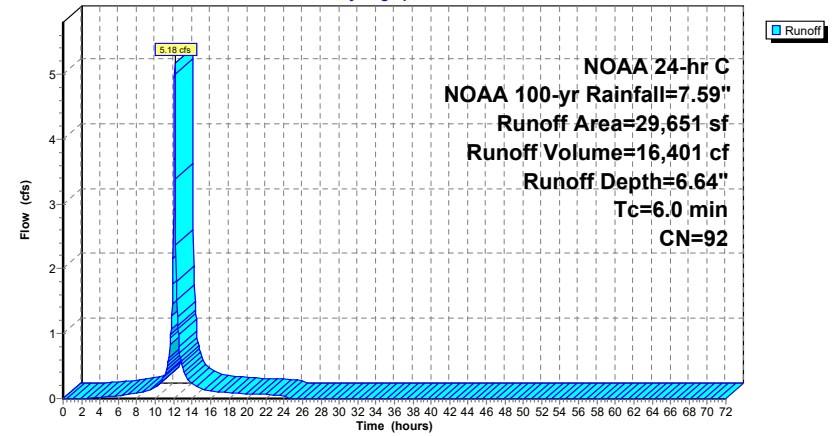
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Area (sf)	CN	Description
8,550	83	1/4 acre lots, 38% imp, HSG C
2,179	74	>75% Grass cover, Good, HSG C
18,922	98	Paved parking, HSG C
29,651	92	Weighted Average
7,480		25.23% Pervious Area
22,171		74.77% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 9: BB-9**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 125

**Summary for Subcatchment CB-1: New CB South (Seymour Street)**

Runoff = 3.28 cfs @ 12.13 hrs, Volume= 10,065 cf, Depth= 6.17"  
 Routed to Pond DMH1 : DIVERSION MANHOLE - SEYMOUR ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

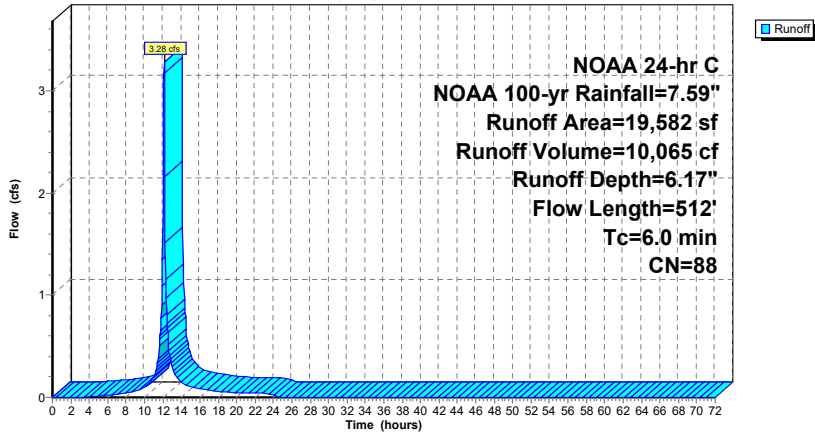
Area (sf)	CN	Description
13,211	83	1/4 acre lots, 38% imp, HSG C
* 6,371	98	Roadway
19,582	88	Weighted Average
8,191		41.83% Pervious Area
11,391		58.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	50	0.0300	1.45		<b>Sheet Flow, A-B</b> Smooth surfaces n= 0.011 P2= 3.40"
2.4	462	0.0249	3.20		<b>Shallow Concentrated Flow, Paved</b> Paved Kv= 20.3 fps
3.0					<b>Direct Entry, Direct entry to 6</b>
6.0	512	Total			

**Subcatchment CB-1: New CB South (Seymour Street)**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 126

**Summary for Subcatchment CB-5: PORTLAND ST SOUTH OFFSITE**

Runoff = 3.23 cfs @ 12.13 hrs, Volume= 9,764 cf, Depth= 5.93"  
 Routed to Pond DMH3 : DIVERSION MANHOLE - PORTLAND ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

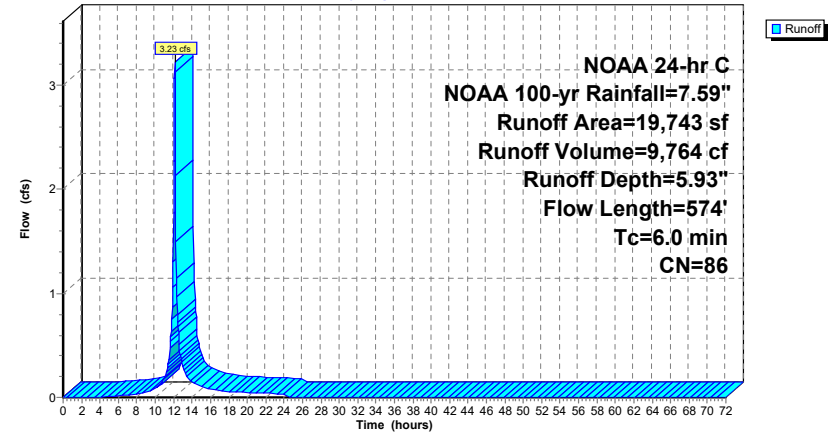
Area (sf)	CN	Description
15,657	83	1/4 acre lots, 38% imp, HSG C
* 4,086	98	Roadway
19,743	86	Weighted Average
9,707		49.17% Pervious Area
10,036		50.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		<b>Sheet Flow, A-B (sheet flow)</b> Smooth surfaces n= 0.011 P2= 3.40"
2.3	524	0.0346	3.78		<b>Shallow Concentrated Flow, B-C (shallow conc.)</b> Paved Kv= 20.3 fps
3.2					<b>Direct Entry, direct to 6</b>
6.0	574	Total			

**Subcatchment CB-5: PORTLAND ST SOUTH OFFSITE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 127

**Summary for Subcatchment CB3: NEW CB SOUTH- HUDSON ST**

Runoff = 4.12 cfs @ 12.13 hrs, Volume= 12,454 cf, Depth= 5.93"  
 Routed to Pond DMH2 : DIVERSION MANHOLE - HUDSON STREET

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

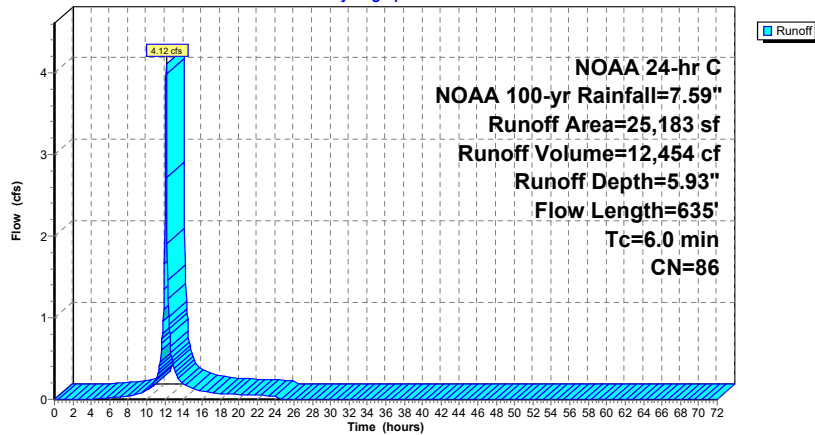
Area (sf)	CN	Description
19,562	83	1/4 acre lots, 38% imp, HSG C
* 5,621	98	Roadway
25,183	86	Weighted Average
12,128		48.16% Pervious Area
13,055		51.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0444	1.70		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
3.0	585	0.0256	3.25		Shallow Concentrated Flow, B-C Paved Kv= 20.3 fps
2.5					Direct Entry, direct entry to 6
6.0	635	Total			

**Subcatchment CB3: NEW CB SOUTH- HUDSON ST**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 128

**Summary for Subcatchment CB4: NEW CB NOTH - HUDSON STREET**

Runoff = 19.32 cfs @ 12.13 hrs, Volume= 58,419 cf, Depth= 5.93"  
 Routed to Pond DMH2 : DIVERSION MANHOLE - HUDSON STREET

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

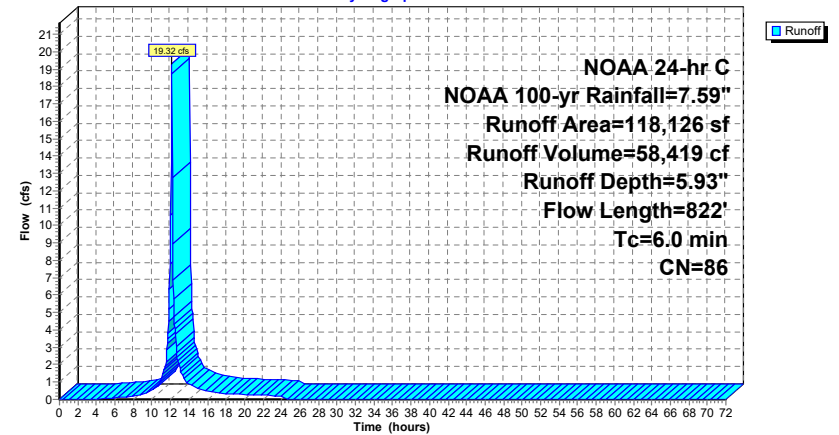
Area (sf)	CN	Description
96,716	83	1/4 acre lots, 38% imp, HSG C
* 21,410	98	Roadway
118,126	86	Weighted Average
59,964		50.76% Pervious Area
58,162		49.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
4.0	772	0.0245	3.18		Shallow Concentrated Flow, B-C (shallow concentrated) Paved Kv= 20.3 fps
1.5					Direct Entry, direct entry to 6
6.0	822	Total			

**Subcatchment CB4: NEW CB NOTH - HUDSON STREET**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 129

**Summary for Reach 1R: ISOLATOR ROW C**

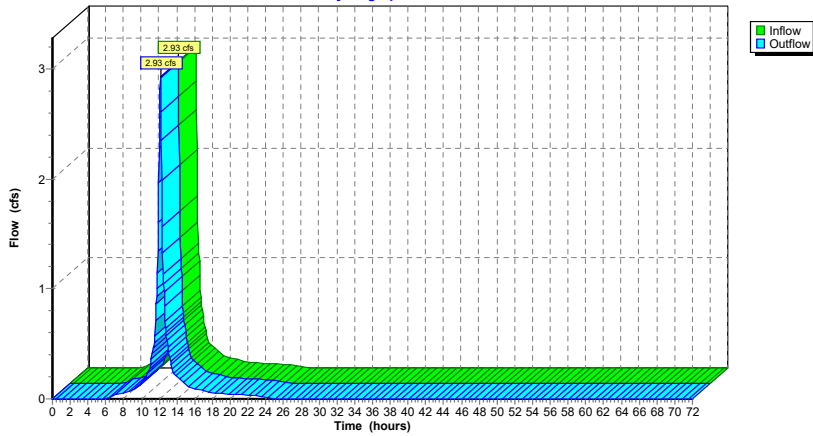
[40] Hint: Not Described (Outflow=Inflow)

Inflow = 2.93 cfs @ 12.14 hrs, Volume= 12,777 cf  
Outflow = 2.93 cfs @ 12.14 hrs, Volume= 12,777 cf, Atten= 0%, Lag= 0.0 min  
Routed to Reach BMP9\_O : BMP-9 OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 1R: ISOLATOR ROW C**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 130

**Summary for Reach 6R: ISOLATOR ROW 2**

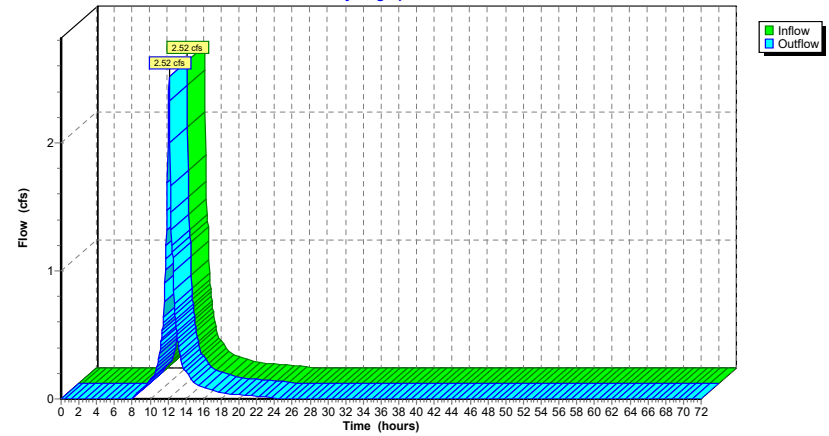
[40] Hint: Not Described (Outflow=Inflow)

Inflow = 2.52 cfs @ 12.14 hrs, Volume= 12,054 cf  
Outflow = 2.52 cfs @ 12.14 hrs, Volume= 12,054 cf, Atten= 0%, Lag= 0.0 min  
Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 6R: ISOLATOR ROW 2**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 131

**Summary for Reach 15R: ISOLATOR ROW 1**

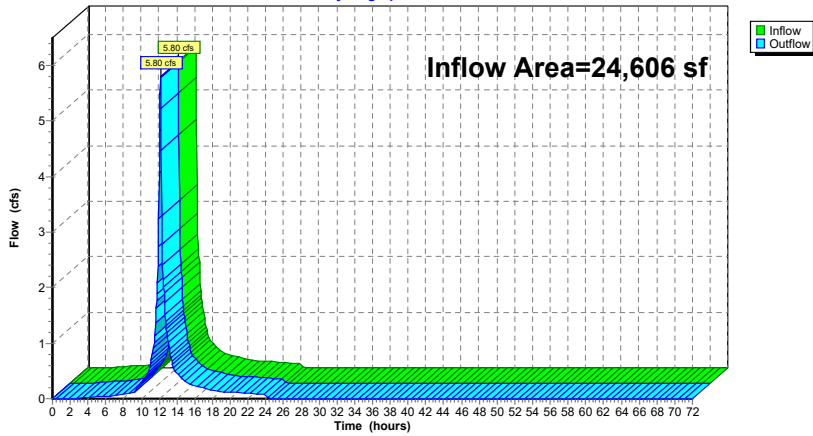
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 24,606 sf, 82.04% Impervious, Inflow Depth = 12.95" for NOAA 100-yr event  
 Inflow = 5.80 cfs @ 12.14 hrs, Volume= 26,550 cf  
 Outflow = 5.80 cfs @ 12.14 hrs, Volume= 26,550 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 15R: ISOLATOR ROW 1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 132

**Summary for Reach B: PARKING LOT B OVERFLOW**

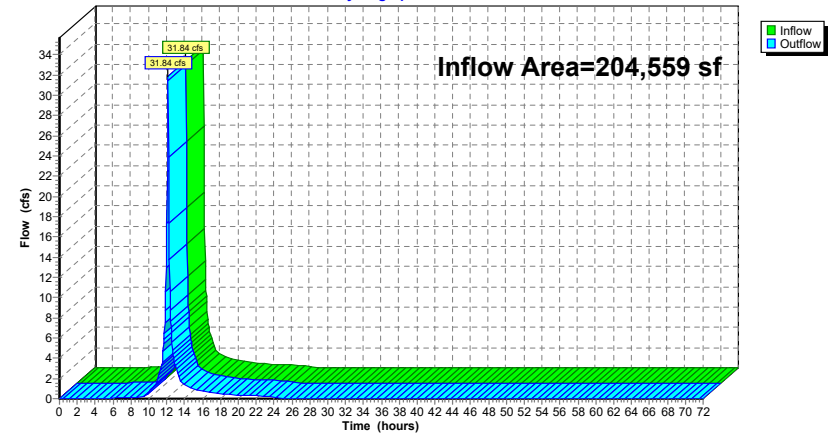
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 204,559 sf, 56.22% Impervious, Inflow Depth = 5.38" for NOAA 100-yr event  
 Inflow = 31.84 cfs @ 12.13 hrs, Volume= 91,717 cf  
 Outflow = 31.84 cfs @ 12.13 hrs, Volume= 91,717 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach B: PARKING LOT B OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 133

**Summary for Reach BMP4\_O: BMP-4 OVERFLOW**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3,048 sf, 86.09% Impervious, Inflow Depth = 12.79" for NOAA 100-yr event

Inflow = 0.57 cfs @ 12.14 hrs, Volume= 3,249 cf

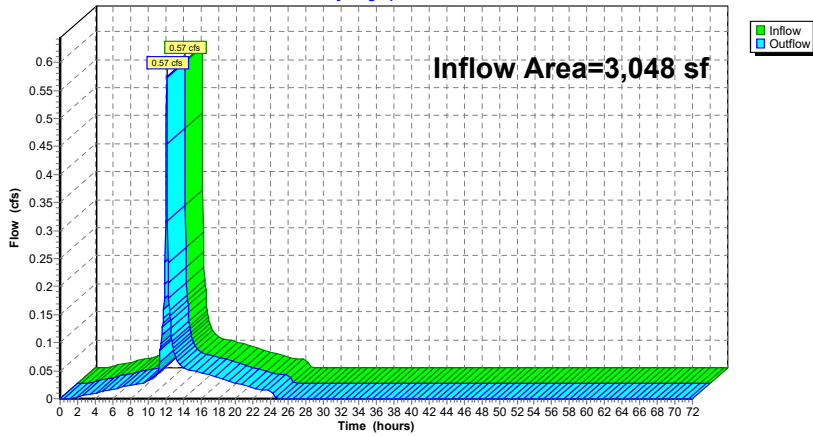
Outflow = 0.57 cfs @ 12.14 hrs, Volume= 3,249 cf, Atten= 0%, Lag= 0.0 min

Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP4\_O: BMP-4 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 134

**Summary for Reach BMP6\_O: BMP-6 OVERFLOW**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 21,643 sf, 56.12% Impervious, Inflow Depth = 5.38" for NOAA 100-yr event

Inflow = 3.58 cfs @ 12.14 hrs, Volume= 9,708 cf

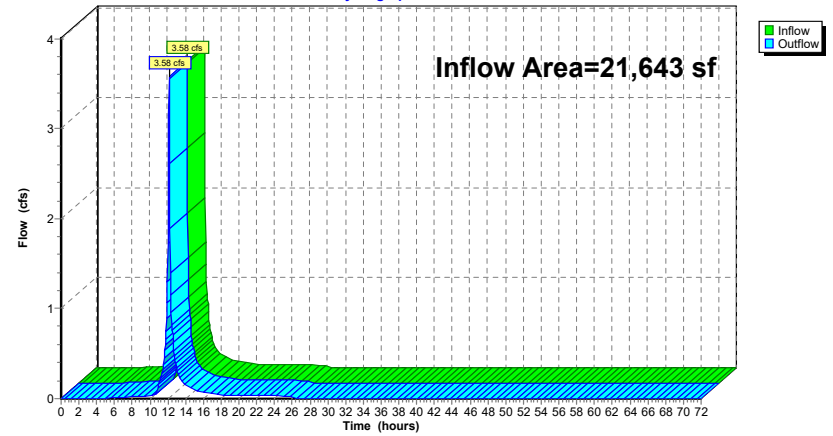
Outflow = 3.58 cfs @ 12.14 hrs, Volume= 9,708 cf, Atten= 0%, Lag= 0.0 min

Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP6\_O: BMP-6 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 135

**Summary for Reach BMP7\_O: BMP-7 OVERFLOW**

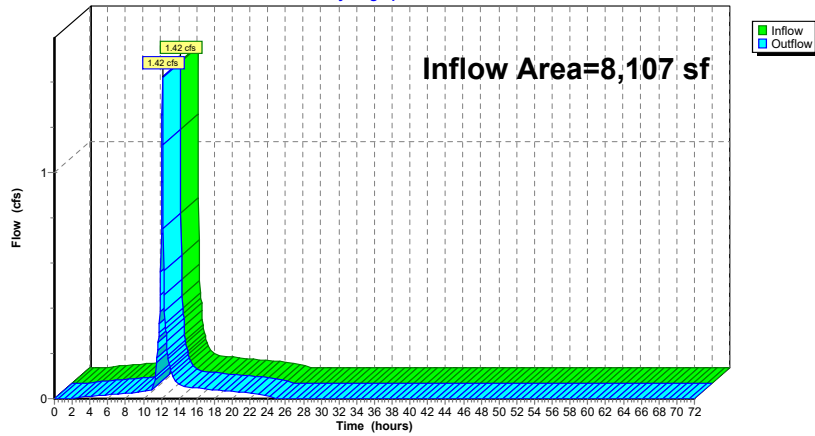
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8,107 sf, 88.34% Impervious, Inflow Depth = 6.99" for NOAA 100-yr event  
 Inflow = 1.42 cfs @ 12.14 hrs, Volume= 4,724 cf  
 Outflow = 1.42 cfs @ 12.14 hrs, Volume= 4,724 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP7\_O: BMP-7 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 136

**Summary for Reach BMP9\_O: BMP-9 OVERFLOW**

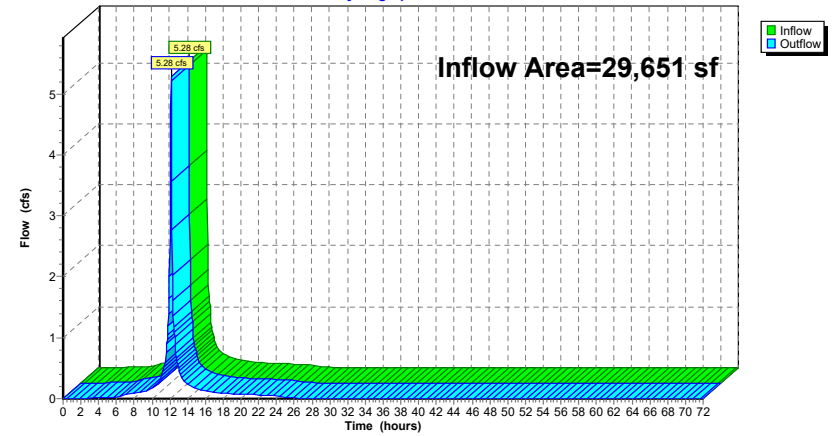
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 29,651 sf, 74.77% Impervious, Inflow Depth = 6.64" for NOAA 100-yr event  
 Inflow = 5.28 cfs @ 12.14 hrs, Volume= 16,401 cf  
 Outflow = 5.28 cfs @ 12.14 hrs, Volume= 16,401 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP9\_O: BMP-9 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 137

**Summary for Reach BMP\_3: BMP-3\_OVERFLOW**

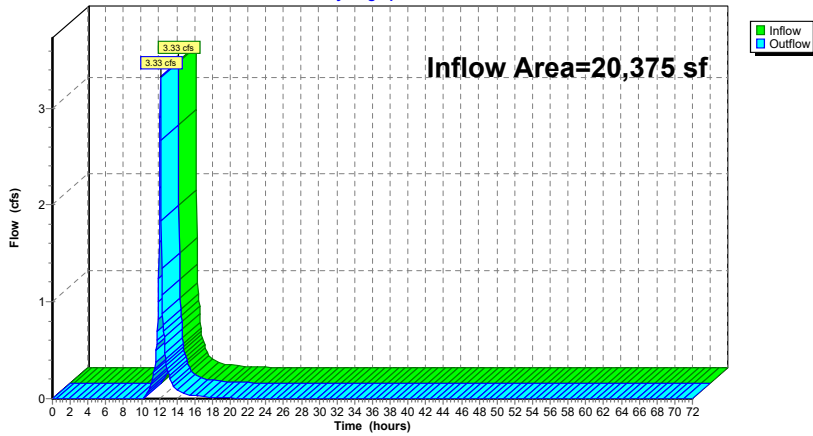
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 20,375 sf, 69.14% Impervious, Inflow Depth = 4.14" for NOAA 100-yr event  
 Inflow = 3.33 cfs @ 12.14 hrs, Volume= 7,035 cf  
 Outflow = 3.33 cfs @ 12.14 hrs, Volume= 7,035 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP\_3: BMP-3\_OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 138

**Summary for Reach DP-1: French Rodney Blvd 14" Outfall**

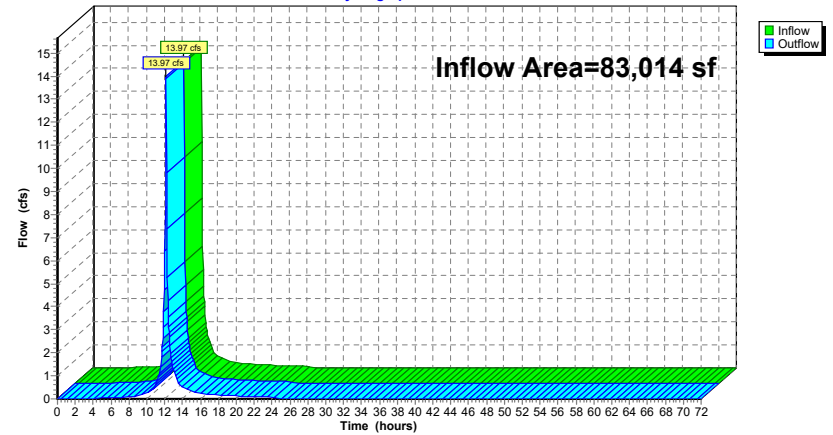
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 83,014 sf, 62.20% Impervious, Inflow Depth = 5.07" for NOAA 100-yr event  
 Inflow = 13.97 cfs @ 12.14 hrs, Volume= 35,096 cf  
 Outflow = 13.97 cfs @ 12.14 hrs, Volume= 35,096 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-1: French Rodney Blvd 14" Outfall**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 139

**Summary for Reach DP-2: NORTHERN OUTFALL**

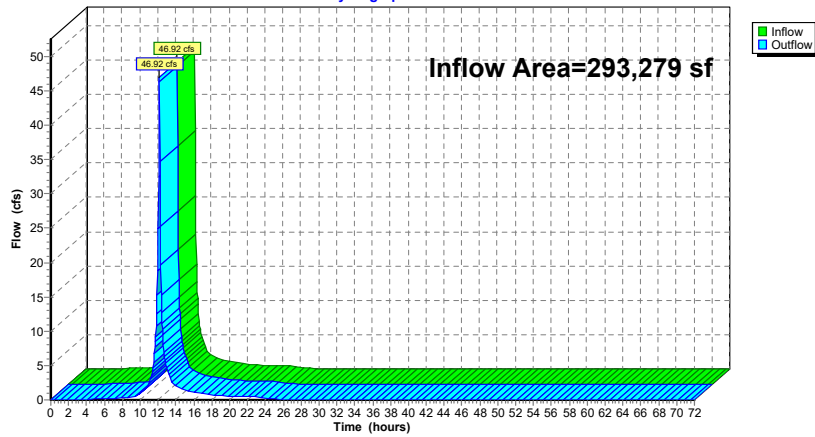
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 293,279 sf, 59.54% Impervious, Inflow Depth = 5.50" for NOAA 100-yr event  
Inflow = 46.92 cfs @ 12.14 hrs, Volume= 134,321 cf  
Outflow = 46.92 cfs @ 12.14 hrs, Volume= 134,321 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-2: NORTHERN OUTFALL**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 140

**Summary for Reach H ST: HUDSON STREET DRAINAGE**

[40] Hint: Not Described (Outflow=Inflow)

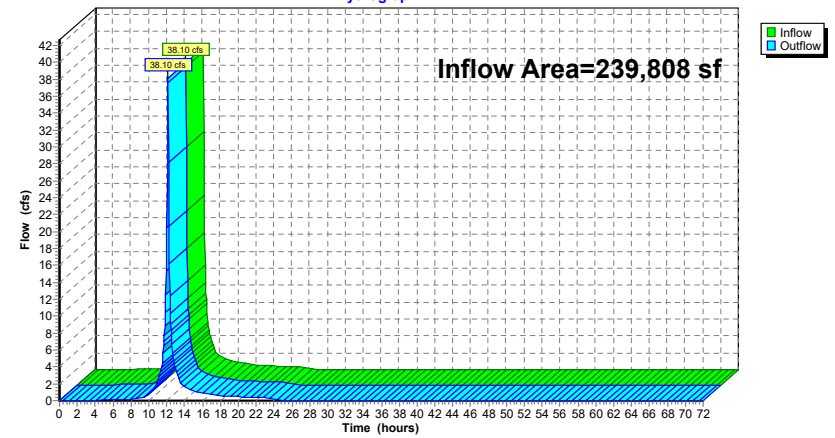
Inflow Area = 239,808 sf, 59.25% Impervious, Inflow Depth = 5.57" for NOAA 100-yr event  
Inflow = 38.10 cfs @ 12.13 hrs, Volume= 111,381 cf  
Outflow = 38.10 cfs @ 12.13 hrs, Volume= 111,381 cf, Atten= 0%, Lag= 0.0 min

Routed to Reach DP-2 : NORTHERN OUTFALL

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach H ST: HUDSON STREET DRAINAGE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 141

**Summary for Reach P ST: PORTLAND STREET DRAINAGE**

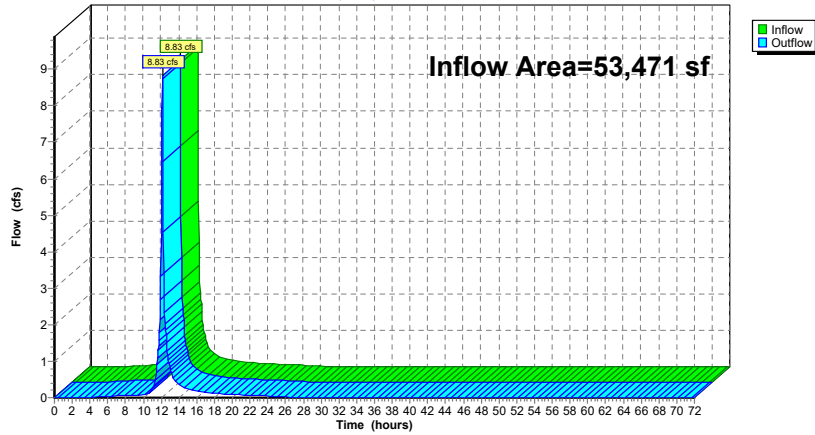
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 53,471 sf, 60.83% Impervious, Inflow Depth = 5.15" for NOAA 100-yr event  
 Inflow = 8.83 cfs @ 12.14 hrs, Volume= 22,940 cf  
 Outflow = 8.83 cfs @ 12.14 hrs, Volume= 22,940 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach DP-2 : NORTHERN OUTFALL

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach P ST: PORTLAND STREET DRAINAGE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 142

**Summary for Pond 1-P: BB 1**

[93] Warning: Storage range exceeded by 0.07'

[88] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 38,826 sf, 51.66% Impervious, Inflow Depth = 5.93" for NOAA 100-yr event  
 Inflow = 6.35 cfs @ 12.13 hrs, Volume= 19,201 cf  
 Outflow = 6.64 cfs @ 12.14 hrs, Volume= 19,201 cf, Atten= 0%, Lag= 0.8 min  
 Discarded = 0.05 cfs @ 12.13 hrs, Volume= 2,860 cf  
 Primary = 4.04 cfs @ 12.14 hrs, Volume= 3,321 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall  
 Secondary = 2.54 cfs @ 12.14 hrs, Volume= 13,021 cf  
 Routed to Reach 15R : ISOLATOR ROW 1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.27' @ 12.14 hrs Surf.Area= 1,830 sf Storage= 1,114 cf

Plug-Flow detention time= 36.1 min calculated for 19,188 cf (100% of inflow)  
 Center-of-Mass det. time= 36.4 min ( 828.3 - 792.0 )

Volume #1	Invert 9.20'	Avail.Storage 1,114 cf	Storage Description
<b>Custom Stage Data (Prismatic) Listed below (Recalc)</b>			
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.20	490	0	0
9.50	800	194	194
10.20	1,830	920	1,114

Device	Routing	Invert	Outlet Devices
#1	Primary	8.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Discarded	9.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.00'
#3	Device 1	10.00'	<b>24inch-Dome Grate Capacity X 2.00</b>
#4	Secondary	9.83'	<b>15inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.05 cfs @ 12.13 hrs HW=10.25' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.05 cfs)

**Primary OutFlow** Max=3.81 cfs @ 12.14 hrs HW=10.26' (Free Discharge)  
 ↳1=Culvert (Passes 3.81 cfs of 5.01 cfs potential flow)  
 ↳3=24inch-Dome Grate Capacity (Custom Controls 3.81 cfs)

**Secondary OutFlow** Max=2.51 cfs @ 12.14 hrs HW=10.26' (Free Discharge)  
 ↳4=15inch-Dome Grate Capacity (Custom Controls 2.51 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

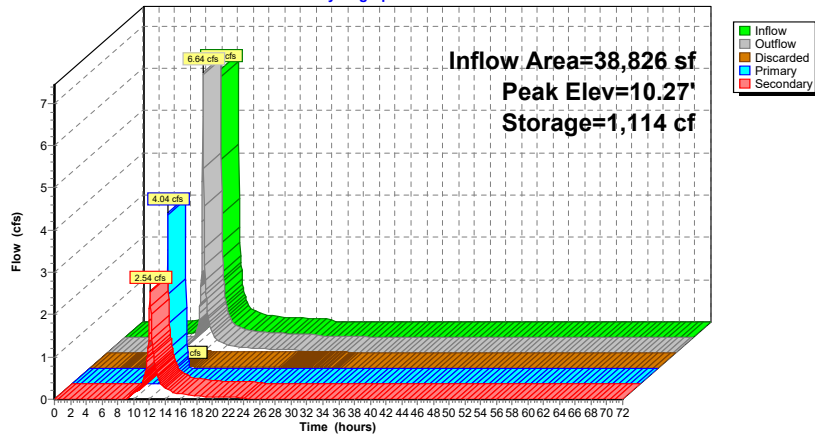
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 143

**Pond 1-P: BB 1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 144

**Summary for Pond 2a-P: BB 2a**

Inflow Area = 3,116 sf, 92.62% Impervious, Inflow Depth = 7.11" for NOAA 100-yr event  
 Inflow = 0.56 cfs @ 12.13 hrs, Volume= 1,847 cf  
 Outflow = 0.54 cfs @ 12.15 hrs, Volume= 1,811 cf, Atten= 3%, Lag= 1.2 min  
 Primary = 0.54 cfs @ 12.15 hrs, Volume= 1,811 cf  
 Routed to Reach 15R : ISOLATOR ROW 1  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.22' @ 12.15 hrs Surf.Area= 494 sf Storage= 91 cf

Plug-Flow detention time= 26.3 min calculated for 1,810 cf (98% of inflow)  
 Center-of-Mass det. time= 13.7 min ( 768.1 - 754.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	710 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)
	8.00	320	0
	9.00	1,100	710

Device	Routing	Invert	Outlet Devices
#1	Secondary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	8.50'	<b>24inch-Dome Grate Capacity X 2.00</b>
#3	Primary	8.10'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.53 cfs @ 12.15 hrs HW=8.22' (Free Discharge)  
 ↳3=15inch-Dome Grate Capacity(Custom Controls 0.53 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.00' (Free Discharge)  
 ↳1=Culvert (Passes 0.00 cfs of 2.27 cfs potential flow)  
 ↳2=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

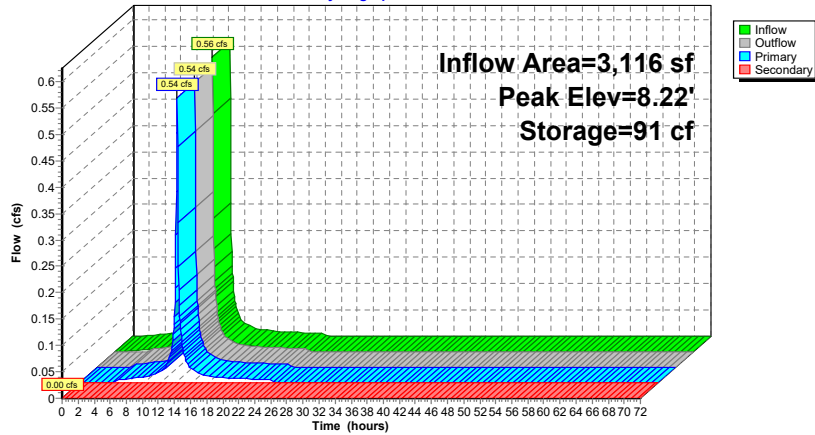
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 145

**Pond 2a-P: BB 2a**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 146

**Summary for Pond 2b-P: BB 2b**

Inflow Area = 21,490 sf, 80.50% Impervious, Inflow Depth = 6.76" for NOAA 100-yr event  
 Inflow = 3.78 cfs @ 12.13 hrs, Volume= 12,099 cf  
 Outflow = 3.74 cfs @ 12.14 hrs, Volume= 12,063 cf, Atten= 1%, Lag= 1.1 min  
 Primary = 2.72 cfs @ 12.14 hrs, Volume= 11,718 cf  
 Routed to Reach 15R : ISOLATOR ROW 1  
 Secondary = 1.02 cfs @ 12.14 hrs, Volume= 344 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.61' @ 12.14 hrs Surf.Area= 793 sf Storage= 338 cf

Plug-Flow detention time= 5.8 min calculated for 12,063 cf (100% of inflow)  
 Center-of-Mass det. time= 3.8 min ( 772.1 - 768.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	710 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)
	8.00	320	0
	9.00	1,100	710

Device	Routing	Invert	Outlet Devices
#1	Secondary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/ S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	8.50'	<b>24inch-Dome Grate Capacity X 2.00</b>
#3	Primary	8.10'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=2.70 cfs @ 12.14 hrs HW=8.60' (Free Discharge)  
 ↳3=15inch-Dome Grate Capacity(Custom Controls 2.70 cfs)

**Secondary OutFlow** Max=0.95 cfs @ 12.14 hrs HW=8.60' (Free Discharge)  
 ↳1=Culvert (Passes 0.95 cfs of 3.92 cfs potential flow)  
 ↳2=24inch-Dome Grate Capacity(Custom Controls 0.95 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

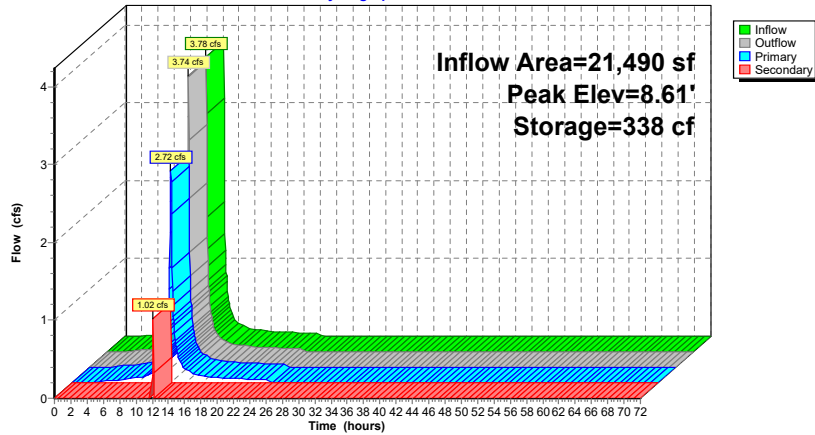
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 147

**Pond 2b-P: BB 2b**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 148

**Summary for Pond 3A-P: BB 3A**

Inflow Area = 10,987 sf, 58.16% Impervious, Inflow Depth = 6.17" for NOAA 100-yr event  
 Inflow = 1.84 cfs @ 12.13 hrs, Volume= 5,647 cf  
 Outflow = 1.75 cfs @ 12.15 hrs, Volume= 5,647 cf, Atten= 5%, Lag= 1.3 min  
 Discarded = 0.03 cfs @ 12.15 hrs, Volume= 1,537 cf  
 Primary = 1.73 cfs @ 12.15 hrs, Volume= 4,110 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.14' @ 12.15 hrs Surf.Area= 953 sf Storage= 511 cf

Plug-Flow detention time= 61.6 min calculated for 5,647 cf (100% of inflow)  
 Center-of-Mass det. time= 61.5 min ( 847.5 - 786.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.25'	622 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.25	271	0	0
10.45	350	62	62
11.25	1,050	560	622

Device	Routing	Invert	Outlet Devices
#1	Primary	9.30'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.30' / 9.20' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.55 sf
#2	Discarded	10.25'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.30'
#3	Primary	11.15'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	10.90'	<b>24inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.03 cfs @ 12.15 hrs HW=11.14' (Free Discharge)  
 ↳ **2=Exfiltration** ( Controls 0.03 cfs)

**Primary OutFlow** Max=1.73 cfs @ 12.15 hrs HW=11.14' (Free Discharge)  
 ↳ **1=Culvert** (Passes 1.73 cfs of 3.13 cfs potential flow)  
 ↳ **4=24inch-Dome Grate Capacity** (Custom Controls 1.73 cfs)  
 ↳ **3=Sharp-Crested Rectangular Weir** ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

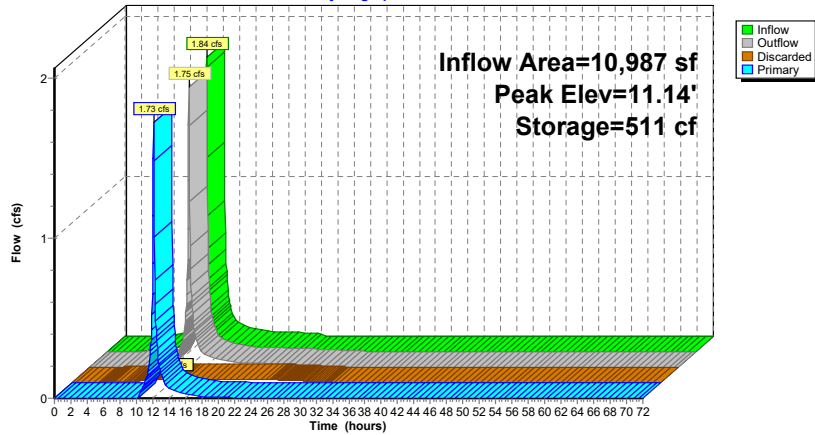
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 149

**Pond 3A-P: BB 3A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 150

**Summary for Pond 3B-P: BB 3B**

[93] Warning: Storage range exceeded by 0.01'

[88] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 4,545 sf, 77.34% Impervious, Inflow Depth = 6.76" for NOAA 100-yr event  
 Inflow = 0.80 cfs @ 12.13 hrs, Volume= 2,559 cf  
 Outflow = 0.82 cfs @ 12.14 hrs, Volume= 2,559 cf, Atten= 0%, Lag= 0.6 min  
 Discarded = 0.02 cfs @ 12.13 hrs, Volume= 983 cf  
 Primary = 0.80 cfs @ 12.14 hrs, Volume= 1,576 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 12.91' @ 12.14 hrs Surf.Area= 570 sf Storage= 263 cf

Plug-Flow detention time= 80.1 min calculated for 2,559 cf (100% of inflow)  
 Center-of-Mass det. time= 80.1 min ( 848.3 - 768.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	12.20'	263 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
12.20	180	0	0
12.90	570	263	263

Device	Routing	Invert	Outlet Devices
#1	Primary	10.70'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.70' / 10.60' S= 0.0100 /' Cc= 0.900 n= 0.013, Flow Area= 0.55 sf
#2	Discarded	12.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 8.70'
#3	Device 1	12.80'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	12.85'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.02 cfs @ 12.13 hrs HW=12.91' (Free Discharge)  
 ↳ **2=Exfiltration** ( Controls 0.02 cfs)

**Primary OutFlow** Max=0.77 cfs @ 12.14 hrs HW=12.91' (Free Discharge)  
 ↳ **1=Culvert** (Passes 0.53 cfs of 3.52 cfs potential flow)  
 ↳ **3=24inch-Dome Grate Capacity** (Custom Controls 0.53 cfs)  
 ↳ **4=Sharp-Crested Rectangular Weir** (Weir Controls 0.24 cfs @ 0.80 fps)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

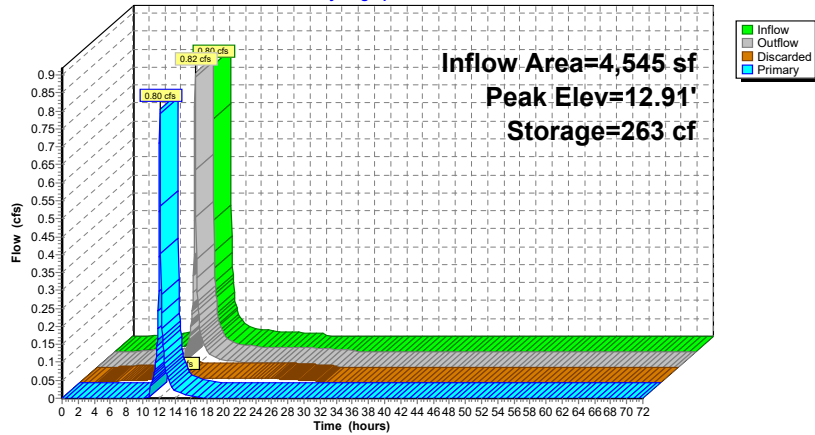
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 151

**Pond 3B-P: BB 3B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 152

**Summary for Pond 4A-P: BB 4A - POND**

Inflow Area = 4,843 sf, 86.37% Impervious, Inflow Depth = 6.99" for NOAA 100-yr event  
 Inflow = 0.86 cfs @ 12.13 hrs, Volume= 2,822 cf  
 Outflow = 0.84 cfs @ 12.14 hrs, Volume= 2,822 cf, Atten= 3%, Lag= 1.1 min  
 Primary = 0.80 cfs @ 12.14 hrs, Volume= 1,349 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW  
 Secondary = 0.04 cfs @ 12.14 hrs, Volume= 1,473 cf  
 Routed to Pond 4A-S : BB4A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.09' @ 12.14 hrs Surf.Area= 601 sf Storage= 253 cf

Plug-Flow detention time= 29.0 min calculated for 2,820 cf (100% of inflow)  
 Center-of-Mass det. time= 29.0 min ( 788.4 - 759.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.50'	320 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet) Cum.Store (cubic-feet)
	9.50	250	0 0
	10.20	664	320 320

Device	Routing	Invert	Outlet Devices
#1	Primary	8.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 '/ S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.00'
#3	Primary	10.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	9.95'	<b>24inchDome Grate Capacity</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950

**Primary OutFlow** Max=0.79 cfs @ 12.14 hrs HW=10.09' (Free Discharge)  
 1=Culvert (Passes 0.79 cfs of 4.77 cfs potential flow)  
 4=24inchDome Grate Capacity (Custom Controls 0.79 cfs)  
 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.14 hrs HW=10.09' (Free Discharge)  
 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

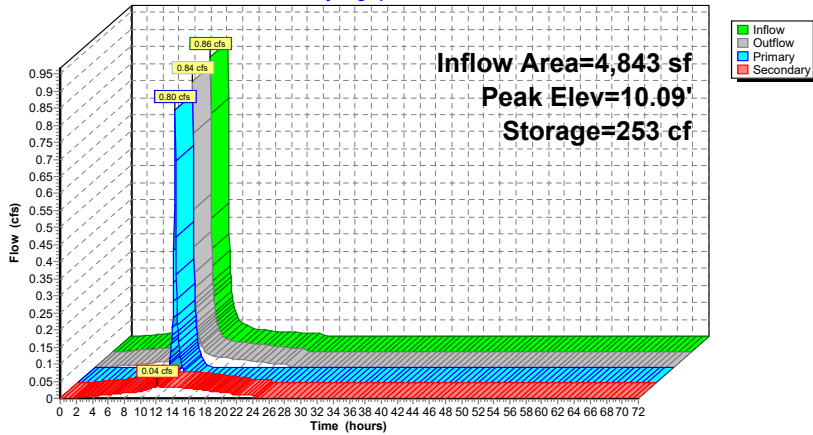
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 153

**Pond 4A-P: BB 4A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 154

**Summary for Pond 4A-S: BB4A-Stone**

Inflow = 0.04 cfs @ 12.14 hrs, Volume= 1,473 cf  
 Outflow = 0.04 cfs @ 12.18 hrs, Volume= 1,473 cf, Atten= 1%, Lag= 1.9 min  
 Primary = 0.04 cfs @ 12.18 hrs, Volume= 1,473 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.13' @ 12.18 hrs Surf.Area= 230 sf Storage= 9 cf

Plug-Flow detention time= 5.4 min calculated for 1,473 cf (100% of inflow)  
 Center-of-Mass det. time= 5.3 min ( 843.7 - 838.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	138 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 460 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	230	0	0
8.00	230	460	460

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.18 hrs HW=6.13' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.21 fps)



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

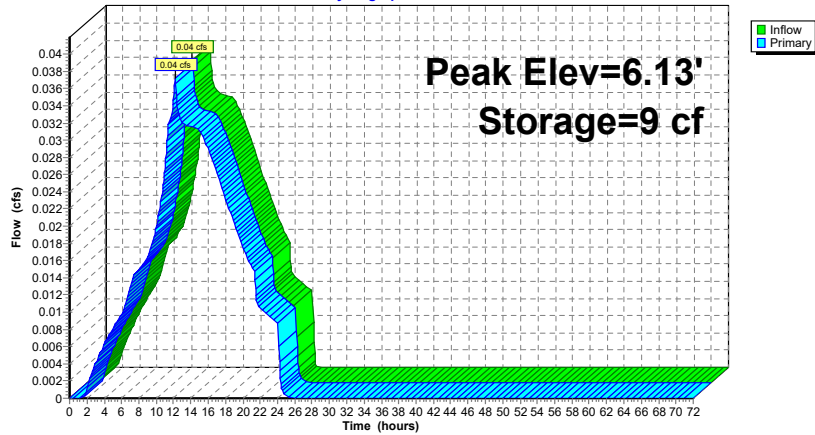
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 155

**Pond 4A-S: BB4A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 156

**Summary for Pond 4B-P: BB 4B - POND**

Inflow Area = 3,048 sf, 86.09% Impervious, Inflow Depth = 6.99" for NOAA 100-yr event  
 Inflow = 0.54 cfs @ 12.13 hrs, Volume= 1,776 cf  
 Outflow = 0.54 cfs @ 12.14 hrs, Volume= 1,776 cf, Atten= 1%, Lag= 1.0 min  
 Primary = 0.52 cfs @ 12.14 hrs, Volume= 858 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW  
 Secondary = 0.02 cfs @ 12.14 hrs, Volume= 919 cf  
 Routed to Pond 4B-S : BB 4A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.06' @ 12.14 hrs Surf.Area= 367 sf Storage= 142 cf

Plug-Flow detention time= 28.5 min calculated for 1,775 cf (100% of inflow)  
 Center-of-Mass det. time= 28.5 min ( 787.9 - 759.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.50'	199 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.50	144	0	0
11.20	424	199	199

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.00' / 8.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.00'
#3	Primary	11.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	10.95'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.50 cfs @ 12.14 hrs HW=11.06' (Free Discharge)  
 1=Culvert (Passes 0.50 cfs of 4.72 cfs potential flow)  
 4=24inch-Dome Grate Capacity (Custom Controls 0.50 cfs)  
 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.02 cfs @ 12.14 hrs HW=11.06' (Free Discharge)  
 2=Exfiltration ( Controls 0.02 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

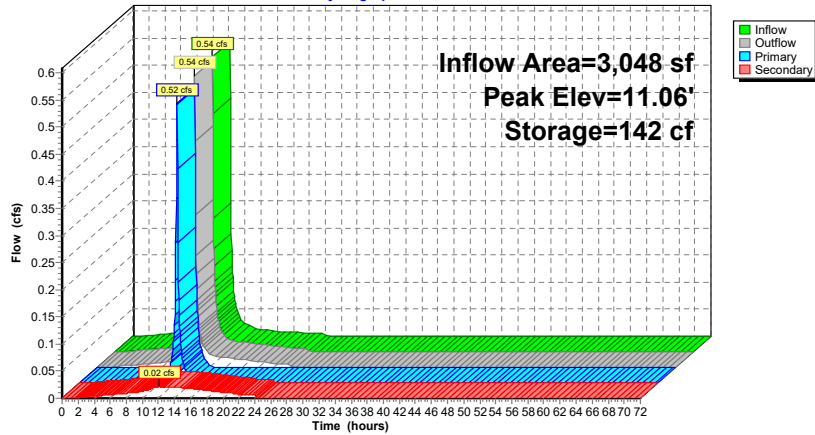
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 157

**Pond 4B-P: BB 4B - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 158

**Summary for Pond 4B-S: BB 4A-Stone**

Inflow = 0.02 cfs @ 12.14 hrs, Volume= 919 cf  
 Outflow = 0.02 cfs @ 12.17 hrs, Volume= 919 cf, Atten= 1%, Lag= 1.5 min  
 Primary = 0.02 cfs @ 12.17 hrs, Volume= 919 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.10' @ 12.17 hrs Surf.Area= 145 sf Storage= 4 cf

Plug-Flow detention time= 4.1 min calculated for 918 cf (100% of inflow)  
 Center-of-Mass det. time= 4.2 min ( 843.4 - 839.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	87 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 290 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	145	0	0
8.00	145	290	290

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.17 hrs HW=6.10' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.02 cfs @ 1.06 fps)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

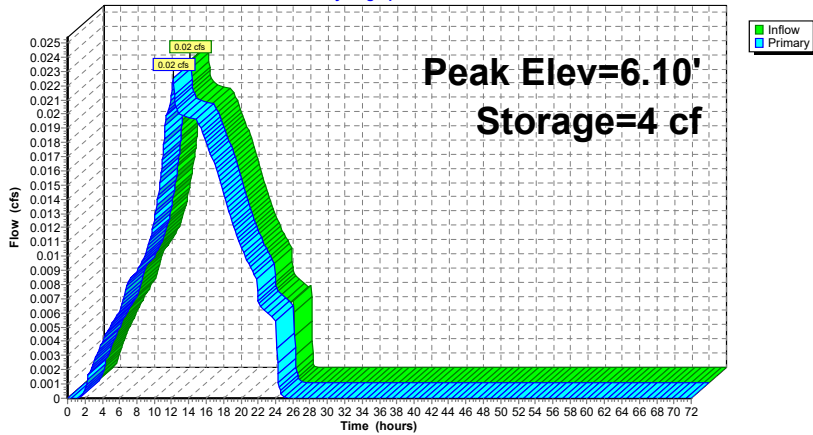
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 159

**Pond 4B-S: BB 4A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 160

**Summary for Pond 5A-P: BB 5A - POND**

Inflow Area = 3,072 sf, 73.44% Impervious, Inflow Depth = 6.64" for NOAA 100-yr event  
 Inflow = 0.54 cfs @ 12.13 hrs, Volume= 1,699 cf  
 Outflow = 0.37 cfs @ 12.22 hrs, Volume= 1,699 cf, Atten= 31%, Lag= 5.4 min  
 Primary = 0.32 cfs @ 12.22 hrs, Volume= 278 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW  
 Secondary = 0.05 cfs @ 12.22 hrs, Volume= 1,422 cf  
 Routed to Pond 5A-PS : BB 5A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.58' @ 12.22 hrs Surf.Area= 736 sf Storage= 472 cf

Plug-Flow detention time= 61.4 min calculated for 1,698 cf (100% of inflow)  
 Center-of-Mass det. time= 61.4 min ( 833.6 - 772.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.80'	645 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.80	480	0	0
9.80	810	645	645

Device	Routing	Invert	Outlet Devices
#1	Primary	7.60'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.60' / 7.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.80'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.60'
#3	Device 1	9.50'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.29 cfs @ 12.22 hrs HW=9.57' (Free Discharge)  
 ↑1=Culvert (Passes 0.29 cfs of 4.58 cfs potential flow)  
 ↑3=24inch-Dome Grate Capacity (Custom Controls 0.29 cfs)

**Secondary OutFlow** Max=0.05 cfs @ 12.22 hrs HW=9.57' (Free Discharge)  
 ↑2=Exfiltration ( Controls 0.05 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

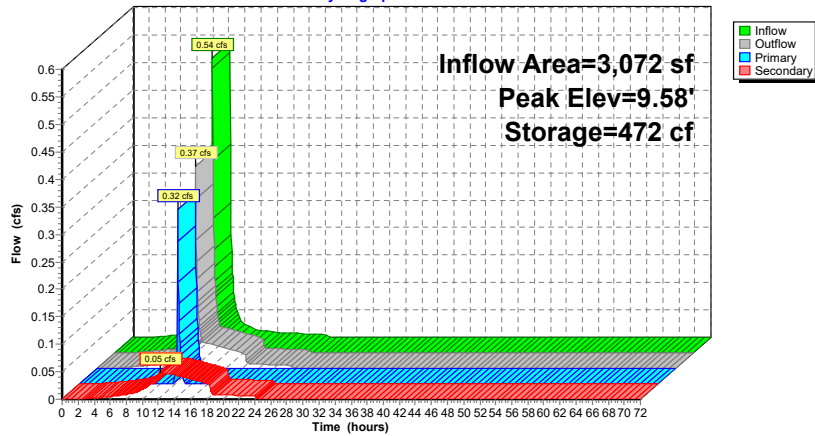
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 161

**Pond 5A-P: BB 5A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 162

**Summary for Pond 5A-PS: BB 5A-Stone**

Inflow = 0.05 cfs @ 12.22 hrs, Volume= 1,422 cf  
 Outflow = 0.05 cfs @ 12.34 hrs, Volume= 1,422 cf, Atten= 2%, Lag= 7.2 min  
 Primary = 0.05 cfs @ 12.34 hrs, Volume= 1,422 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.15' @ 12.34 hrs Surf.Area= 480 sf Storage= 21 cf

Plug-Flow detention time= 10.3 min calculated for 1,421 cf (100% of inflow)  
 Center-of-Mass det. time= 10.4 min ( 862.0 - 851.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	288 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 960 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	480	0	0
8.00	480	960	960

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.05 cfs @ 12.34 hrs HW=6.15' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.05 cfs @ 1.30 fps)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

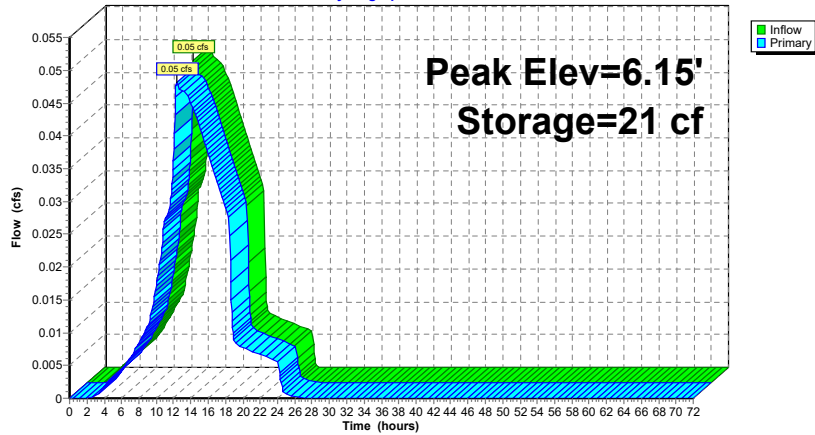
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 163

**Pond 5A-PS: BB 5A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 164

**Summary for Pond 5B-P: BB 5B - POND**

Inflow Area = 34,755 sf, 71.39% Impervious, Inflow Depth = 6.52" for NOAA 100-yr event  
 Inflow = 6.01 cfs @ 12.13 hrs, Volume= 18,883 cf  
 Outflow = 5.90 cfs @ 12.14 hrs, Volume= 18,883 cf, Atten= 2%, Lag= 0.9 min  
 Primary = 3.29 cfs @ 12.14 hrs, Volume= 2,579 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW  
 Secondary = 0.09 cfs @ 12.14 hrs, Volume= 4,250 cf  
 Routed to Pond 5B-PS : BB 5B-Stone  
 Tertiary = 2.52 cfs @ 12.14 hrs, Volume= 12,054 cf  
 Routed to Reach 6R : ISOLATOR ROW 2

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.13' @ 12.14 hrs Surf.Area= 1,372 sf Storage= 790 cf

Plug-Flow detention time= 20.4 min calculated for 18,883 cf (100% of inflow)  
 Center-of-Mass det. time= 20.4 min ( 796.3 - 775.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.20'	889 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.20	327	0	0
9.20	1,450	889	889

Device	Routing	Invert	Outlet Devices
#1	Primary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 ' /' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.20'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.00'
#3	Device 1	8.90'	<b>24inchDome Grate Capacity X 2.00</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Tertiary	8.70'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=3.19 cfs @ 12.14 hrs HW=9.12' (Free Discharge)

1=Culvert (Passes 3.19 cfs of 4.82 cfs potential flow)

3=24inchDome Grate Capacity (Custom Controls 3.19 cfs)

**Secondary OutFlow** Max=0.09 cfs @ 12.14 hrs HW=9.13' (Free Discharge)

2=Exfiltration ( Controls 0.09 cfs)

**Tertiary OutFlow** Max=2.50 cfs @ 12.14 hrs HW=9.13' (Free Discharge)

4=15inch-Dome Grate Capacity (Custom Controls 2.50 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

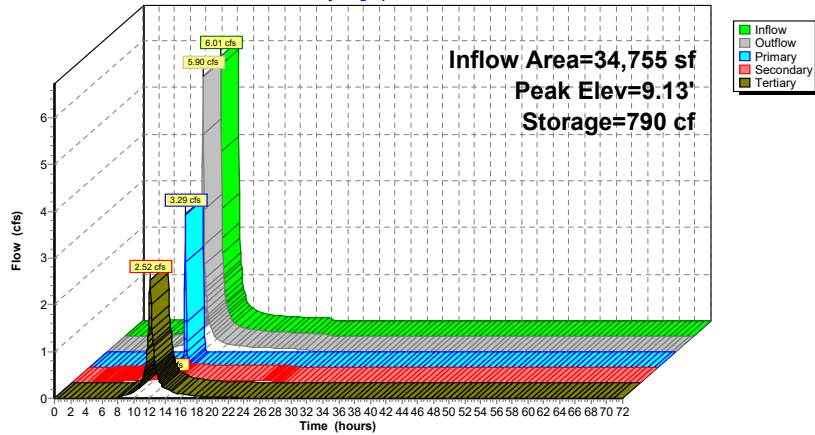
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 165

**Pond 5B-P: BB 5B - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 166

**Summary for Pond 5B-PS: BB 5B-Stone**

[44] Hint: Outlet device #1 is below defined storage

Inflow = 0.09 cfs @ 12.14 hrs, Volume= 4,250 cf  
 Outflow = 0.09 cfs @ 12.14 hrs, Volume= 4,250 cf, Atten= 0%, Lag= 0.1 min  
 Primary = 0.09 cfs @ 12.14 hrs, Volume= 4,250 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.00' @ 12.14 hrs Surf.Area= 690 sf Storage= 1 cf

Plug-Flow detention time= 0.1 min calculated for 4,248 cf (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 917.0 - 916.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	414 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 1,380 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	690	0	0
8.00	690	1,380	1,380

Device	Routing	Invert	Outlet Devices
#1	Primary	4.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.57 cfs @ 12.14 hrs HW=6.00' (Free Discharge)  
 #1=Orifice/Grate (Orifice Controls 0.57 cfs @ 6.53 fps)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

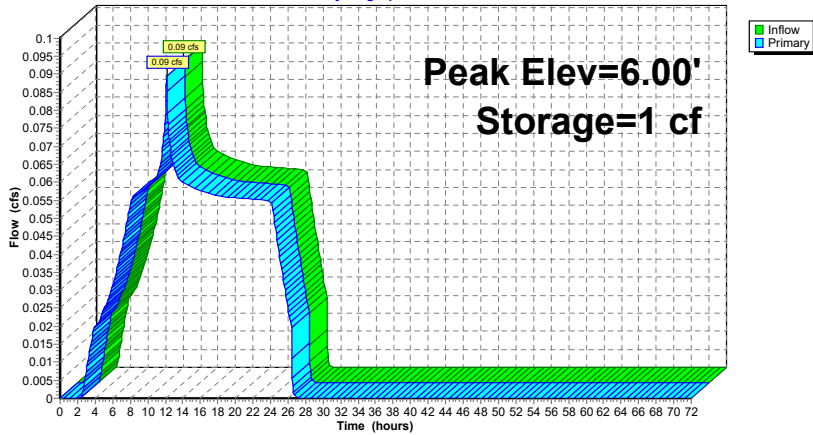
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 167

**Pond 5B-PS: BB 5B-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 168

**Summary for Pond 6A-P: BB 6A - POND**

Inflow Area = 15,148 sf, 46.97% Impervious, Inflow Depth = 5.82" for NOAA 100-yr event  
 Inflow = 2.44 cfs @ 12.13 hrs, Volume= 7,345 cf  
 Outflow = 2.43 cfs @ 12.14 hrs, Volume= 7,345 cf, Atten= 0%, Lag= 0.8 min  
 Primary = 2.38 cfs @ 12.14 hrs, Volume= 4,859 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW  
 Secondary = 0.05 cfs @ 12.14 hrs, Volume= 2,486 cf  
 Routed to Pond 6A-PS : BB 6A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.07' @ 12.14 hrs Surf.Area= 727 sf Storage= 469 cf

Plug-Flow detention time= 35.4 min calculated for 7,345 cf (100% of inflow)  
 Center-of-Mass det. time= 35.3 min ( 830.1 - 794.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.20'	491 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.20	350	0	0
11.10	740	491	491

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.00' / 8.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.20'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.00'
#3	Device 1	10.80'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	11.00'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=2.32 cfs @ 12.14 hrs HW=11.07' (Free Discharge)  
 1=Culvert (Passes 2.04 cfs of 4.73 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 2.04 cfs)  
 4=Sharp-Crested Rectangular Weir (Weir Controls 0.29 cfs @ 0.85 fps)

**Secondary OutFlow** Max=0.05 cfs @ 12.14 hrs HW=11.07' (Free Discharge)  
 2=Exfiltration ( Controls 0.05 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

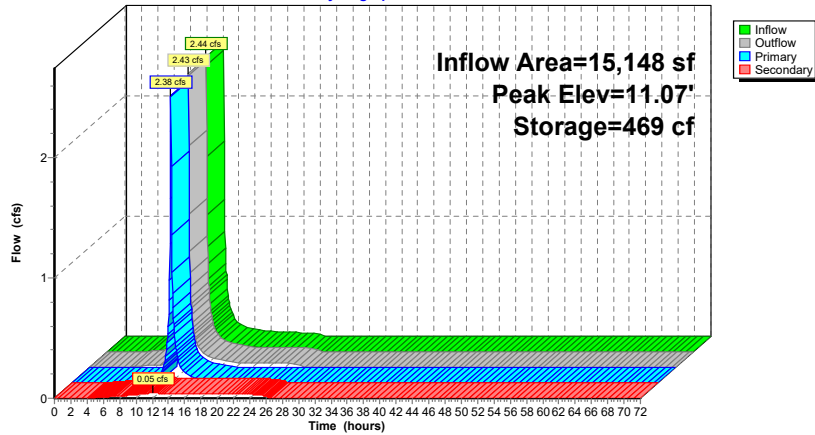
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 169

**Pond 6A-P: BB 6A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 170

**Summary for Pond 6A-PS: BB 6A - STONE**

Inflow = 0.05 cfs @ 12.14 hrs, Volume= 2,486 cf  
 Outflow = 0.05 cfs @ 12.17 hrs, Volume= 2,486 cf, Atten= 1%, Lag= 2.0 min  
 Primary = 0.05 cfs @ 12.17 hrs, Volume= 2,486 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.15' @ 12.17 hrs Surf.Area= 290 sf Storage= 13 cf

Plug-Flow detention time= 5.2 min calculated for 2,484 cf (100% of inflow)  
 Center-of-Mass det. time= 5.3 min ( 992.1 - 986.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	174 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 580 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	290	0	0
8.00	290	580	580

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.05 cfs @ 12.17 hrs HW=6.15' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.05 cfs @ 1.30 fps)



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

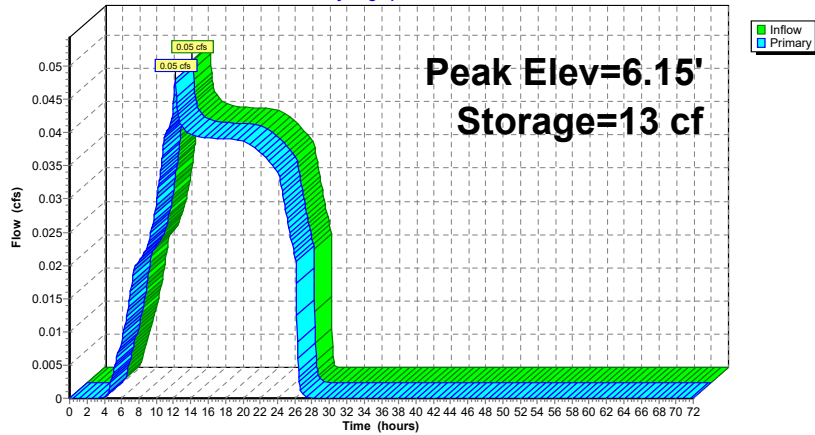
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 171

**Pond 6A-PS: BB 6A - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 172

**Summary for Pond 6B-P: BB 6B**

[93] Warning: Storage range exceeded by 0.03'

[88] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 6,495 sf, 77.45% Impervious, Inflow Depth = 6.76" for NOAA 100-yr event  
 Inflow = 1.14 cfs @ 12.13 hrs, Volume= 3,657 cf  
 Outflow = 1.17 cfs @ 12.14 hrs, Volume= 3,657 cf, Atten= 0%, Lag= 0.6 min  
 Discarded = 0.02 cfs @ 12.13 hrs, Volume= 1,294 cf  
 Primary = 1.15 cfs @ 12.14 hrs, Volume= 2,363 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 12.13' @ 12.14 hrs Surf.Area= 690 sf Storage= 394 cf

Plug-Flow detention time= 88.5 min calculated for 3,654 cf (100% of inflow)  
 Center-of-Mass det. time= 88.7 min ( 857.0 - 768.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	11.20'	394 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
11.20	185	0	0
12.10	690	394	394

Device	Routing	Invert	Outlet Devices
#1	Primary	10.10'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.10' / 10.00' S= 0.0100 /' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Discarded	11.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 8.10'
#3	Device 1	11.95'	<b>24inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.02 cfs @ 12.13 hrs HW=12.12' (Free Discharge)  
 ↳ **2=Exfiltration** ( Controls 0.02 cfs)

**Primary OutFlow** Max=1.10 cfs @ 12.14 hrs HW=12.12' (Free Discharge)  
 ↳ **1=Culvert** (Passes 1.10 cfs of 4.67 cfs potential flow)  
 ↳ **3=24inch-Dome Grate Capacity** (Custom Controls 1.10 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

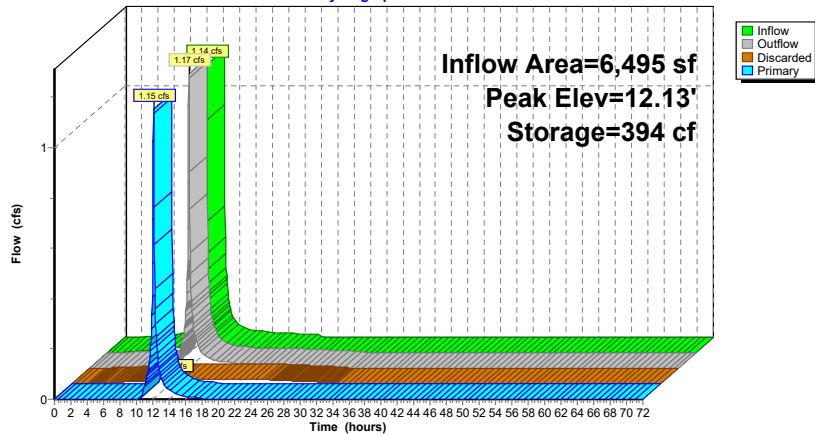
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 173

**Pond 6B-P: BB 6B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 174

**Summary for Pond 7A-P: BB 7A PONDING**

Inflow Area = 3,165 sf, 87.74% Impervious, Inflow Depth = 6.99" for NOAA 100-yr event  
 Inflow = 0.56 cfs @ 12.13 hrs, Volume= 1,844 cf  
 Outflow = 0.56 cfs @ 12.14 hrs, Volume= 1,844 cf, Atten= 0%, Lag= 0.9 min  
 Primary = 0.54 cfs @ 12.14 hrs, Volume= 916 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW  
 Secondary = 0.02 cfs @ 12.14 hrs, Volume= 928 cf  
 Routed to Pond 7A-S : BB 7A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.01' @ 12.14 hrs Surf.Area= 332 sf Storage= 159 cf

Plug-Flow detention time= 37.5 min calculated for 1,843 cf (100% of inflow)  
 Center-of-Mass det. time= 37.5 min ( 796.9 - 759.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.30'	227 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.30	115	0	0
10.20	390	227	227

Device	Routing	Invert	Outlet Devices
#1	Primary	8.10'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.10' / 8.00' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.30'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.10'
#3	Device 1	9.90'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	10.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.53 cfs @ 12.14 hrs HW=10.01' (Free Discharge)  
 1=Culvert (Passes 0.53 cfs of 4.49 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 0.53 cfs)  
 4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.02 cfs @ 12.14 hrs HW=10.01' (Free Discharge)  
 2=Exfiltration ( Controls 0.02 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

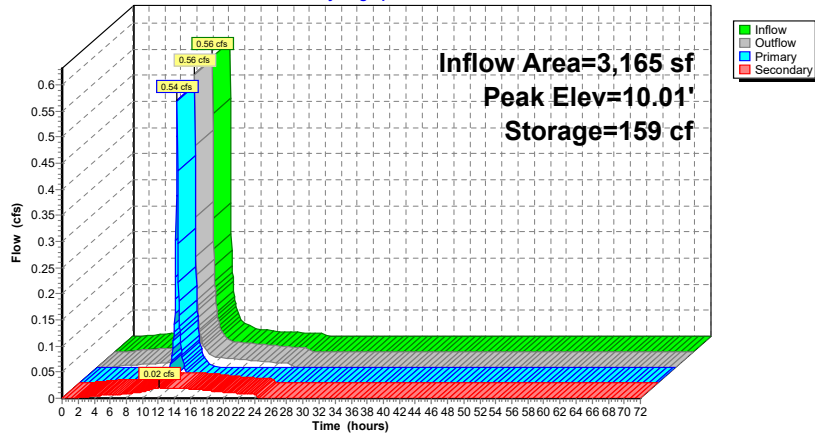
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 175

**Pond 7A-P: BB 7A PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 176

**Summary for Pond 7A-S: BB 7A - STONE**

Inflow = 0.02 cfs @ 12.14 hrs, Volume= 928 cf  
 Outflow = 0.02 cfs @ 12.16 hrs, Volume= 928 cf, Atten= 1%, Lag= 1.5 min  
 Primary = 0.02 cfs @ 12.16 hrs, Volume= 928 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5.19' @ 12.16 hrs Surf.Area= 150 sf Storage= 4 cf

Plug-Flow detention time= 4.3 min calculated for 928 cf (100% of inflow)  
 Center-of-Mass det. time= 4.3 min ( 863.7 - 859.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	5.10'	90 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 300 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
5.10	150	0	0
7.10	150	300	300

Device	Routing	Invert	Outlet Devices
#1	Primary	5.10'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.16 hrs HW=5.19' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.02 cfs @ 1.04 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

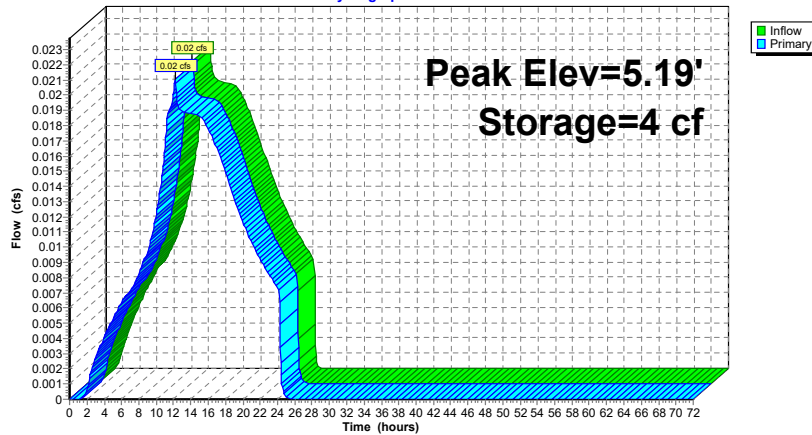
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 177

**Pond 7A-S: BB 7A - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 178

**Summary for Pond 7B-P: BB 7B PONDING**

Inflow Area = 4,942 sf, 88.73% Impervious, Inflow Depth = 6.99" for NOAA 100-yr event  
 Inflow = 0.88 cfs @ 12.13 hrs, Volume= 2,880 cf  
 Outflow = 0.86 cfs @ 12.14 hrs, Volume= 2,880 cf, Atten= 2%, Lag= 1.0 min  
 Primary = 0.82 cfs @ 12.14 hrs, Volume= 1,369 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW  
 Secondary = 0.04 cfs @ 12.14 hrs, Volume= 1,511 cf  
 Routed to Pond 7B-S : BB 7B - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.75' @ 12.14 hrs Surf.Area= 539 sf Storage= 295 cf

Plug-Flow detention time= 38.8 min calculated for 2,878 cf (100% of inflow)  
 Center-of-Mass det. time= 38.8 min ( 798.3 - 759.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	324 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	250	0	0
10.80	560	324	324

Device	Routing	Invert	Outlet Devices
#1	Primary	8.90'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.90' / 8.80' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.00'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.90'
#3	Device 1	10.60'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.81 cfs @ 12.14 hrs HW=10.74' (Free Discharge)  
 1=Culvert (Passes 0.81 cfs of 4.39 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 0.81 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.14 hrs HW=10.74' (Free Discharge)  
 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

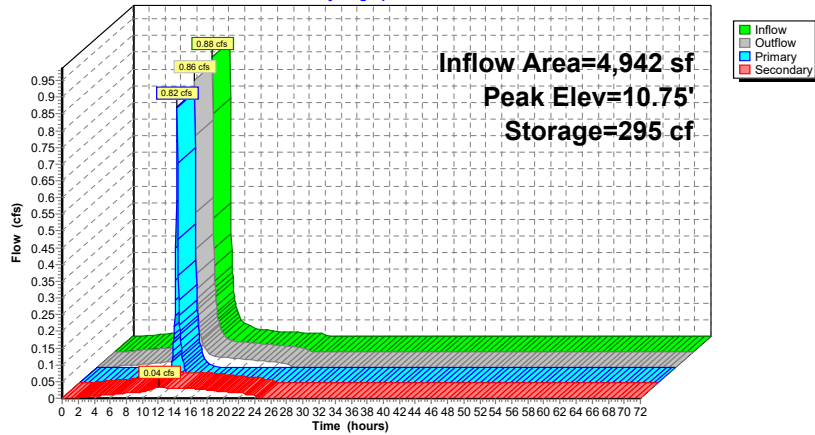
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 179

**Pond 7B-P: BB 7B PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 180

**Summary for Pond 7B-S: BB 7B - STONE**

Inflow = 0.04 cfs @ 12.14 hrs, Volume= 1,511 cf  
 Outflow = 0.04 cfs @ 12.16 hrs, Volume= 1,511 cf, Atten= 0%, Lag= 1.2 min  
 Primary = 0.04 cfs @ 12.16 hrs, Volume= 1,511 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5.22' @ 12.16 hrs Surf.Area= 150 sf Storage= 6 cf

Plug-Flow detention time= 3.4 min calculated for 1,510 cf (100% of inflow)  
 Center-of-Mass det. time= 3.4 min ( 858.9 - 855.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	5.10'	90 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 300 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
5.10	150	0	0
7.10	150	300	300

Device	Routing	Invert	Outlet Devices
#1	Primary	5.10'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.03 cfs @ 12.16 hrs HW=5.22' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.19 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

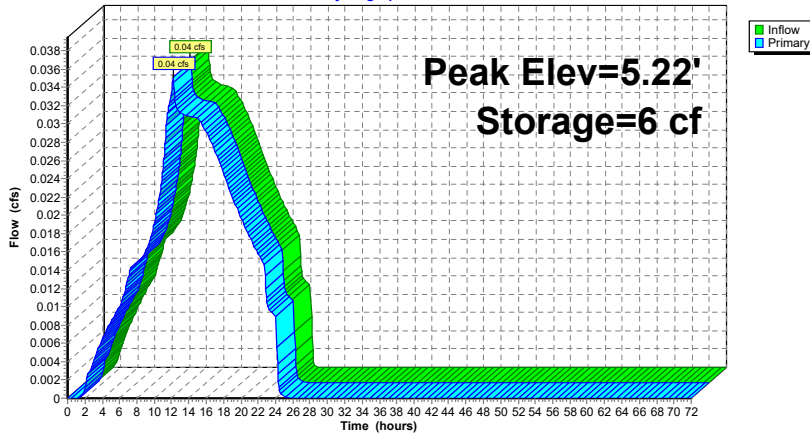
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 181

**Pond 7B-S: BB 7B - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 182

**Summary for Pond 8a-P: BB 8A PONDING**

Inflow Area = 3,978 sf, 79.99% Impervious, Inflow Depth = 6.76" for NOAA 100-yr event  
 Inflow = 0.70 cfs @ 12.13 hrs, Volume= 2,240 cf  
 Outflow = 0.68 cfs @ 12.15 hrs, Volume= 2,240 cf, Atten= 3%, Lag= 1.3 min  
 Primary = 0.64 cfs @ 12.15 hrs, Volume= 855 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE  
 Secondary = 0.04 cfs @ 12.15 hrs, Volume= 1,385 cf  
 Routed to Pond 8a-s : BB 8A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.12' @ 12.15 hrs Surf.Area= 628 sf Storage= 308 cf

Plug-Flow detention time= 35.0 min calculated for 2,240 cf (100% of inflow)  
 Center-of-Mass det. time= 35.0 min ( 803.3 - 768.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.50'	575 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet) Cum.Store (cubic-feet)
	8.50	360	0 0
	9.50	790	575 575

Device	Routing	Invert	Outlet Devices
#1	Primary	7.40'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.40' / 7.30' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.40'
#3	Device 1	9.00'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	9.40'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.63 cfs @ 12.15 hrs HW=9.12' (Free Discharge)  
 1=Culvert (Passes 0.63 cfs of 4.18 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 0.63 cfs)  
 4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.15 hrs HW=9.12' (Free Discharge)  
 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

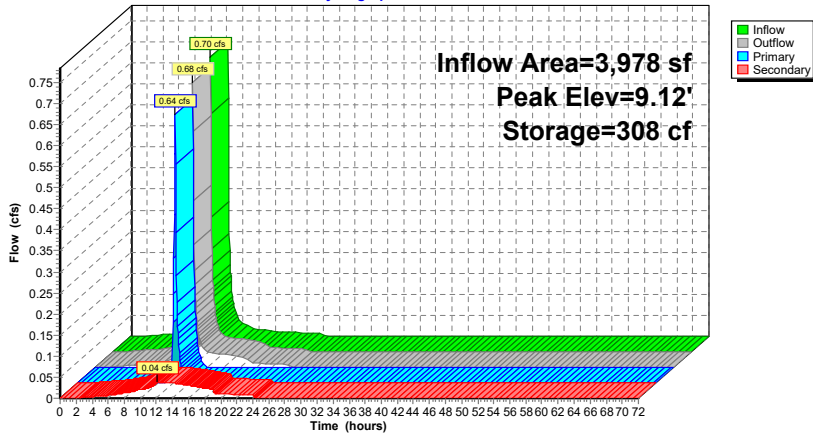
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 183

**Pond 8a-P: BB 8A PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 184

**Summary for Pond 8a-s: BB 8A - STONE**

Inflow = 0.04 cfs @ 12.15 hrs, Volume= 1,385 cf  
 Outflow = 0.04 cfs @ 12.19 hrs, Volume= 1,385 cf, Atten= 1%, Lag= 2.4 min  
 Primary = 0.04 cfs @ 12.19 hrs, Volume= 1,385 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 4.53' @ 12.19 hrs Surf.Area= 300 sf Storage= 12 cf

Plug-Flow detention time= 6.8 min calculated for 1,384 cf (100% of inflow)  
 Center-of-Mass det. time= 6.9 min ( 851.6 - 844.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	4.40'	180 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 600 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
4.40	300	0	0
6.40	300	600	600

Device	Routing	Invert	Outlet Devices
#1	Primary	4.40'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.19 hrs HW=4.53' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.24 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

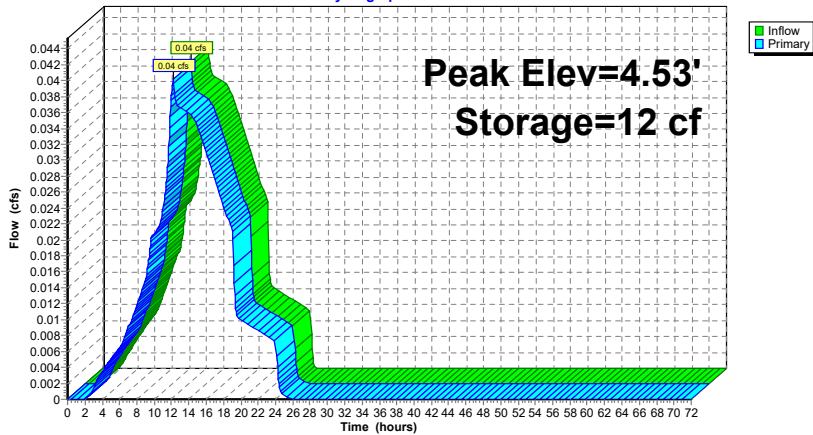
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 185

**Pond 8a-s: BB 8A - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 186

**Summary for Pond 8B-P: BB 8B-PONDING**

Inflow Area = 5,598 sf, 87.78% Impervious, Inflow Depth = 6.99" for NOAA 100-yr event  
 Inflow = 1.00 cfs @ 12.13 hrs, Volume= 3,262 cf  
 Outflow = 0.98 cfs @ 12.14 hrs, Volume= 3,262 cf, Atten= 1%, Lag= 0.9 min  
 Primary = 0.94 cfs @ 12.14 hrs, Volume= 1,582 cf  
 Routed to Reach H ST : HUDSON STREET DRAINAGE  
 Secondary = 0.04 cfs @ 12.14 hrs, Volume= 1,680 cf  
 Routed to Pond 8B-S : BB 8B-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.75' @ 12.14 hrs Surf.Area= 651 sf Storage= 274 cf

Plug-Flow detention time= 34.0 min calculated for 3,260 cf (100% of inflow)  
 Center-of-Mass det. time= 34.0 min ( 793.4 - 759.4 )

Volume #	Invert	Avail.Storage	Storage Description
#1	9.10'	306 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet) Cum.Store (cubic-feet)
	9.10	190	0 0
	9.80	685	306 306

Device	Routing	Invert	Outlet Devices
#1	Primary	7.90'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.90' / 7.80' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.10'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.90'
#3	Device 1	9.65'	<b>24inch-Dome Grate Capacity X 2.00</b>

**Primary OutFlow** Max=0.92 cfs @ 12.14 hrs HW=9.75' (Free Discharge)  
 1=Culvert (Passes 0.92 cfs of 4.39 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 0.92 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.14 hrs HW=9.75' (Free Discharge)  
 2=Exfiltration ( Controls 0.04 cfs)



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

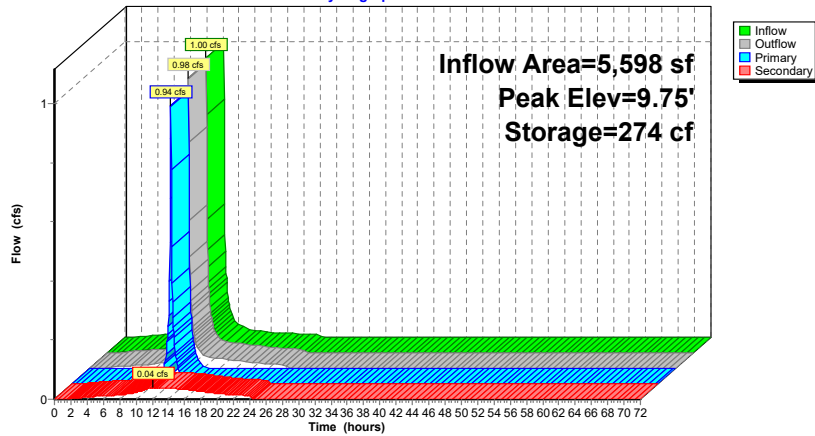
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 187

**Pond 8B-P: BB 8B-PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 188

**Summary for Pond 8B-S: BB 8B-Stone**

Inflow = 0.04 cfs @ 12.14 hrs, Volume= 1,680 cf  
 Outflow = 0.04 cfs @ 12.18 hrs, Volume= 1,680 cf, Atten= 1%, Lag= 2.2 min  
 Primary = 0.04 cfs @ 12.18 hrs, Volume= 1,680 cf  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 4.53' @ 12.18 hrs Surf.Area= 300 sf Storage= 12 cf

Plug-Flow detention time= 6.7 min calculated for 1,680 cf (100% of inflow)  
 Center-of-Mass det. time= 6.5 min ( 857.0 - 850.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	4.40'	180 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 600 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
4.40	300	0	0
6.40	300	600	600

Device	Routing	Invert	Outlet Devices
#1	Primary	4.40'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.18 hrs HW=4.53' (Free Discharge)  
 ↳ **1=Orifice/Grate** (Orifice Controls 0.04 cfs @ 1.24 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

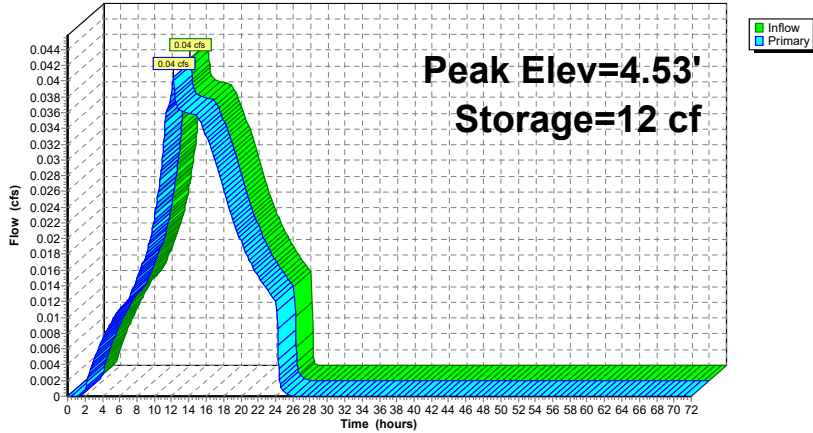
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 189

**Pond 8B-S: BB 8B-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 190

**Summary for Pond 9-P: BB9 - POND**

[93] Warning: Storage range exceeded by 0.09'

[88] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 29,651 sf, 74.77% Impervious, Inflow Depth = 6.64" for NOAA 100-yr event  
 Inflow = 5.18 cfs @ 12.13 hrs, Volume= 16,401 cf  
 Outflow = 5.28 cfs @ 12.14 hrs, Volume= 16,401 cf, Atten= 0%, Lag= 0.6 min  
 Primary = 2.30 cfs @ 12.14 hrs, Volume= 1,149 cf  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW  
 Secondary = 0.05 cfs @ 12.13 hrs, Volume= 2,475 cf  
 Routed to Pond 9-PS : BB9 - STONE  
 Tertiary = 2.93 cfs @ 12.14 hrs, Volume= 12,777 cf  
 Routed to Reach 1R : ISOLATOR ROW C

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.09' @ 12.13 hrs Surf.Area= 780 sf Storage= 485 cf

Plug-Flow detention time= 14.0 min calculated for 16,390 cf (100% of inflow)  
 Center-of-Mass det. time= 14.1 min ( 786.3 - 772.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	485 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	190	0	0
9.00	780	485	485

Device	Routing	Invert	Outlet Devices
#1	Primary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.00'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.00'
#3	Device 1	8.80'	<b>24inchDome Grate Capacity</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Tertiary	8.50'	<b>15inch-Dome Grate Capacity</b>

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 191

**Primary OutFlow** Max=2.12 cfs @ 12.14 hrs HW=9.07' (Free Discharge)

↳1=Culvert (Passes 2.12 cfs of 4.75 cfs potential flow)

↳3=24inchDome Grate Capacity (Custom Controls 2.12 cfs)

**Secondary OutFlow** Max=0.05 cfs @ 12.13 hrs HW=9.07' (Free Discharge)

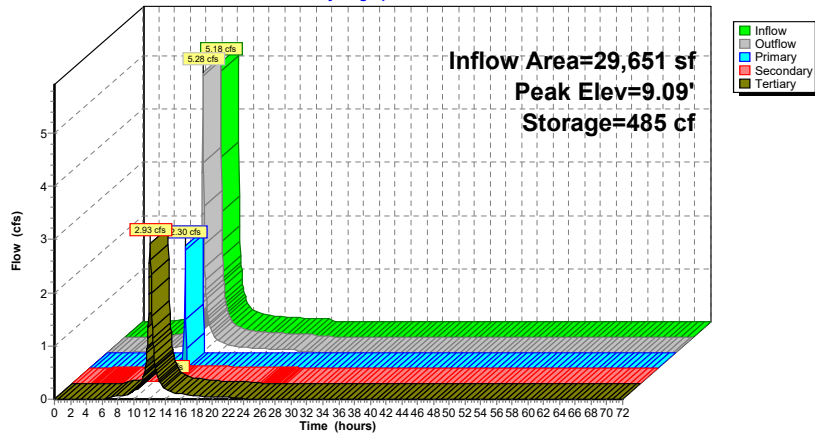
↳2=Exfiltration ( Controls 0.05 cfs)

**Tertiary OutFlow** Max=2.89 cfs @ 12.14 hrs HW=9.08' (Free Discharge)

↳4=15inch-Dome Grate Capacity (Custom Controls 2.89 cfs)

**Pond 9-P: BB9 - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 192

**Summary for Pond 9-PS: BB9 - STONE**

Inflow = 0.05 cfs @ 12.13 hrs, Volume= 2,475 cf  
 Outflow = 0.05 cfs @ 12.16 hrs, Volume= 2,475 cf, Atten= 1%, Lag= 1.5 min  
 Primary = 0.05 cfs @ 12.16 hrs, Volume= 2,475 cf  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.15' @ 12.16 hrs Surf.Area= 190 sf Storage= 9 cf

Plug-Flow detention time= 3.8 min calculated for 2,475 cf (100% of inflow)  
 Center-of-Mass det. time= 3.6 min ( 891.9 - 888.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	114 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 380 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	190	0	0
8.00	190	380	380

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.05 cfs @ 12.16 hrs HW=6.15' (Free Discharge)

↳1=Orifice/Grate (Orifice Controls 0.05 cfs @ 1.34 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

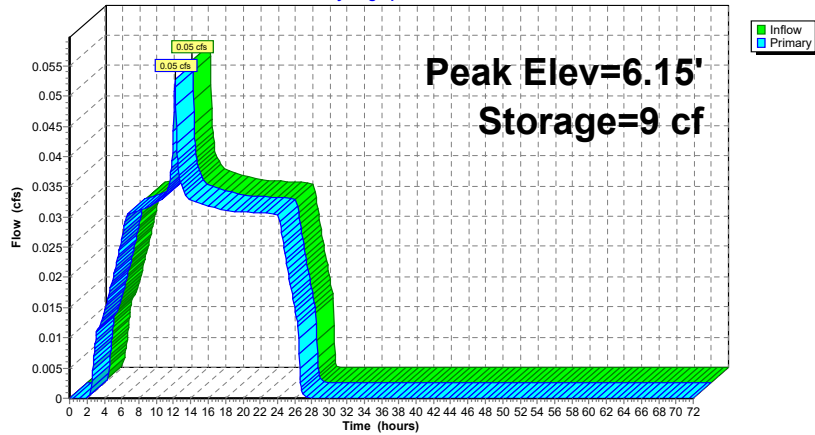
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 193

**Pond 9-PS: BB9 - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 194

**Summary for Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST**

[57] Hint: Peaked at 10.10' (Flood elevation advised)

Inflow Area = 19,582 sf, 58.17% Impervious, Inflow Depth = 6.17" for NOAA 100-yr event  
 Inflow = 3.28 cfs @ 12.13 hrs, Volume= 10,065 cf  
 Outflow = 3.28 cfs @ 12.13 hrs, Volume= 10,065 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 0.87 cfs @ 12.13 hrs, Volume= 7,068 cf  
 Routed to Pond INF-1 : INFILTRATION SYSTEM #1  
 Secondary = 2.41 cfs @ 12.13 hrs, Volume= 2,996 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.10' @ 12.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	6.0" Vert. WATER QUALITY STORM DIVERSION C= 0.600 Limited to weir flow at low heads
#2	Secondary	9.20'	12.0" Vert. LARGE STORM OVEFLOW C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.86 cfs @ 12.13 hrs HW=10.07' (Free Discharge)

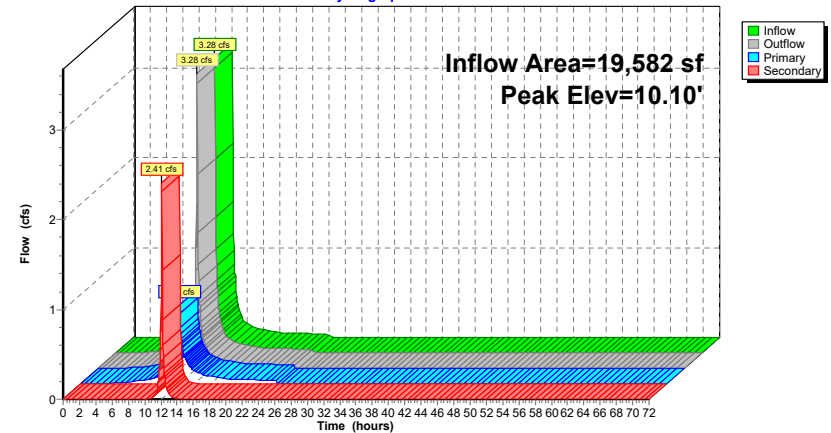
↳1=WATER QUALITY STORM DIVERSION(Orifice Controls 0.86 cfs @ 4.35 fps)

**Secondary OutFlow** Max=2.30 cfs @ 12.13 hrs HW=10.07' (Free Discharge)

↳2=LARGE STORM OVEFLOW (Orifice Controls 2.30 cfs @ 3.17 fps)

**Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 195

**Summary for Pond DMH2: DIVERSION MANHOLE - HUDSON STREET**

[57] Hint: Peaked at 35.73' (Flood elevation advised)

Inflow Area = 143,309 sf, 49.69% Impervious, Inflow Depth = 5.93" for NOAA 100-yr event  
 Inflow = 23.44 cfs @ 12.13 hrs, Volume= 70,873 cf  
 Outflow = 23.44 cfs @ 12.13 hrs, Volume= 70,873 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 4.74 cfs @ 12.13 hrs, Volume= 35,221 cf  
 Routed to Pond INF-2 : INFILTRATION SYSTEM #2  
 Secondary = 18.70 cfs @ 12.13 hrs, Volume= 35,651 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 35.73' @ 12.13 hrs

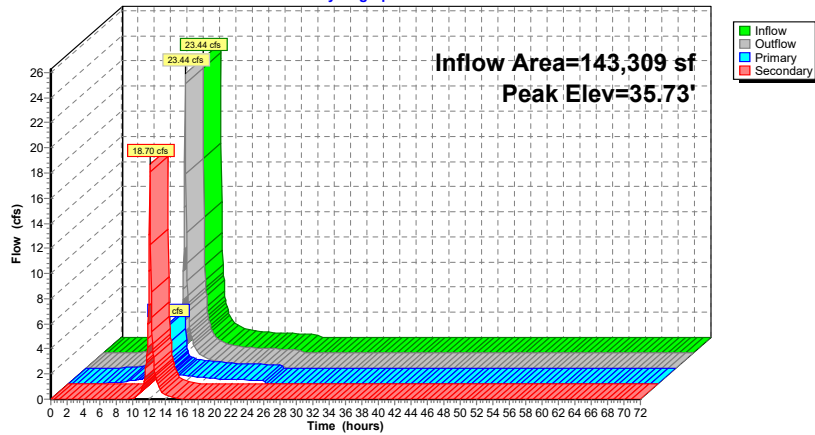
Device	Routing	Invert	Outlet Devices
#1	Primary	10.60'	<b>6.0" Vert. WATER QUALITY STORM DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	11.10'	<b>12.0" Vert. LARGE STORM OVERFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=4.56 cfs @ 12.13 hrs HW=34.10' (Free Discharge)  
 ↳1=WATER QUALITY STORM DIVERSION(Orifice Controls 4.56 cfs @ 23.22 fps)

**Secondary OutFlow** Max=17.94 cfs @ 12.13 hrs HW=34.10' (Free Discharge)  
 ↳2=LARGE STORM OVERFLOW(Orifice Controls 17.94 cfs @ 22.84 fps)

**Pond DMH2: DIVERSION MANHOLE - HUDSON STREET**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 196

**Summary for Pond DMH3: DIVERSION MANHOLE - PORTLAND ST**

[57] Hint: Peaked at 12.07' (Flood elevation advised)

Inflow Area = 19,743 sf, 50.83% Impervious, Inflow Depth = 5.93" for NOAA 100-yr event  
 Inflow = 3.23 cfs @ 12.13 hrs, Volume= 9,764 cf  
 Outflow = 3.23 cfs @ 12.13 hrs, Volume= 9,764 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 1.44 cfs @ 12.13 hrs, Volume= 7,738 cf  
 Routed to Pond INF3 : INFILTRATION SYSTEM #1  
 Secondary = 1.78 cfs @ 12.13 hrs, Volume= 2,026 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 12.07' @ 12.13 hrs

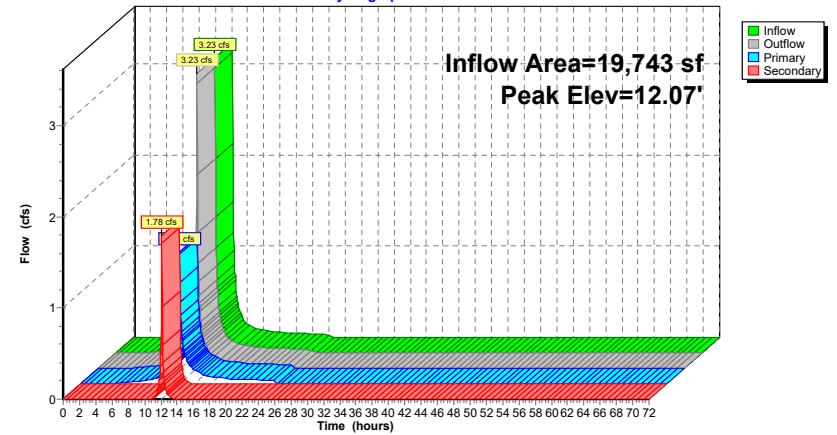
Device	Routing	Invert	Outlet Devices
#1	Primary	11.00'	<b>8.0" Vert. WATER QUALITY DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	11.20'	<b>10.0" Vert. LARGE STORM OVERFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=1.40 cfs @ 12.13 hrs HW=12.03' (Free Discharge)  
 ↳1=WATER QUALITY DIVERSION(Orifice Controls 1.40 cfs @ 4.02 fps)

**Secondary OutFlow** Max=1.69 cfs @ 12.13 hrs HW=12.03' (Free Discharge)  
 ↳2=LARGE STORM OVERFLOW(Orifice Controls 1.69 cfs @ 3.10 fps)

**Pond DMH3: DIVERSION MANHOLE - PORTLAND ST**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 197

**Summary for Pond INF-1: INFILTRATION SYSTEM #1**

[88] Warning: Qout>Qin may require smaller dt or Finer Routing

[81] Warning: Exceeded Pond DMH1 by 0.24' @ 16.80 hrs

Inflow Area = 19,582 sf, 58.17% Impervious, Inflow Depth = 4.33" for NOAA 100-yr event  
 Inflow = 0.87 cfs @ 12.13 hrs, Volume= 7,068 cf  
 Outflow = 0.89 cfs @ 12.17 hrs, Volume= 7,068 cf, Atten= 0%, Lag= 2.5 min  
 Discarded = 0.08 cfs @ 12.17 hrs, Volume= 5,184 cf  
 Primary = 0.82 cfs @ 12.17 hrs, Volume= 1,885 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.53' @ 12.17 hrs Surf.Area= 1,772 sf Storage= 1,727 cf

Plug-Flow detention time= 185.9 min calculated for 7,068 cf (100% of inflow)  
 Center-of-Mass det. time= 185.8 min ( 996.2 - 810.4 )

Volume	Invert	Avail.Storage	Storage Description
#1A	7.80'	1,091 cf	<b>21.50'W x 81.52'L x 2.33'H Field A</b> 4,090 cf Overall - 973 cf Embedded = 3,117 cf x 35.0% Voids
#2A	8.30'	973 cf	<b>ADS_StormTech SC-310 +Cap</b> x 66 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 66 Chambers in 6 Rows
#3	7.80'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		2,201 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	7.80'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.80'
#2	Primary	8.10'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.10' / 8.00' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.40'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.08 cfs @ 12.17 hrs HW=9.53' (Free Discharge)  
 ↳ **1=Exfiltration** ( Controls 0.08 cfs)

**Primary OutFlow** Max=0.74 cfs @ 12.17 hrs HW=9.53' (Free Discharge)  
 ↳ **2=Culvert** (Passes 0.74 cfs of 2.64 cfs potential flow)  
 ↳ **3=Sharp-Crested Rectangular Weir**(Weir Controls 0.74 cfs @ 1.17 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 198

**Pond INF-1: INFILTRATION SYSTEM #1 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-310 +Cap (ADS StormTech®SC-310 with cap length)**

Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf

Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap

34.0" Wide + 6.0" Spacing = 40.0" C-C Row Spacing

11 Chambers/Row x 7.12' Long +0.60' Cap Length x 2 = 79.52' Row Length +12.0" End Stone x 2 = 81.52' Base Length

6 Rows x 34.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 21.50' Base Width

6.0" Stone Base + 16.0" Chamber Height + 6.0" Stone Cover = 2.33' Field Height

66 Chambers x 14.7 cf = 973.0 cf Chamber Storage

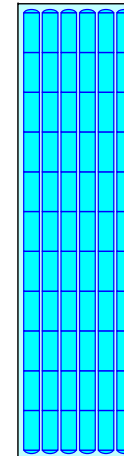
4,089.6 cf Field - 973.0 cf Chambers = 3,116.6 cf Stone x 35.0% Voids = 1,090.8 cf Stone Storage

Chamber Storage + Stone Storage = 2,063.8 cf = 0.047 af

Overall Storage Efficiency = 50.5%

Overall System Size = 81.52' x 21.50' x 2.33'

66 Chambers  
 151.5 cy Field  
 115.4 cy Stone



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

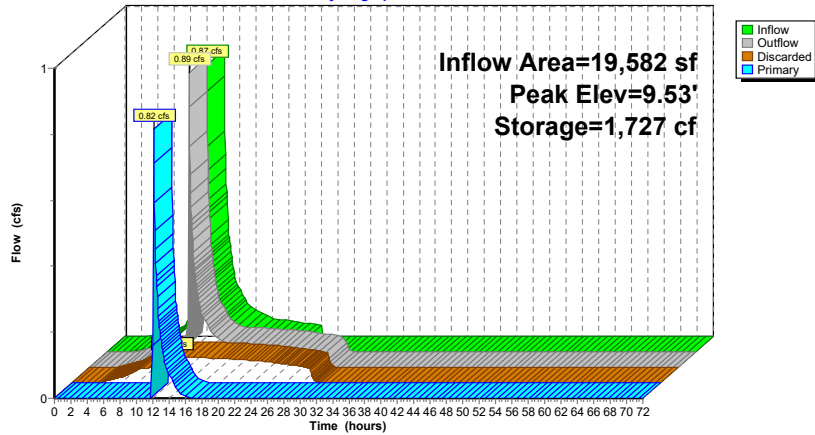
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 199

**Pond INF-1: INFILTRATION SYSTEM #1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 200

**Summary for Pond INF-2: INFILTRATION SYSTEM #2**

Inflow Area = 143,309 sf, 49.69% Impervious, Inflow Depth = 2.95" for NOAA 100-yr event  
 Inflow = 4.74 cfs @ 12.13 hrs, Volume= 35,221 cf  
 Outflow = 3.57 cfs @ 12.19 hrs, Volume= 35,221 cf, Atten= 25%, Lag= 3.8 min  
 Discarded = 0.12 cfs @ 12.19 hrs, Volume= 10,022 cf  
 Primary = 3.44 cfs @ 12.19 hrs, Volume= 25,199 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.13' @ 12.19 hrs Surf.Area= 2,268 sf Storage= 3,861 cf

Plug-Flow detention time= 107.8 min calculated for 35,221 cf (100% of inflow)  
 Center-of-Mass det. time= 107.7 min ( 957.5 - 849.8 )

Volume	Invert	Avail.Storage	Storage Description
#1A	7.50'	1,790 cf	<b>25.25'W x 89.06'L x 3.50'H Field A</b> 7,870 cf Overall - 2,756 cf Embedded = 5,114 cf x 35.0% Voids
#2A	8.00'	2,756 cf	<b>ADS_StormTech SC-740 +Cap</b> x 60 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 60 Chambers in 5 Rows
#3	7.50'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		4,684 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	7.50'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.50'
#2	Primary	8.00'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100'/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.50'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.12 cfs @ 12.19 hrs HW=10.12' (Free Discharge)  
 ↳1=Exfiltration ( Controls 0.12 cfs)

**Primary OutFlow** Max=3.43 cfs @ 12.19 hrs HW=10.12' (Free Discharge)  
 ↳2=Culvert (Inlet Controls 3.43 cfs @ 6.29 fps)  
 ↳3=Sharp-Crested Rectangular Weir(Passes 3.43 cfs of 7.81 cfs potential flow)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 201

**Pond INF-2: INFILTRATION SYSTEM #2 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-740+Cap (ADS StormTech®SC-740 with cap length)**

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf

Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

12 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 87.06' Row Length +12.0" End Stone x 2 = 89.06' Base Length

5 Rows x 51.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.25' Base Width

6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

60 Chambers x 45.9 cf = 2,756.4 cf Chamber Storage

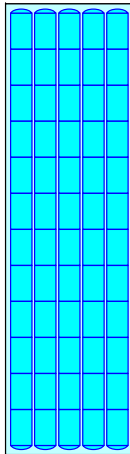
7,870.4 cf Field - 2,756.4 cf Chambers = 5,114.0 cf Stone x 35.0% Voids = 1,789.9 cf Stone Storage

Chamber Storage + Stone Storage = 4,546.3 cf = 0.104 af

Overall Storage Efficiency = 57.8%

Overall System Size = 89.06' x 25.25' x 3.50'

60 Chambers  
291.5 cy Field  
189.4 cy Stone



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

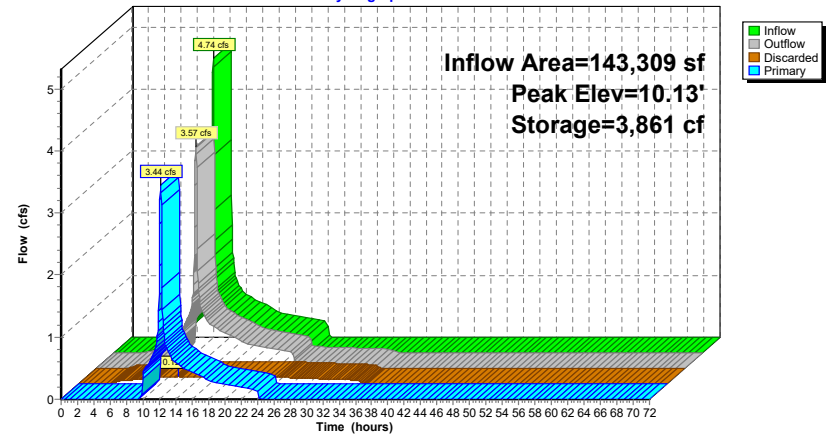
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 202

**Pond INF-2: INFILTRATION SYSTEM #2**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 203

**Summary for Pond INF3: INFILTRATION SYSTEM #1**

Inflow Area = 19,743 sf, 50.83% Impervious, Inflow Depth = 4.70" for NOAA 100-yr event  
 Inflow = 1.44 cfs @ 12.13 hrs, Volume= 7,738 cf  
 Outflow = 1.43 cfs @ 12.14 hrs, Volume= 7,738 cf, Atten= 1%, Lag= 0.9 min  
 Discarded = 0.05 cfs @ 12.14 hrs, Volume= 3,496 cf  
 Primary = 1.39 cfs @ 12.14 hrs, Volume= 4,242 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.69' @ 12.14 hrs Surf.Area= 1,113 sf Storage= 1,012 cf

Plug-Flow detention time= 116.2 min calculated for 7,732 cf (100% of inflow)  
 Center-of-Mass det. time= 116.4 min ( 925.1 - 808.6 )

Volume	Invert	Avail.Storage	Storage Description
#1A	8.10'	686 cf	<b>18.17'W x 60.16'L x 2.33'H Field A</b> 2,550 cf Overall - 590 cf Embedded = 1,960 cf x 35.0% Voids
#2A	8.60'	590 cf	<b>ADS_StormTech SC-310 +Cap</b> x 40 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 40 Chambers in 5 Rows
#3	8.10'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		1,413 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	8.10'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.10'
#2	Primary	8.40'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.40' / 8.30' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.50'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.05 cfs @ 12.14 hrs HW=9.69' (Free Discharge)  
 ↳1=Exfiltration ( Controls 0.05 cfs)

**Primary OutFlow** Max=1.36 cfs @ 12.14 hrs HW=9.69' (Free Discharge)  
 ↳2=Culvert (Passes 1.36 cfs of 2.41 cfs potential flow)  
 ↳3=Sharp-Crested Rectangular Weir(Weir Controls 1.36 cfs @ 1.43 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 204

**Pond INF3: INFILTRATION SYSTEM #1 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-310 +Cap (ADS StormTech®SC-310 with cap length)**

Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf

Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap

34.0" Wide + 6.0" Spacing = 40.0" C-C Row Spacing

8 Chambers/Row x 7.12' Long +0.60' Cap Length x 2 = 58.16' Row Length +12.0" End Stone x 2 = 60.16' Base Length

5 Rows x 34.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 18.17' Base Width

6.0" Stone Base + 16.0" Chamber Height + 6.0" Stone Cover = 2.33' Field Height

40 Chambers x 14.7 cf = 589.7 cf Chamber Storage

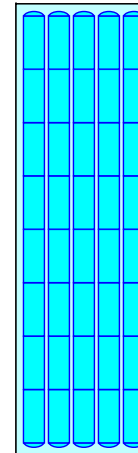
2,550.1 cf Field - 589.7 cf Chambers = 1,960.4 cf Stone x 35.0% Voids = 686.2 cf Stone Storage

Chamber Storage + Stone Storage = 1,275.8 cf = 0.029 af

Overall Storage Efficiency = 50.0%

Overall System Size = 60.16' x 18.17' x 2.33'

40 Chambers  
 94.4 cy Field  
 72.6 cy Stone



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

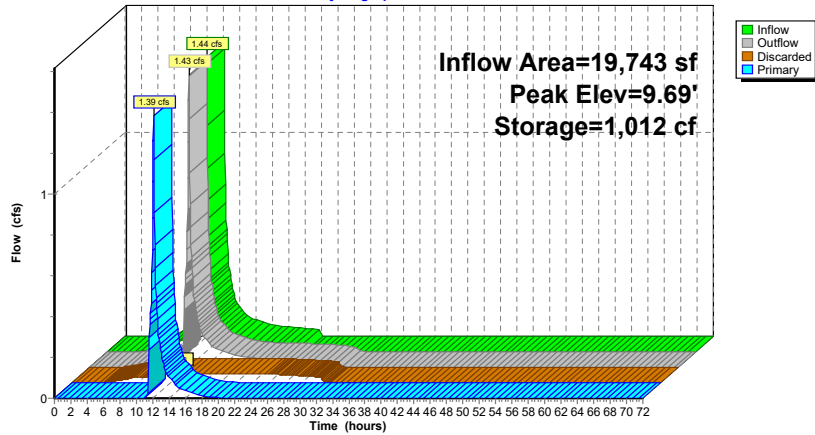
NOAA 24-hr C NOAA 100-yr Rainfall=7.59"

Printed 4/6/2022

Page 205

**Pond INF3: INFILTRATION SYSTEM #1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Printed 4/6/2022

Page 206

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: BB-1</b>	Runoff Area=38,826 sf 51.66% Impervious Runoff Depth=2.01" Tc=6.0 min CN=86 Runoff=2.27 cfs 6,504 cf
<b>Subcatchment2a: BB-2a</b>	Runoff Area=3,116 sf 92.62% Impervious Runoff Depth=2.95" Tc=6.0 min CN=96 Runoff=0.24 cfs 765 cf
<b>Subcatchment2b: BB-2b</b>	Runoff Area=21,490 sf 80.50% Impervious Runoff Depth=2.64" Tc=6.0 min CN=93 Runoff=1.57 cfs 4,725 cf
<b>Subcatchment3A: BB-3A</b>	Runoff Area=10,987 sf 58.16% Impervious Runoff Depth=2.18" Tc=6.0 min CN=88 Runoff=0.69 cfs 1,994 cf
<b>Subcatchment3B: BB-3B</b>	Runoff Area=4,545 sf 77.34% Impervious Runoff Depth=2.64" Tc=6.0 min CN=93 Runoff=0.33 cfs 999 cf
<b>Subcatchment4A: BB-4A</b>	Runoff Area=4,843 sf 86.37% Impervious Runoff Depth=2.84" Tc=6.0 min CN=95 Runoff=0.37 cfs 1,147 cf
<b>Subcatchment4B: BB-4B</b>	Runoff Area=3,048 sf 86.09% Impervious Runoff Depth=2.84" Tc=6.0 min CN=95 Runoff=0.23 cfs 722 cf
<b>Subcatchment5A: BB-5A</b>	Runoff Area=3,072 sf 73.44% Impervious Runoff Depth=2.54" Tc=6.0 min CN=92 Runoff=0.22 cfs 651 cf
<b>Subcatchment5B: BB-5B</b>	Runoff Area=34,755 sf 71.39% Impervious Runoff Depth=2.45" Tc=6.0 min CN=91 Runoff=2.40 cfs 7,086 cf
<b>Subcatchment6A: BB-6A</b>	Runoff Area=15,148 sf 46.97% Impervious Runoff Depth=1.93" Tc=6.0 min CN=85 Runoff=0.85 cfs 2,436 cf
<b>Subcatchment6B: BB-6B</b>	Runoff Area=6,495 sf 77.45% Impervious Runoff Depth=2.64" Tc=6.0 min CN=93 Runoff=0.47 cfs 1,428 cf
<b>Subcatchment7A: BB-7A</b>	Runoff Area=3,165 sf 87.74% Impervious Runoff Depth=2.84" Tc=6.0 min CN=95 Runoff=0.24 cfs 749 cf
<b>Subcatchment7B: BB-7B</b>	Runoff Area=4,942 sf 88.73% Impervious Runoff Depth=2.84" Tc=6.0 min CN=95 Runoff=0.38 cfs 1,170 cf
<b>Subcatchment8A: BB-8A</b>	Runoff Area=3,978 sf 79.99% Impervious Runoff Depth=2.64" Tc=6.0 min CN=93 Runoff=0.29 cfs 875 cf
<b>Subcatchment8B: BB-8B</b>	Runoff Area=5,598 sf 87.78% Impervious Runoff Depth=2.84" Tc=6.0 min CN=95 Runoff=0.43 cfs 1,325 cf
<b>Subcatchment9: BB-9</b>	Runoff Area=29,651 sf 74.77% Impervious Runoff Depth=2.54" Tc=6.0 min CN=92 Runoff=2.10 cfs 6,279 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 207

<b>SubcatchmentCB-1: New CB South</b>	Runoff Area=19,582 sf 58.17% Impervious Runoff Depth=2.18" Flow Length=512' Tc=6.0 min CN=88 Runoff=1.23 cfs 3,554 cf
<b>SubcatchmentCB-5: PORTLANDST</b>	Runoff Area=19,743 sf 50.83% Impervious Runoff Depth=2.01" Flow Length=574' Tc=6.0 min CN=86 Runoff=1.15 cfs 3,307 cf
<b>SubcatchmentCB3: NEW CB SOUTH-</b>	Runoff Area=25,183 sf 51.84% Impervious Runoff Depth=2.01" Flow Length=635' Tc=6.0 min CN=86 Runoff=1.47 cfs 4,218 cf
<b>SubcatchmentCB4: NEW CB NOTH -</b>	Runoff Area=118,126 sf 49.24% Impervious Runoff Depth=2.01" Flow Length=822' Tc=6.0 min CN=86 Runoff=6.90 cfs 19,787 cf
<b>Reach 1R: ISOLATORROW C</b>	Inflow=2.00 cfs 4,348 cf Outflow=2.00 cfs 4,348 cf
<b>Reach 6R: ISOLATORROW 2</b>	Inflow=1.70 cfs 3,926 cf Outflow=1.70 cfs 3,926 cf
<b>Reach 15R: ISOLATORROW 1</b>	Inflow=3.23 cfs 9,271 cf Outflow=3.23 cfs 9,271 cf
<b>Reach B: PARKINGLOT B OVERFLOW</b>	Inflow=11.55 cfs 26,345 cf Outflow=11.55 cfs 26,345 cf
<b>Reach BMP4_O: BMP-4 OVERFLOW</b>	Inflow=0.26 cfs 1,561 cf Outflow=0.26 cfs 1,561 cf
<b>Reach BMP6_O: BMP-6 OVERFLOW</b>	Inflow=1.24 cfs 2,992 cf Outflow=1.24 cfs 2,992 cf
<b>Reach BMP7_O: BMP-7 OVERFLOW</b>	Inflow=0.60 cfs 1,919 cf Outflow=0.60 cfs 1,919 cf
<b>Reach BMP9_O: BMP-9 OVERFLOW</b>	Inflow=2.05 cfs 6,279 cf Outflow=2.05 cfs 6,279 cf
<b>Reach BMP_3: BMP-3_OVERFLOW</b>	Inflow=1.26 cfs 1,596 cf Outflow=1.26 cfs 1,596 cf
<b>Reach DP-1: French Rodney Blvd 14" Outfall</b>	Inflow=4.53 cfs 10,213 cf Outflow=4.53 cfs 10,213 cf
<b>Reach DP-2: NORTHERNOUTFALL</b>	Inflow=16.27 cfs 40,704 cf Outflow=16.27 cfs 40,704 cf
<b>Reach H ST: HUDSON STREET DRAINAGE</b>	Inflow=14.02 cfs 33,949 cf Outflow=14.02 cfs 33,949 cf
<b>Reach P ST: PORTLANDSTREET DRAINAGE</b>	Inflow=2.26 cfs 6,755 cf Outflow=2.26 cfs 6,755 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 208

<b>Pond 1-P: BB 1</b>	Peak Elev=10.08' Storage=900 cf Inflow=2.27 cfs 6,504 cf Discarded=0.05 cfs 2,289 cf Primary=0.66 cfs 362 cf Secondary=1.47 cfs 3,853 cf Outflow=2.18 cfs 6,504 cf
<b>Pond 2a-P: BB 2a</b>	Peak Elev=8.17' Storage=66 cf Inflow=0.24 cfs 765 cf Primary=0.23 cfs 729 cf Secondary=0.00 cfs 0 cf Outflow=0.23 cfs 729 cf
<b>Pond 2b-P: BB 2b</b>	Peak Elev=8.35' Storage=161 cf Inflow=1.57 cfs 4,725 cf Primary=1.53 cfs 4,689 cf Secondary=0.00 cfs 0 cf Outflow=1.53 cfs 4,689 cf
<b>Pond 3A-P: BB 3A</b>	Peak Elev=11.02' Storage=405 cf Inflow=0.69 cfs 1,994 cf Discarded=0.02 cfs 1,062 cf Primary=0.63 cfs 932 cf Outflow=0.65 cfs 1,994 cf
<b>Pond 3B-P: BB 3B</b>	Peak Elev=12.87' Storage=244 cf Inflow=0.33 cfs 999 cf Discarded=0.01 cfs 642 cf Primary=0.31 cfs 358 cf Outflow=0.33 cfs 999 cf
<b>Pond 4A-P: BB 4A - POND</b>	Peak Elev=10.02' Storage=212 cf Inflow=0.37 cfs 1,147 cf Primary=0.32 cfs 307 cf Secondary=0.03 cfs 839 cf Outflow=0.35 cfs 1,147 cf
<b>Pond 4A-S: BB4A-Stone</b>	Peak Elev=6.12' Storage=8 cf Inflow=0.03 cfs 839 cf Outflow=0.03 cfs 839 cf
<b>Pond 4B-P: BB 4B - POND</b>	Peak Elev=11.00' Storage=124 cf Inflow=0.23 cfs 722 cf Primary=0.21 cfs 199 cf Secondary=0.02 cfs 522 cf Outflow=0.23 cfs 722 cf
<b>Pond 4B-S: BB 4A-Stone</b>	Peak Elev=6.09' Storage=4 cf Inflow=0.02 cfs 522 cf Outflow=0.02 cfs 522 cf
<b>Pond 5A-P: BB 5A - POND</b>	Peak Elev=9.16' Storage=195 cf Inflow=0.22 cfs 651 cf Primary=0.00 cfs 0 cf Secondary=0.04 cfs 651 cf Outflow=0.04 cfs 651 cf
<b>Pond 5A-PS: BB 5A-Stone</b>	Peak Elev=6.13' Storage=18 cf Inflow=0.04 cfs 651 cf Outflow=0.04 cfs 651 cf
<b>Pond 5B-P: BB 5B - POND</b>	Peak Elev=8.97' Storage=585 cf Inflow=2.40 cfs 7,086 cf Primary=0.59 cfs 277 cf Secondary=0.08 cfs 2,882 cf Tertiary=1.70 cfs 3,926 cf Outflow=2.36 cfs 7,086 cf
<b>Pond 5B-PS: BB 5B-Stone</b>	Peak Elev=6.00' Storage=1 cf Inflow=0.08 cfs 2,882 cf Outflow=0.08 cfs 2,882 cf
<b>Pond 6A-P: BB 6A - POND</b>	Peak Elev=10.94' Storage=378 cf Inflow=0.85 cfs 2,436 cf Primary=0.77 cfs 996 cf Secondary=0.04 cfs 1,440 cf Outflow=0.82 cfs 2,436 cf
<b>Pond 6A-PS: BB 6A - STONE</b>	Peak Elev=6.14' Storage=12 cf Inflow=0.04 cfs 1,440 cf Outflow=0.04 cfs 1,440 cf
<b>Pond 6B-P: BB 6B</b>	Peak Elev=12.04' Storage=356 cf Inflow=0.47 cfs 1,428 cf Discarded=0.02 cfs 871 cf Primary=0.43 cfs 557 cf Outflow=0.44 cfs 1,428 cf
<b>Pond 7A-P: BB 7A PONDING</b>	Peak Elev=9.96' Storage=141 cf Inflow=0.24 cfs 749 cf Primary=0.22 cfs 210 cf Secondary=0.02 cfs 539 cf Outflow=0.24 cfs 749 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 209

**Pond 7A-S: BB 7A - STONE** Peak Elev=5.19' Storage=4 cf Inflow=0.02 cfs 539 cf  
Outflow=0.02 cfs 539 cf

**Pond 7B-P: BB 7B PONDING** Peak Elev=10.68' Storage=258 cf Inflow=0.38 cfs 1,170 cf  
Primary=0.33 cfs 283 cf Secondary=0.03 cfs 887 cf Outflow=0.37 cfs 1,170 cf

**Pond 7B-S: BB 7B - STONE** Peak Elev=5.22' Storage=5 cf Inflow=0.03 cfs 887 cf  
Outflow=0.03 cfs 887 cf

**Pond 8a-P: BB 8A PONDING** Peak Elev=9.03' Storage=250 cf Inflow=0.29 cfs 875 cf  
Primary=0.10 cfs 79 cf Secondary=0.04 cfs 795 cf Outflow=0.14 cfs 875 cf

**Pond 8a-s: BB 8A - STONE** Peak Elev=4.53' Storage=11 cf Inflow=0.04 cfs 795 cf  
Outflow=0.04 cfs 795 cf

**Pond 8B-P: BB 8B-PONDING** Peak Elev=9.70' Storage=242 cf Inflow=0.43 cfs 1,325 cf  
Primary=0.38 cfs 364 cf Secondary=0.04 cfs 962 cf Outflow=0.41 cfs 1,325 cf

**Pond 8B-S: BB 8B-Stone** Peak Elev=4.53' Storage=12 cf Inflow=0.04 cfs 962 cf  
Outflow=0.04 cfs 962 cf

**Pond 9-P: BB9 - POND** Peak Elev=8.80' Storage=343 cf Inflow=2.10 cfs 6,279 cf  
Primary=0.01 cfs 2 cf Secondary=0.04 cfs 1,929 cf Tertiary=2.00 cfs 4,348 cf Outflow=2.05 cfs 6,279 cf

**Pond 9-PS: BB9 - STONE** Peak Elev=6.14' Storage=8 cf Inflow=0.04 cfs 1,929 cf  
Outflow=0.04 cfs 1,929 cf

**Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST** Peak Elev=9.61' Inflow=1.23 cfs 3,554 cf  
Primary=0.57 cfs 2,974 cf Secondary=0.66 cfs 580 cf Outflow=1.23 cfs 3,554 cf

**Pond DMH2: DIVERSION MANHOLE - HUDSON STREET** Peak Elev=14.56' Inflow=8.38 cfs 24,005 cf  
Primary=1.83 cfs 15,418 cf Secondary=6.55 cfs 8,588 cf Outflow=8.38 cfs 24,005 cf

**Pond DMH3: DIVERSION MANHOLE - PORTLAND ST** Peak Elev=11.54' Inflow=1.15 cfs 3,307 cf  
Primary=0.75 cfs 2,972 cf Secondary=0.40 cfs 335 cf Outflow=1.15 cfs 3,307 cf

**Pond INF-1: INFILTRATIONSYSTEM #1** Peak Elev=9.01' Storage=1,232 cf Inflow=0.57 cfs 2,974 cf  
Discarded=0.07 cfs 2,974 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 2,974 cf

**Pond INF-2: INFILTRATIONSYSTEM #2** Peak Elev=9.70' Storage=3,280 cf Inflow=1.83 cfs 15,418 cf  
Discarded=0.11 cfs 8,554 cf Primary=1.42 cfs 6,864 cf Outflow=1.53 cfs 15,418 cf

**Pond INF3: INFILTRATIONSYSTEM #1** Peak Elev=9.57' Storage=944 cf Inflow=0.75 cfs 2,972 cf  
Discarded=0.05 cfs 2,339 cf Primary=0.33 cfs 633 cf Outflow=0.38 cfs 2,972 cf

**Total Runoff Area = 376,293 sf Runoff Volume = 69,719 cf Average Runoff Depth = 2.22"**  
**39.88% Pervious = 150,053 sf 60.12% Impervious = 226,240 sf**

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 210

**Summary for Subcatchment 1: BB-1**

Runoff = 2.27 cfs @ 12.13 hrs, Volume= 6,504 cf, Depth= 2.01"  
Routed to Pond 1-P : BB 1

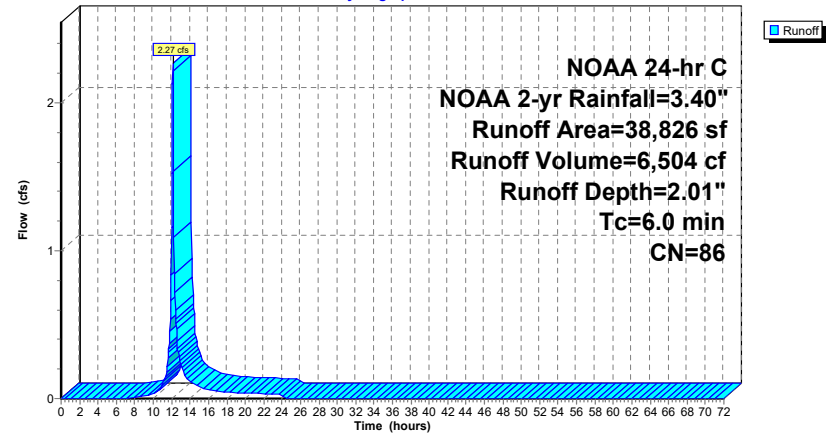
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
27,309	83	1/4 acre lots, 38% imp, HSG C
1,838	74	>75% Grass cover, Good, HSG C
9,679	98	Paved parking, HSG C
38,826	86	Weighted Average
18,770		48.34% Pervious Area
20,056		51.66% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 1: BB-1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 211

**Summary for Subcatchment 2a: BB-2a**

Runoff = 0.24 cfs @ 12.13 hrs, Volume= 765 cf, Depth= 2.95"  
Routed to Pond 2a-P : BB 2a

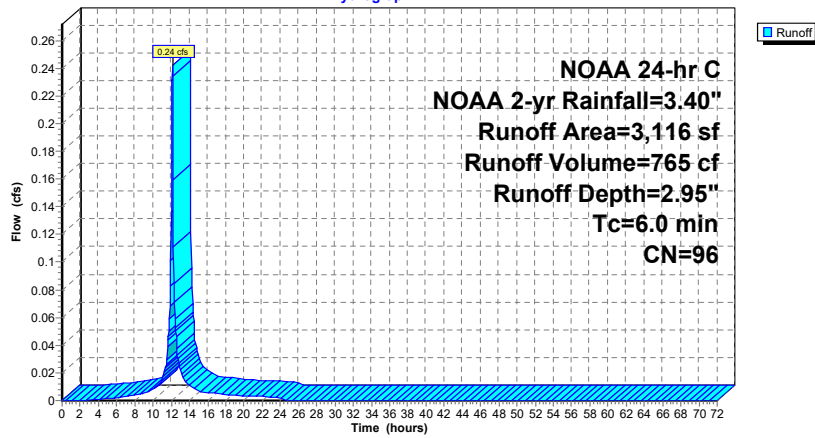
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
230	74	>75% Grass cover, Good, HSG C
2,886	98	Paved parking, HSG C
3,116	96	Weighted Average
230		7.38% Pervious Area
2,886		92.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2a: BB-2a**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 212

**Summary for Subcatchment 2b: BB-2b**

Runoff = 1.57 cfs @ 12.13 hrs, Volume= 4,725 cf, Depth= 2.64"  
Routed to Pond 2b-P : BB 2b

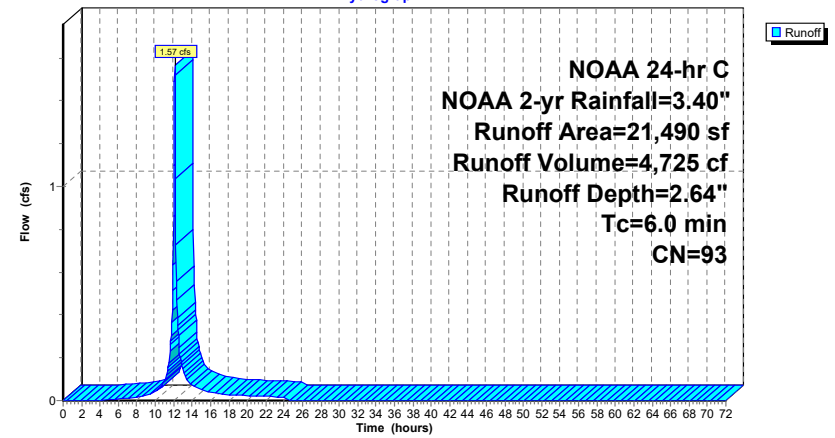
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
3,097	83	1/4 acre lots, 38% imp, HSG C
2,270	74	>75% Grass cover, Good, HSG C
16,123	98	Paved parking, HSG C
21,490	93	Weighted Average
4,190		19.50% Pervious Area
17,300		80.50% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2b: BB-2b**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 213

**Summary for Subcatchment 3A: BB-3A**

Runoff = 0.69 cfs @ 12.13 hrs, Volume= 1,994 cf, Depth= 2.18"  
Routed to Pond 3A-P : BB 3A

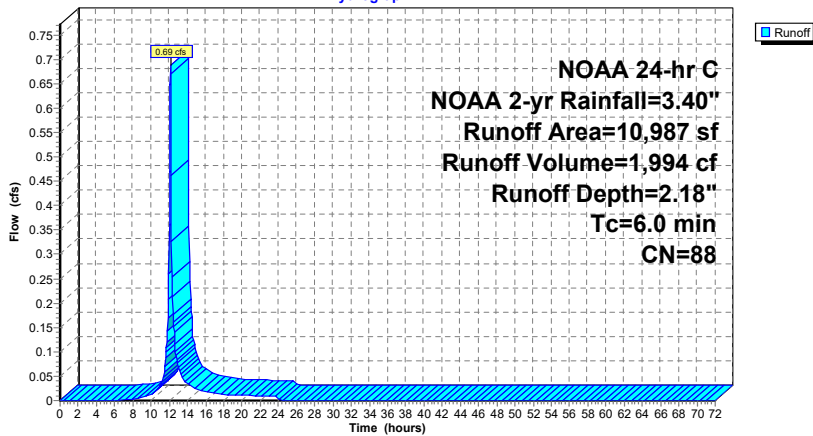
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
5,791	83	1/4 acre lots, 38% imp, HSG C
1,007	74	>75% Grass cover, Good, HSG C
4,189	98	Paved parking, HSG C
10,987	88	Weighted Average
4,597		41.84% Pervious Area
6,390		58.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3A: BB-3A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 214

**Summary for Subcatchment 3B: BB-3B**

Runoff = 0.33 cfs @ 12.13 hrs, Volume= 999 cf, Depth= 2.64"  
Routed to Pond 3B-P : BB 3B

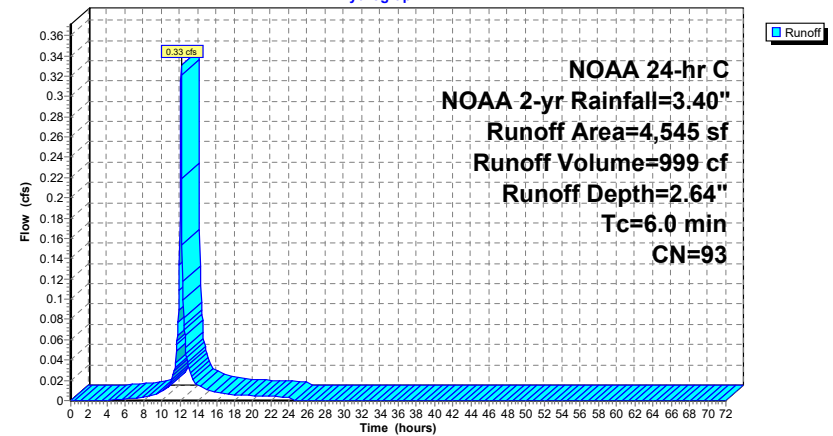
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
* 1,030	74	>75% Grass cover, Good, HSG C
3,515	98	Paved parking, HSG C
4,545	93	Weighted Average
1,030		22.66% Pervious Area
3,515		77.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3B: BB-3B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 215

**Summary for Subcatchment 4A: BB-4A**

Runoff = 0.37 cfs @ 12.13 hrs, Volume= 1,147 cf, Depth= 2.84"  
Routed to Pond 4A-P : BB 4A - POND

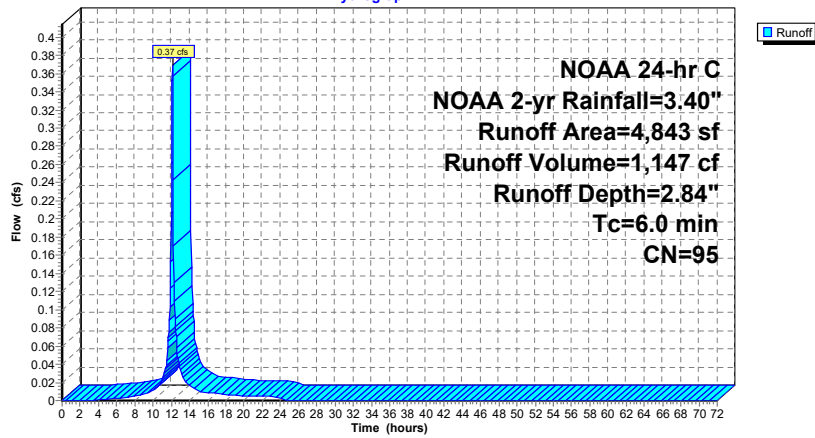
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
660	74	>75% Grass cover, Good, HSG C
4,183	98	Paved parking, HSG C
4,843	95	Weighted Average
660		13.63% Pervious Area
4,183		86.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 4A: BB-4A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 216

**Summary for Subcatchment 4B: BB-4B**

Runoff = 0.23 cfs @ 12.13 hrs, Volume= 722 cf, Depth= 2.84"  
Routed to Pond 4B-P : BB 4B - POND

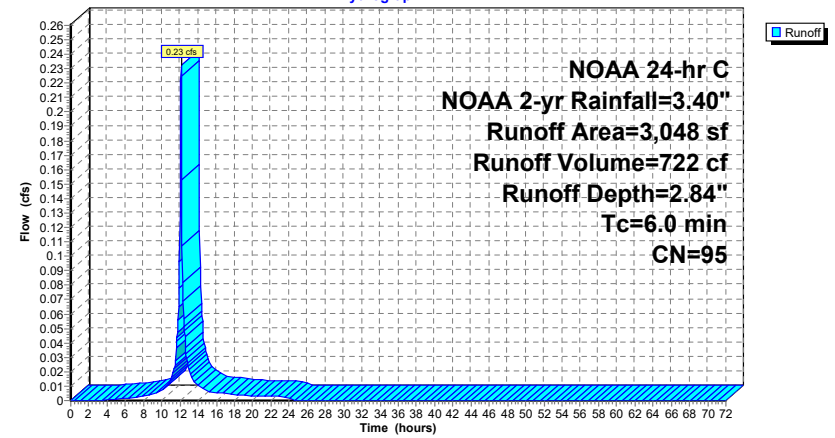
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
424	74	>75% Grass cover, Good, HSG C
2,624	98	Paved parking, HSG C
3,048	95	Weighted Average
424		13.91% Pervious Area
2,624		86.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 4B: BB-4B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 217

**Summary for Subcatchment 5A: BB-5A**

Runoff = 0.22 cfs @ 12.13 hrs, Volume= 651 cf, Depth= 2.54"  
 Routed to Pond 5A-P : BB 5A - POND

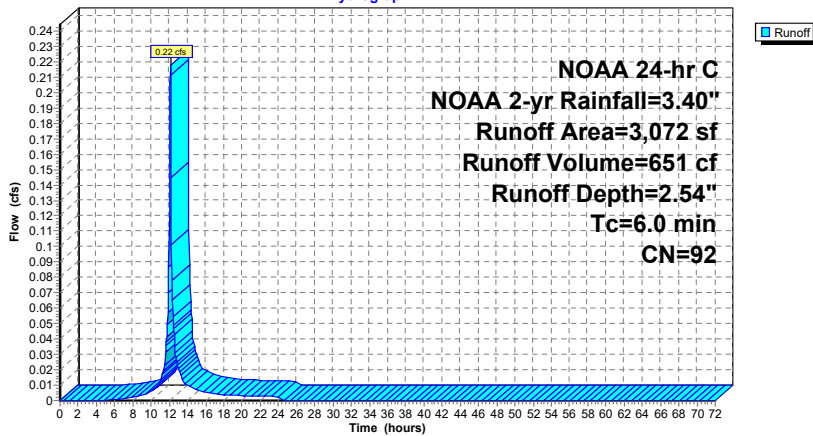
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
816	74	>75% Grass cover, Good, HSG C
2,256	98	Paved parking, HSG C
3,072	92	Weighted Average
816		26.56% Pervious Area
2,256		73.44% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5A: BB-5A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 218

**Summary for Subcatchment 5B: BB-5B**

Runoff = 2.40 cfs @ 12.13 hrs, Volume= 7,086 cf, Depth= 2.45"  
 Routed to Pond 5B-P : BB 5B - POND

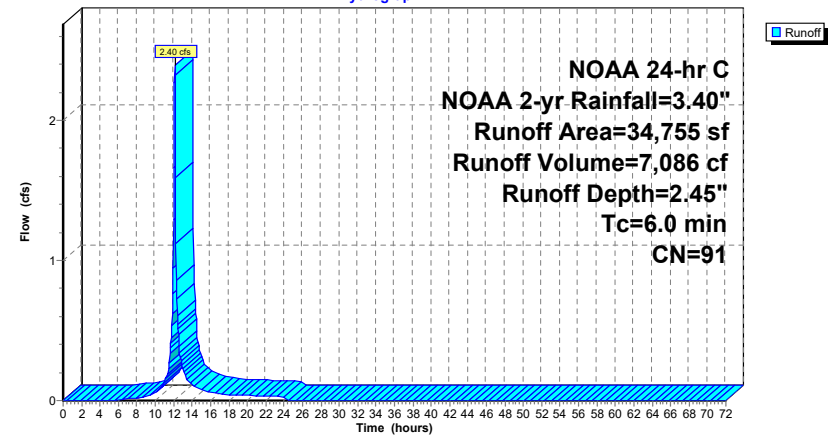
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
12,062	83	1/4 acre lots, 38% imp, HSG C
2,464	74	>75% Grass cover, Good, HSG C
20,229	98	Paved parking, HSG C
34,755	91	Weighted Average
9,942		28.61% Pervious Area
24,813		71.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5B: BB-5B**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 219

**Summary for Subcatchment 6A: BB-6A**

Runoff = 0.85 cfs @ 12.13 hrs, Volume= 2,436 cf, Depth= 1.93"  
Routed to Pond 6A-P : BB 6A - POND

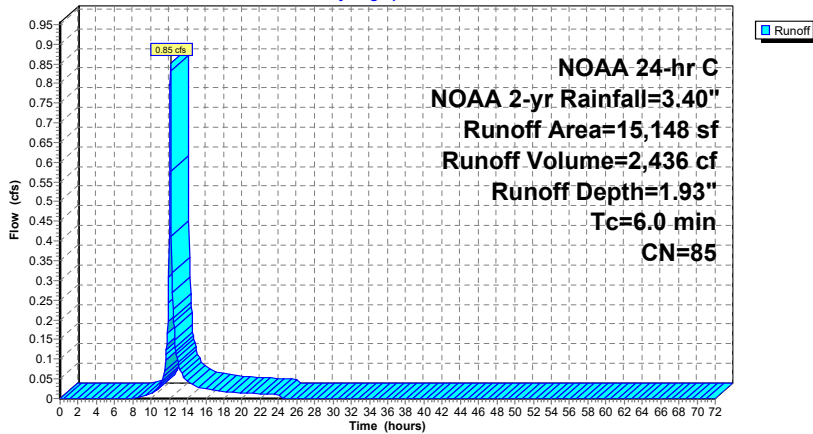
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
11,763	83	1/4 acre lots, 38% imp, HSG C
740	74	>75% Grass cover, Good, HSG C
2,645	98	Paved parking, HSG C
15,148	85	Weighted Average
8,033		53.03% Pervious Area
7,115		46.97% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6A: BB-6A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 220

**Summary for Subcatchment 6B: BB-6B**

Runoff = 0.47 cfs @ 12.13 hrs, Volume= 1,428 cf, Depth= 2.64"  
Routed to Pond 6B-P : BB 6B

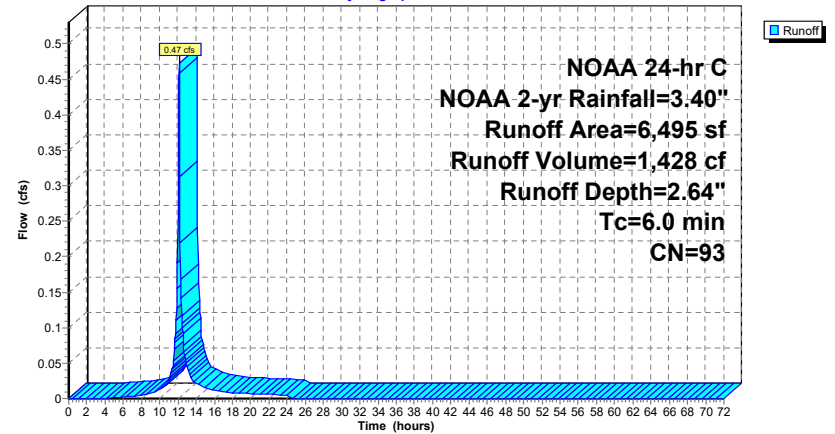
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
1,259	83	1/4 acre lots, 38% imp, HSG C
684	74	>75% Grass cover, Good, HSG C
4,552	98	Paved parking, HSG C
6,495	93	Weighted Average
1,465		22.55% Pervious Area
5,030		77.45% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6B: BB-6B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 221

**Summary for Subcatchment 7A: BB-7A**

Runoff = 0.24 cfs @ 12.13 hrs, Volume= 749 cf, Depth= 2.84"  
 Routed to Pond 7A-P : BB 7A PONDING

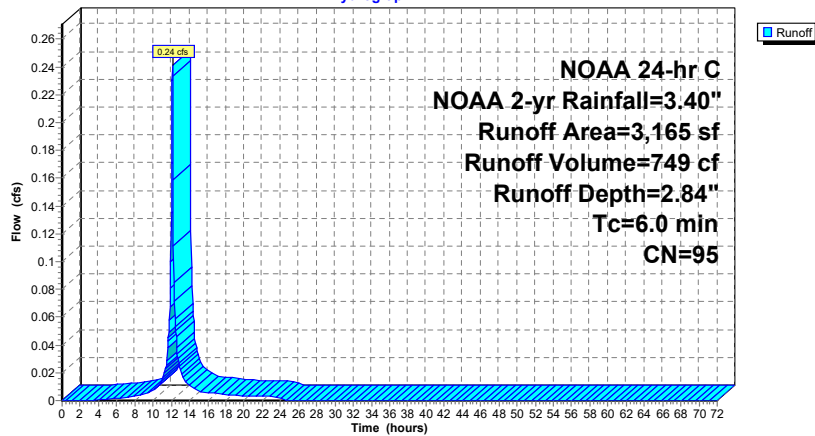
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
388	74	>75% Grass cover, Good, HSG C
2,777	98	Paved parking, HSG C
3,165	95	Weighted Average
388		12.26% Pervious Area
2,777		87.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 7A: BB-7A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 222

**Summary for Subcatchment 7B: BB-7B**

Runoff = 0.38 cfs @ 12.13 hrs, Volume= 1,170 cf, Depth= 2.84"  
 Routed to Pond 7B-P : BB 7B PONDING

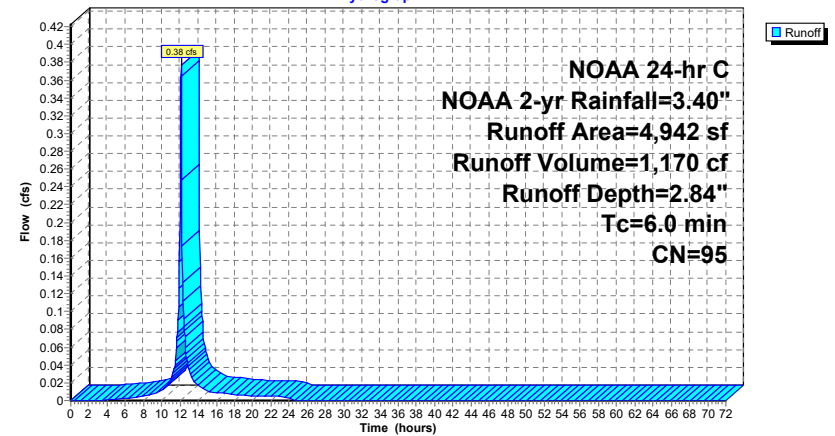
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
557	74	>75% Grass cover, Good, HSG C
4,385	98	Paved parking, HSG C
4,942	95	Weighted Average
557		11.27% Pervious Area
4,385		88.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 7B: BB-7B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 223

**Summary for Subcatchment 8A: BB-8A**

Runoff = 0.29 cfs @ 12.13 hrs, Volume= 875 cf, Depth= 2.64"  
Routed to Pond 8a-P : BB 8A PONDING

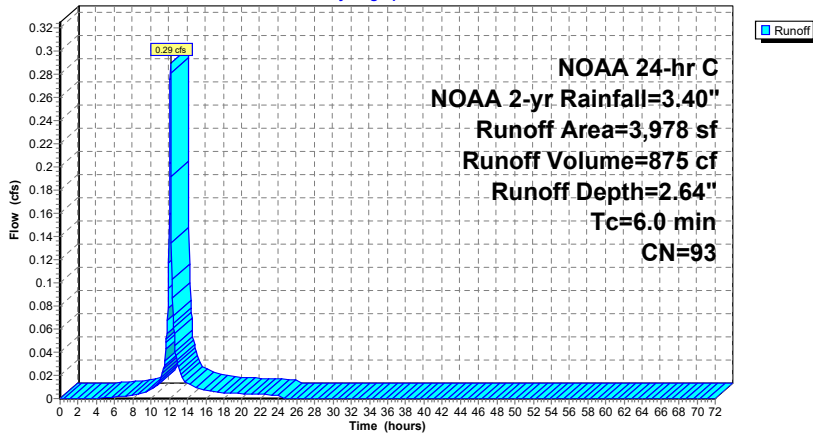
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
796	74	>75% Grass cover, Good, HSG C
3,182	98	Paved parking, HSG C
3,978	93	Weighted Average
796		20.01% Pervious Area
3,182		79.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8A: BB-8A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 224

**Summary for Subcatchment 8B: BB-8B**

Runoff = 0.43 cfs @ 12.13 hrs, Volume= 1,325 cf, Depth= 2.84"  
Routed to Pond 8B-P : BB 8B-PONDING

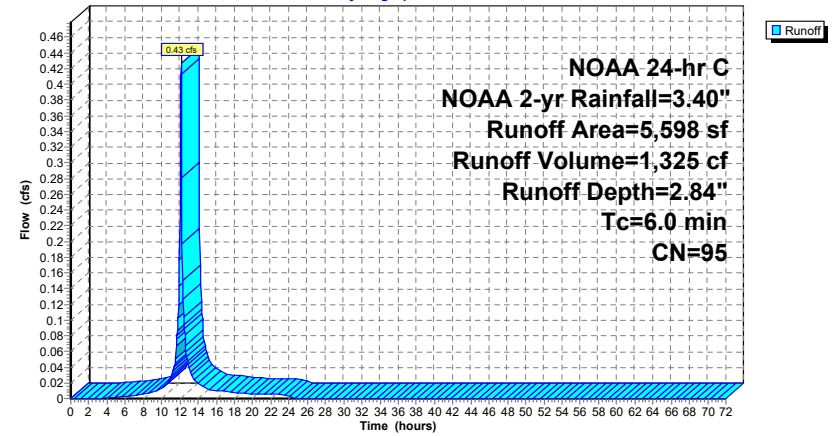
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
684	74	>75% Grass cover, Good, HSG C
4,914	98	Paved parking, HSG C
5,598	95	Weighted Average
684		12.22% Pervious Area
4,914		87.78% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8B: BB-8B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 225

**Summary for Subcatchment 9: BB-9**

Runoff = 2.10 cfs @ 12.13 hrs, Volume= 6,279 cf, Depth= 2.54"  
Routed to Pond 9-P : BB9 - POND

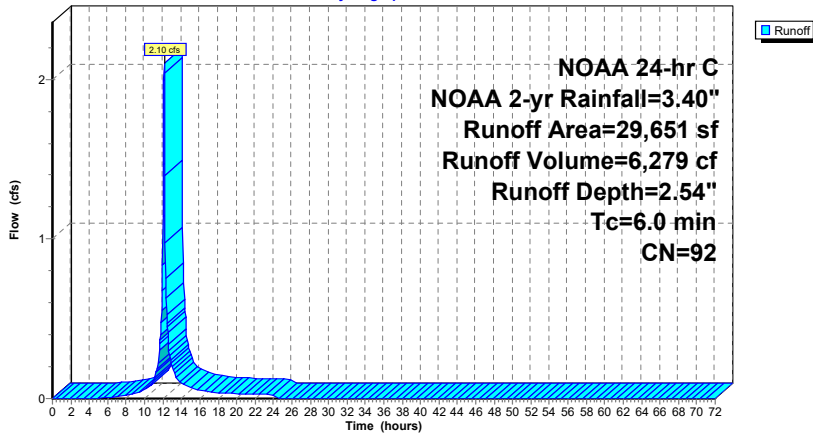
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
8,550	83	1/4 acre lots, 38% imp, HSG C
2,179	74	>75% Grass cover, Good, HSG C
18,922	98	Paved parking, HSG C
29,651	92	Weighted Average
7,480		25.23% Pervious Area
22,171		74.77% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 9: BB-9**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 226

**Summary for Subcatchment CB-1: New CB South (Seymour Street)**

Runoff = 1.23 cfs @ 12.13 hrs, Volume= 3,554 cf, Depth= 2.18"  
Routed to Pond DMH1 : DIVERSION MANHOLE - SEYMOUR ST

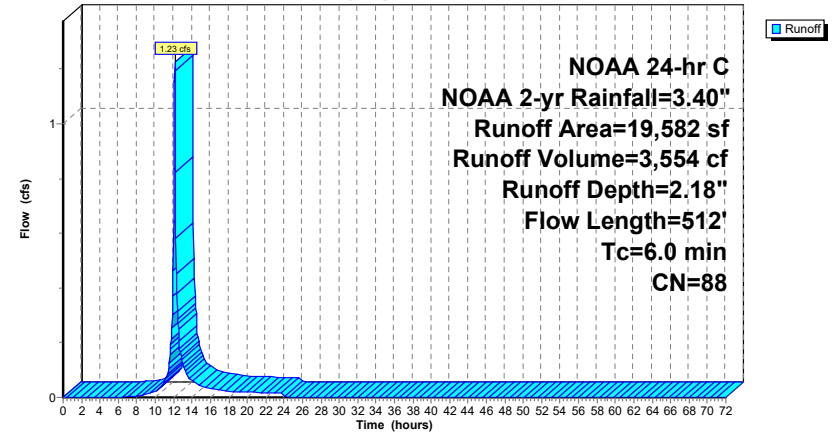
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
13,211	83	1/4 acre lots, 38% imp, HSG C
6,371	98	Roadway
19,582	88	Weighted Average
8,191		41.83% Pervious Area
11,391		58.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	50	0.0300	1.45		Sheet Flow, A-B Smooth surfaces n= 0.011 P2= 3.40"
2.4	462	0.0249	3.20		Shallow Concentrated Flow, Paved Paved Kv= 20.3 fps
3.0					Direct Entry, Direct entry to 6
6.0	512				Total

**Subcatchment CB-1: New CB South (Seymour Street)**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 227

**Summary for Subcatchment CB-5: PORTLAND ST SOUTH OFFSITE**

Runoff = 1.15 cfs @ 12.13 hrs, Volume= 3,307 cf, Depth= 2.01"  
 Routed to Pond DMH3 : DIVERSION MANHOLE - PORTLAND ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

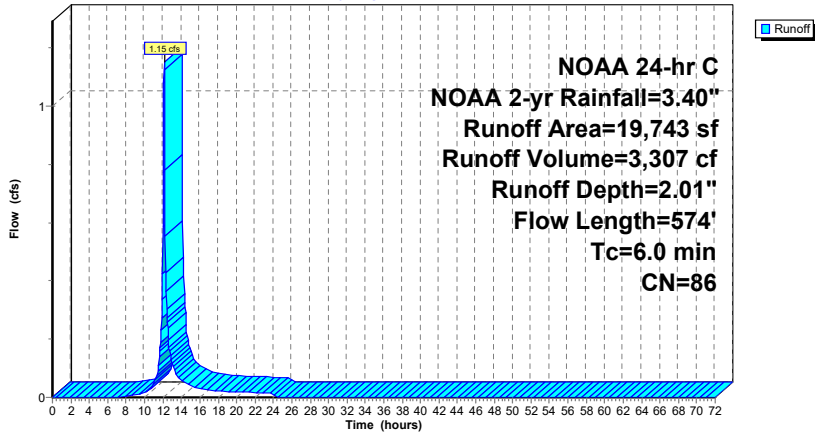
Area (sf)	CN	Description
15,657	83	1/4 acre lots, 38% imp, HSG C
* 4,086	98	Roadway
19,743	86	Weighted Average
9,707		49.17% Pervious Area
10,036		50.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
2.3	524	0.0346	3.78		Shallow Concentrated Flow, B-C (shallow conc.) Paved Kv= 20.3 fps
3.2					Direct Entry, direct to 6
6.0	574	Total			

**Subcatchment CB-5: PORTLAND ST SOUTH OFFSITE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 228

**Summary for Subcatchment CB3: NEW CB SOUTH- HUDSON ST**

Runoff = 1.47 cfs @ 12.13 hrs, Volume= 4,218 cf, Depth= 2.01"  
 Routed to Pond DMH2 : DIVERSION MANHOLE - HUDSON STREET

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

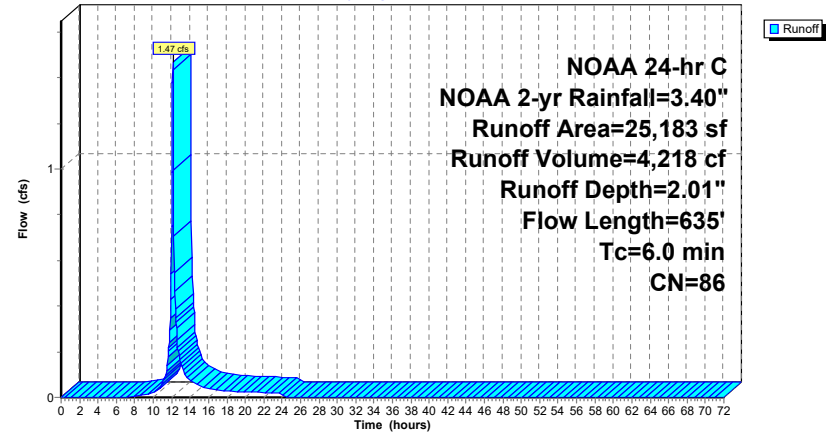
Area (sf)	CN	Description
19,562	83	1/4 acre lots, 38% imp, HSG C
* 5,621	98	Roadway
25,183	86	Weighted Average
12,128		48.16% Pervious Area
13,055		51.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0444	1.70		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
3.0	585	0.0256	3.25		Shallow Concentrated Flow, B-C Paved Kv= 20.3 fps
2.5					Direct Entry, direct entry to 6
6.0	635	Total			

**Subcatchment CB3: NEW CB SOUTH- HUDSON ST**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 229

**Summary for Subcatchment CB4: NEW CB NOTH - HUDSON STREET**

Runoff = 6.90 cfs @ 12.13 hrs, Volume= 19,787 cf, Depth= 2.01"  
 Routed to Pond DMH2 : DIVERSION MANHOLE - HUDSON STREET

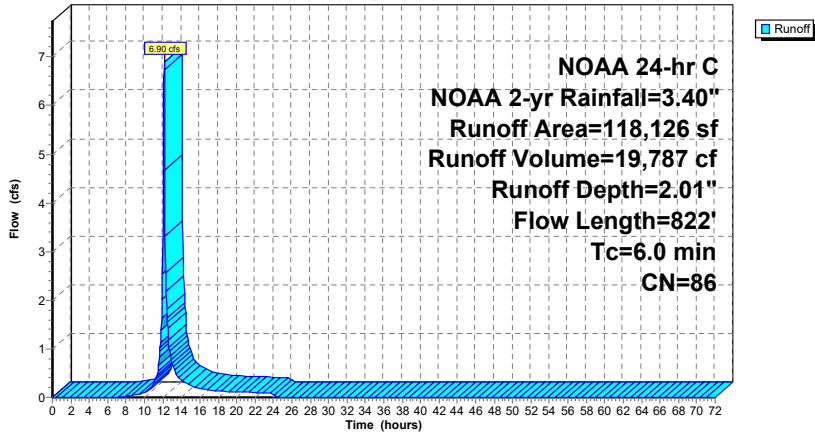
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Area (sf)	CN	Description
96,716	83	1/4 acre lots, 38% imp, HSG C
* 21,410	98	Roadway
118,126	86	Weighted Average
59,964		50.76% Pervious Area
58,162		49.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
4.0	772	0.0245	3.18		Shallow Concentrated Flow, B-C (shallow concentrated) Paved Kv= 20.3 fps
1.5					Direct Entry, direct entry to 6
6.0	822	Total			

**Subcatchment CB4: NEW CB NOTH - HUDSON STREET**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 230

**Summary for Reach 1R: ISOLATOR ROW C**

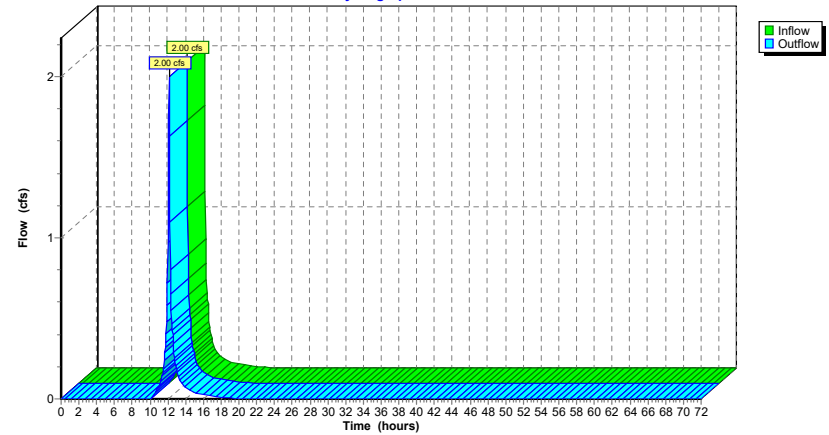
[40] Hint: Not Described (Outflow=Inflow)

Inflow = 2.00 cfs @ 12.15 hrs, Volume= 4,348 cf  
 Outflow = 2.00 cfs @ 12.15 hrs, Volume= 4,348 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 1R: ISOLATOR ROW C**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 231

**Summary for Reach 6R: ISOLATOR ROW 2**

[40] Hint: Not Described (Outflow=Inflow)

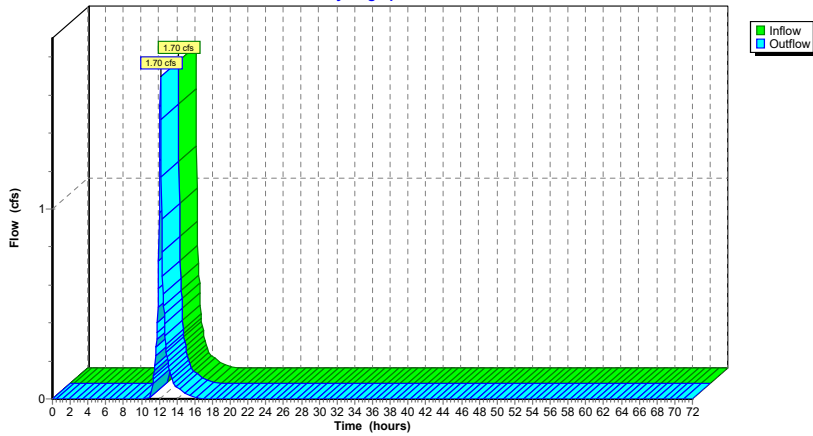
Inflow	=	1.70 cfs @ 12.15 hrs,	Volume=	3,926 cf
Outflow	=	1.70 cfs @ 12.15 hrs,	Volume=	3,926 cf, Atten= 0%, Lag= 0.0 min

Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 6R: ISOLATOR ROW 2**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 232

**Summary for Reach 15R: ISOLATOR ROW 1**

[40] Hint: Not Described (Outflow=Inflow)

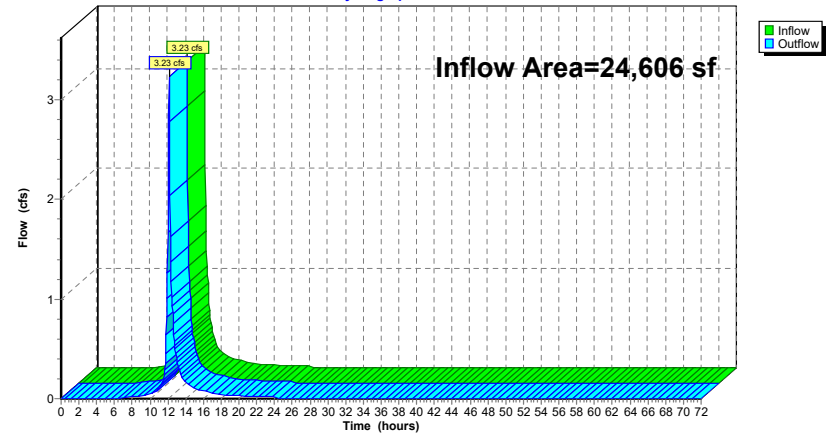
Inflow Area	=	24,606 sf, 82.04% Impervious,	Inflow Depth =	4.52" for NOAA 2-yr event
Inflow	=	3.23 cfs @ 12.15 hrs,	Volume=	9,271 cf
Outflow	=	3.23 cfs @ 12.15 hrs,	Volume=	9,271 cf, Atten= 0%, Lag= 0.0 min

Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 15R: ISOLATOR ROW 1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 233

**Summary for Reach B: PARKING LOT B OVERFLOW**

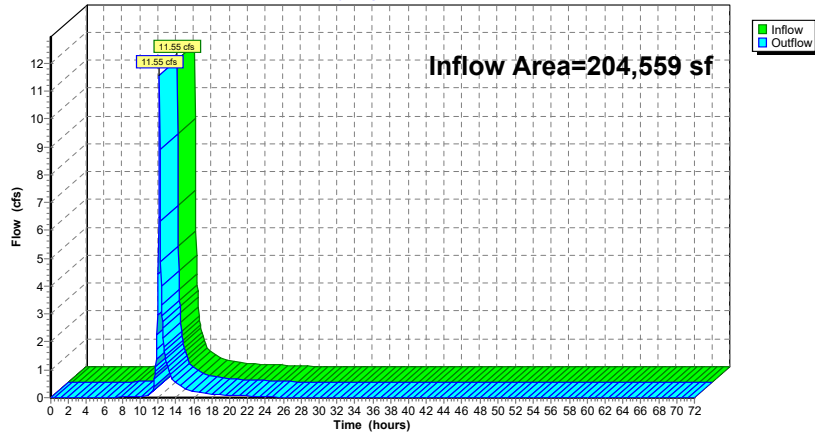
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 204,559 sf, 56.22% Impervious, Inflow Depth = 1.55" for NOAA 2-yr event  
Inflow = 11.55 cfs @ 12.15 hrs, Volume= 26,345 cf  
Outflow = 11.55 cfs @ 12.15 hrs, Volume= 26,345 cf, Atten= 0%, Lag= 0.0 min  
Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach B: PARKING LOT B OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 234

**Summary for Reach BMP4\_O: BMP-4 OVERFLOW**

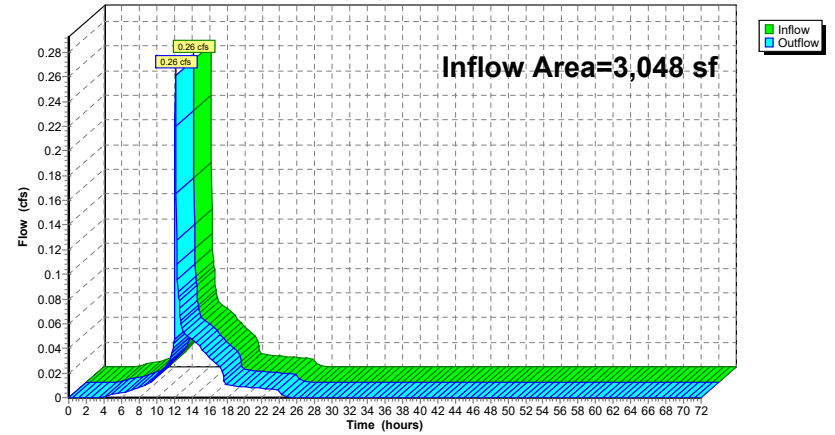
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3,048 sf, 86.09% Impervious, Inflow Depth = 6.15" for NOAA 2-yr event  
Inflow = 0.26 cfs @ 12.15 hrs, Volume= 1,561 cf  
Outflow = 0.26 cfs @ 12.15 hrs, Volume= 1,561 cf, Atten= 0%, Lag= 0.0 min  
Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP4\_O: BMP-4 OVERFLOW**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 235

**Summary for Reach BMP6\_O: BMP-6 OVERFLOW**

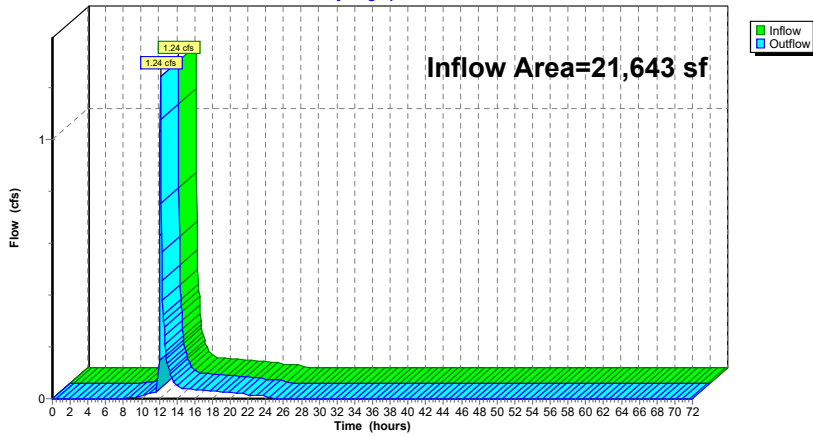
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 21,643 sf, 56.12% Impervious, Inflow Depth = 1.66" for NOAA 2-yr event  
Inflow = 1.24 cfs @ 12.15 hrs, Volume= 2,992 cf  
Outflow = 1.24 cfs @ 12.15 hrs, Volume= 2,992 cf, Atten= 0%, Lag= 0.0 min  
Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP6\_O: BMP-6 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 236

**Summary for Reach BMP7\_O: BMP-7 OVERFLOW**

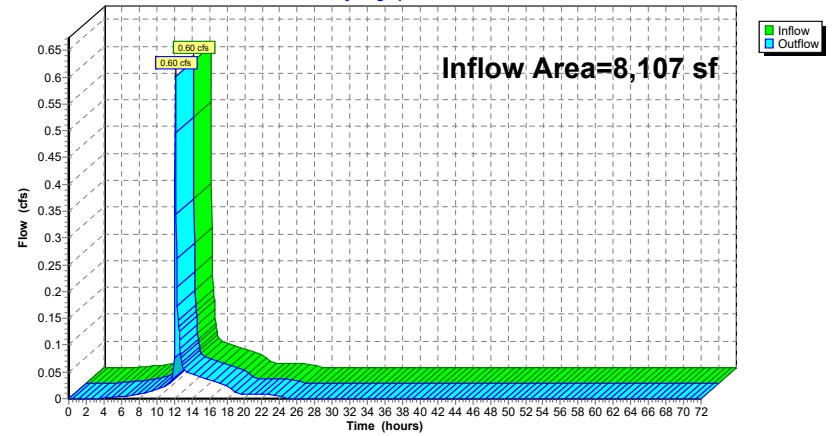
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8,107 sf, 88.34% Impervious, Inflow Depth = 2.84" for NOAA 2-yr event  
Inflow = 0.60 cfs @ 12.16 hrs, Volume= 1,919 cf  
Outflow = 0.60 cfs @ 12.16 hrs, Volume= 1,919 cf, Atten= 0%, Lag= 0.0 min  
Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP7\_O: BMP-7 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 237

**Summary for Reach BMP9\_O: BMP-9 OVERFLOW**

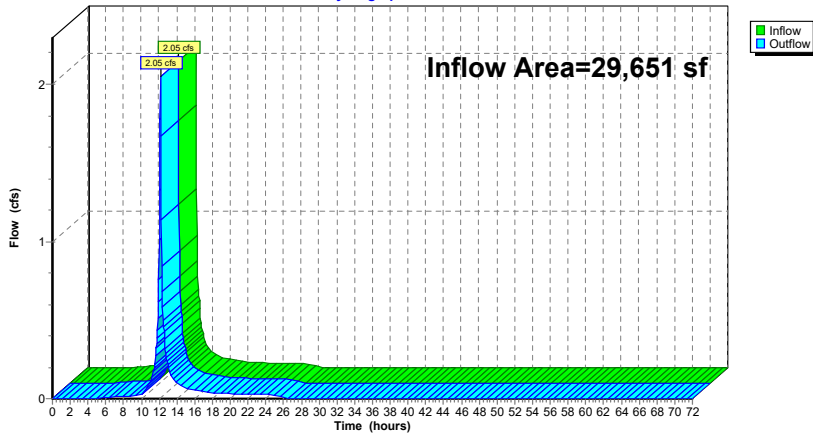
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 29,651 sf, 74.77% Impervious, Inflow Depth = 2.54" for NOAA 2-yr event  
 Inflow = 2.05 cfs @ 12.15 hrs, Volume= 6,279 cf  
 Outflow = 2.05 cfs @ 12.15 hrs, Volume= 6,279 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP9\_O: BMP-9 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 238

**Summary for Reach BMP\_3: BMP-3\_OVERFLOW**

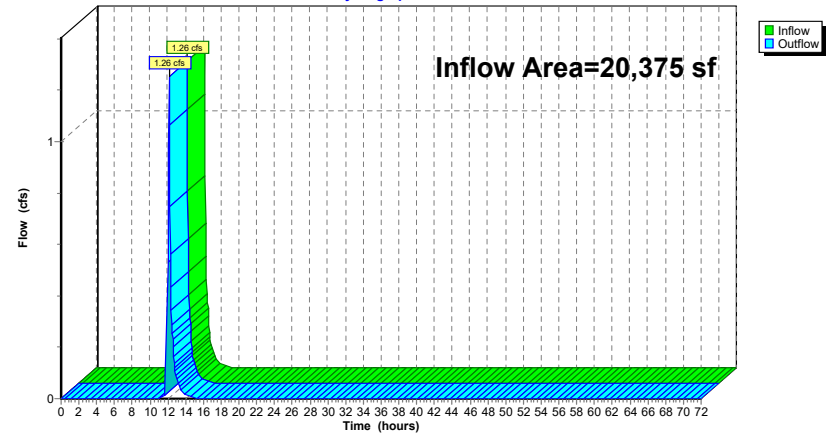
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 20,375 sf, 69.14% Impervious, Inflow Depth = 0.94" for NOAA 2-yr event  
 Inflow = 1.26 cfs @ 12.15 hrs, Volume= 1,596 cf  
 Outflow = 1.26 cfs @ 12.15 hrs, Volume= 1,596 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP\_3: BMP-3\_OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 239

**Summary for Reach DP-1: French Rodney Blvd 14" Outfall**

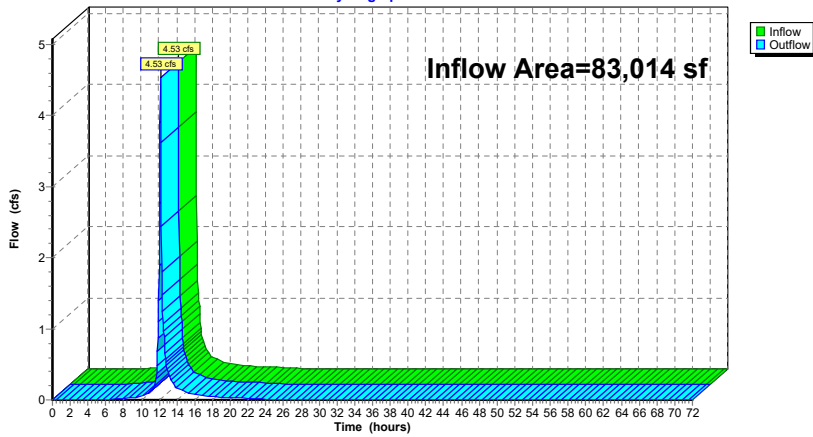
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 83,014 sf, 62.20% Impervious, Inflow Depth = 1.48" for NOAA 2-yr event  
Inflow = 4.53 cfs @ 12.15 hrs, Volume= 10,213 cf  
Outflow = 4.53 cfs @ 12.15 hrs, Volume= 10,213 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-1: French Rodney Blvd 14" Outfall**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 240

**Summary for Reach DP-2: NORTHERN OUTFALL**

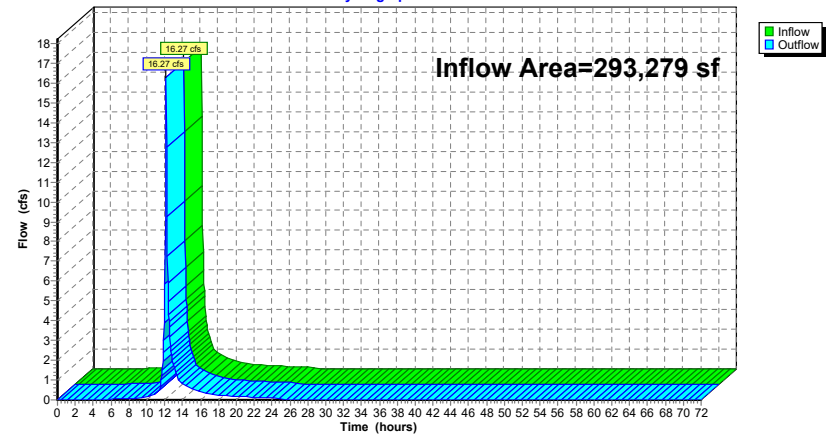
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 293,279 sf, 59.54% Impervious, Inflow Depth = 1.67" for NOAA 2-yr event  
Inflow = 16.27 cfs @ 12.15 hrs, Volume= 40,704 cf  
Outflow = 16.27 cfs @ 12.15 hrs, Volume= 40,704 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-2: NORTHERN OUTFALL**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 241

**Summary for Reach H ST: HUDSON STREET DRAINAGE**

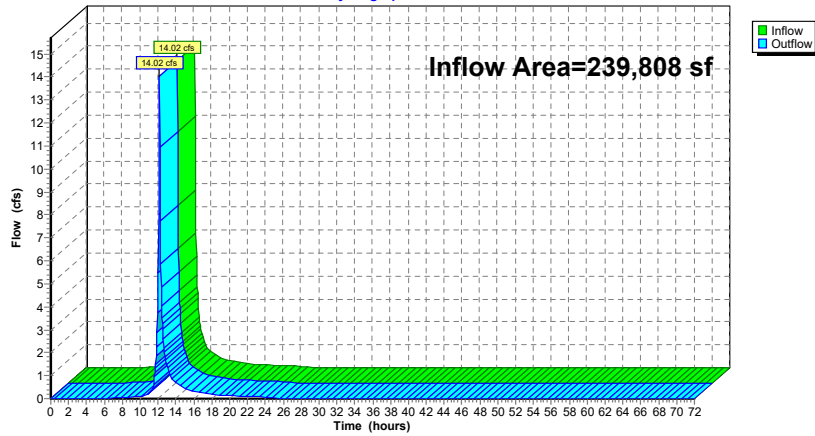
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 239,808 sf, 59.25% Impervious, Inflow Depth = 1.70" for NOAA 2-yr event  
 Inflow = 14.02 cfs @ 12.15 hrs, Volume= 33,949 cf  
 Outflow = 14.02 cfs @ 12.15 hrs, Volume= 33,949 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach DP-2 : NORTHERN OUTFALL

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach H ST: HUDSON STREET DRAINAGE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 242

**Summary for Reach P ST: PORTLAND STREET DRAINAGE**

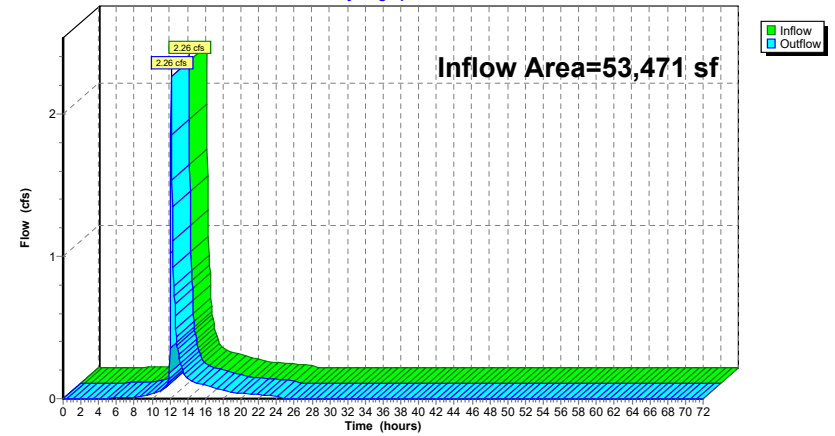
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 53,471 sf, 60.83% Impervious, Inflow Depth = 1.52" for NOAA 2-yr event  
 Inflow = 2.26 cfs @ 12.15 hrs, Volume= 6,755 cf  
 Outflow = 2.26 cfs @ 12.15 hrs, Volume= 6,755 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach DP-2 : NORTHERN OUTFALL

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach P ST: PORTLAND STREET DRAINAGE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 243

**Summary for Pond 1-P: BB 1**

Inflow Area = 38,826 sf, 51.66% Impervious, Inflow Depth = 2.01" for NOAA 2-yr event  
 Inflow = 2.27 cfs @ 12.13 hrs, Volume= 6,504 cf  
 Outflow = 2.18 cfs @ 12.15 hrs, Volume= 6,504 cf, Atten= 4%, Lag= 1.3 min  
 Discarded = 0.05 cfs @ 12.15 hrs, Volume= 2,289 cf  
 Primary = 0.66 cfs @ 12.15 hrs, Volume= 362 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall  
 Secondary = 1.47 cfs @ 12.15 hrs, Volume= 3,853 cf  
 Routed to Reach 15R : ISOLATOR ROW 1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.08' @ 12.15 hrs Surf.Area= 1,649 sf Storage= 900 cf

Plug-Flow detention time= 81.1 min calculated for 6,499 cf (100% of inflow)  
 Center-of-Mass det. time= 81.4 min ( 904.5 - 823.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.20'	1,114 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.20	490	0	0
9.50	800	194	194
10.20	1,830	920	1,114

Device	Routing	Invert	Outlet Devices
#1	Primary	8.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 ' S= 0.0100 ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Discarded	9.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.00'
#3	Device 1	10.00'	<b>24inch-Dome Grate Capacity X 2.00</b>
#4	Secondary	9.83'	<b>15inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.05 cfs @ 12.15 hrs HW=10.08' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.05 cfs)

**Primary OutFlow** Max=0.65 cfs @ 12.15 hrs HW=10.08' (Free Discharge)  
 ↳1=Culvert (Passes 0.65 cfs of 4.75 cfs potential flow)  
 ↳3=24inch-Dome Grate Capacity (Custom Controls 0.65 cfs)

**Secondary OutFlow** Max=1.47 cfs @ 12.15 hrs HW=10.08' (Free Discharge)  
 ↳4=15inch-Dome Grate Capacity (Custom Controls 1.47 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

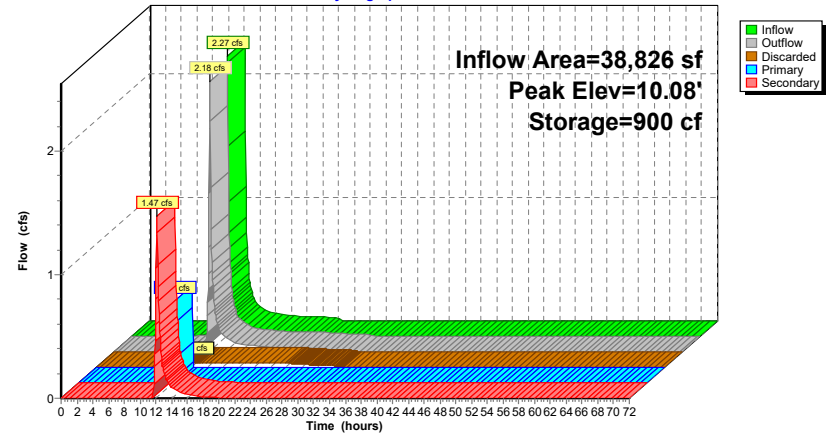
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 244

**Pond 1-P: BB 1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 245

**Summary for Pond 2a-P: BB 2a**

Inflow Area = 3,116 sf, 92.62% Impervious, Inflow Depth = 2.95" for NOAA 2-yr event  
 Inflow = 0.24 cfs @ 12.13 hrs, Volume= 765 cf  
 Outflow = 0.23 cfs @ 12.15 hrs, Volume= 729 cf, Atten= 3%, Lag= 1.2 min  
 Primary = 0.23 cfs @ 12.15 hrs, Volume= 729 cf  
 Routed to Reach 15R : ISOLATOR ROW 1  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.17' @ 12.15 hrs Surf.Area= 453 sf Storage= 66 cf

Plug-Flow detention time= 50.1 min calculated for 729 cf (95% of inflow)  
 Center-of-Mass det. time= 22.6 min ( 796.7 - 774.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	710 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	320	0	0
9.00	1,100	710	710

Device	Routing	Invert	Outlet Devices
#1	Secondary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/ S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	8.50'	<b>24inch-Dome Grate Capacity X 2.00</b>
#3	Primary	8.10'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.23 cfs @ 12.15 hrs HW=8.17' (Free Discharge)  
 ↳3=15inch-Dome Grate Capacity (Custom Controls 0.23 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.00' (Free Discharge)  
 ↳1=Culvert (Passes 0.00 cfs of 2.27 cfs potential flow)  
 ↳2=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

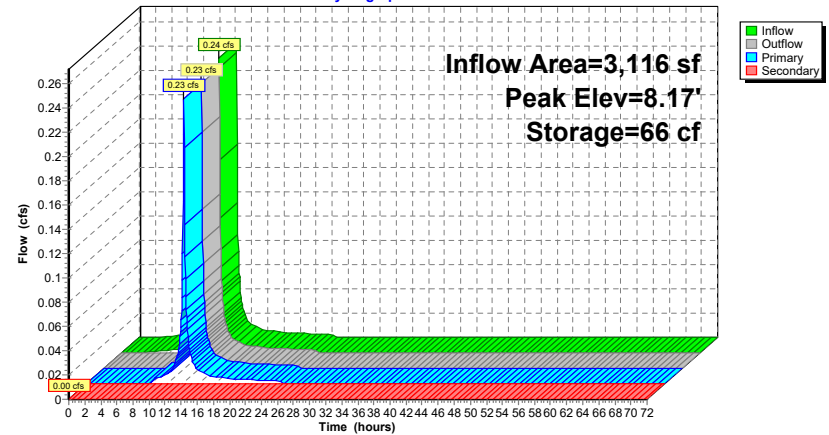
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 246

**Pond 2a-P: BB 2a**

**Hydrograph**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 247

**Summary for Pond 2b-P: BB 2b**

Inflow Area = 21,490 sf, 80.50% Impervious, Inflow Depth = 2.64" for NOAA 2-yr event  
 Inflow = 1.57 cfs @ 12.13 hrs, Volume= 4,725 cf  
 Outflow = 1.53 cfs @ 12.15 hrs, Volume= 4,689 cf, Atten= 3%, Lag= 1.1 min  
 Primary = 1.53 cfs @ 12.15 hrs, Volume= 4,689 cf  
 Routed to Reach 15R : ISOLATOR ROW 1  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.35' @ 12.15 hrs Surf.Area= 595 sf Storage= 161 cf

Plug-Flow detention time= 10.6 min calculated for 4,689 cf (99% of inflow)  
 Center-of-Mass det. time= 5.8 min ( 798.8 - 793.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	710 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	320	0	0
9.00	1,100	710	710

Device	Routing	Invert	Outlet Devices
#1	Secondary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/ S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	8.50'	<b>24inch-Dome Grate Capacity X 2.00</b>
#3	Primary	8.10'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=1.50 cfs @ 12.15 hrs HW=8.35' (Free Discharge)  
 ↳3=15inch-Dome Grate Capacity (Custom Controls 1.50 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.00' (Free Discharge)  
 ↳1=Culvert (Passes 0.00 cfs of 2.27 cfs potential flow)  
 ↳2=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

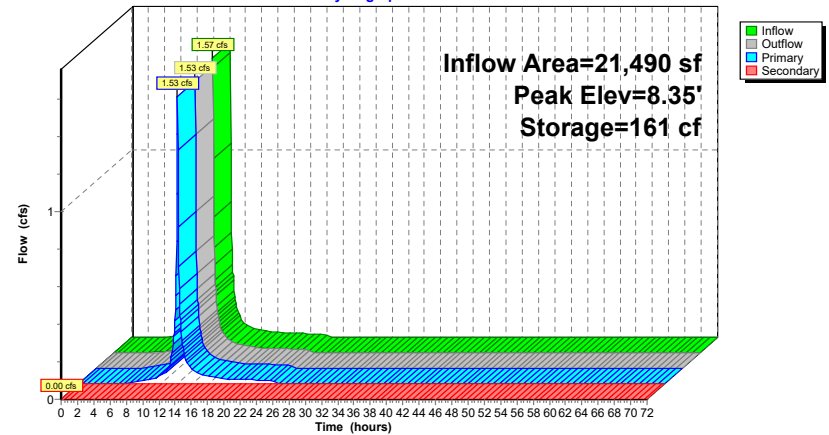
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 248

**Pond 2b-P: BB 2b**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 249

**Summary for Pond 3A-P: BB 3A**

Inflow Area = 10,987 sf, 58.16% Impervious, Inflow Depth = 2.18" for NOAA 2-yr event  
 Inflow = 0.69 cfs @ 12.13 hrs, Volume= 1,994 cf  
 Outflow = 0.65 cfs @ 12.16 hrs, Volume= 1,994 cf, Atten= 6%, Lag= 1.6 min  
 Discarded = 0.02 cfs @ 12.16 hrs, Volume= 1,062 cf  
 Primary = 0.63 cfs @ 12.16 hrs, Volume= 932 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.02' @ 12.16 hrs Surf.Area= 850 sf Storage= 405 cf

Plug-Flow detention time= 110.1 min calculated for 1,992 cf (100% of inflow)  
 Center-of-Mass det. time= 110.2 min ( 925.8 - 815.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.25'	622 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.25	271	0	0
10.45	350	62	62
11.25	1,050	560	622

Device	Routing	Invert	Outlet Devices
#1	Primary	9.30'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.30' / 9.20' S= 0.0100 1' Cc= 0.900 n= 0.013, Flow Area= 0.55 sf
#2	Discarded	10.25'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.30'
#3	Primary	11.15'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	10.90'	<b>24inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.02 cfs @ 12.16 hrs HW=11.02' (Free Discharge)  
 ↳ **2=Exfiltration** ( Controls 0.02 cfs)

**Primary OutFlow** Max=0.61 cfs @ 12.16 hrs HW=11.02' (Free Discharge)  
 ↳ **1=Culvert** (Passes 0.61 cfs of 3.00 cfs potential flow)  
 ↳ **4=24inch-Dome Grate Capacity** (Custom Controls 0.61 cfs)  
 ↳ **3=Sharp-Crested Rectangular Weir** ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

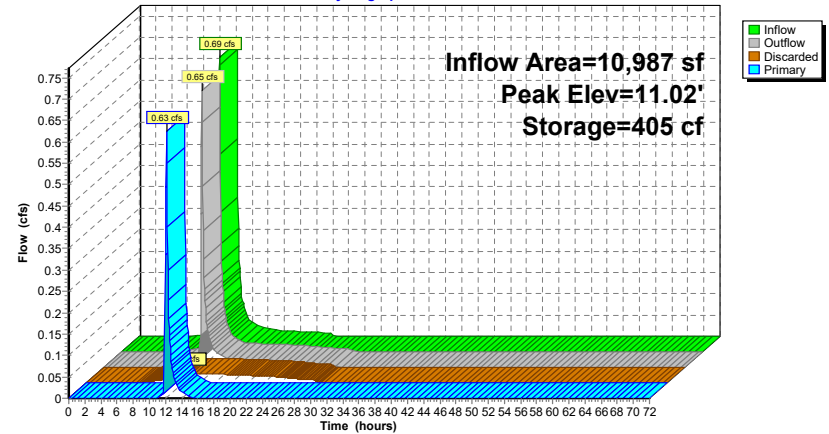
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 250

**Pond 3A-P: BB 3A**

**Hydrograph**





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 251

**Summary for Pond 3B-P: BB 3B**

Inflow Area = 4,545 sf, 77.34% Impervious, Inflow Depth = 2.64" for NOAA 2-yr event  
 Inflow = 0.33 cfs @ 12.13 hrs, Volume= 999 cf  
 Outflow = 0.33 cfs @ 12.15 hrs, Volume= 999 cf, Atten= 1%, Lag= 1.4 min  
 Discarded = 0.01 cfs @ 12.15 hrs, Volume= 642 cf  
 Primary = 0.31 cfs @ 12.15 hrs, Volume= 358 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 12.87' @ 12.15 hrs Surf.Area= 552 sf Storage= 244 cf

Plug-Flow detention time= 116.0 min calculated for 999 cf (100% of inflow)  
 Center-of-Mass det. time= 116.0 min ( 909.1 - 793.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	12.20'	263 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
12.20	180	0	0
12.90	570	263	263

Device	Routing	Invert	Outlet Devices
#1	Primary	10.70'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.70' / 10.60' S= 0.0100 ' / Cc= 0.900 n= 0.013, Flow Area= 0.55 sf
#2	Discarded	12.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 8.70'
#3	Device 1	12.80'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	12.85'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.01 cfs @ 12.15 hrs HW=12.87' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.01 cfs)

**Primary OutFlow** Max=0.31 cfs @ 12.15 hrs HW=12.87' (Free Discharge)  
 ↳1=Culvert (Passes 0.27 cfs of 3.47 cfs potential flow)  
 ↳3=24inch-Dome Grate Capacity (Custom Controls 0.27 cfs)  
 ↳4=Sharp-Crested Rectangular Weir (Weir Controls 0.03 cfs @ 0.42 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

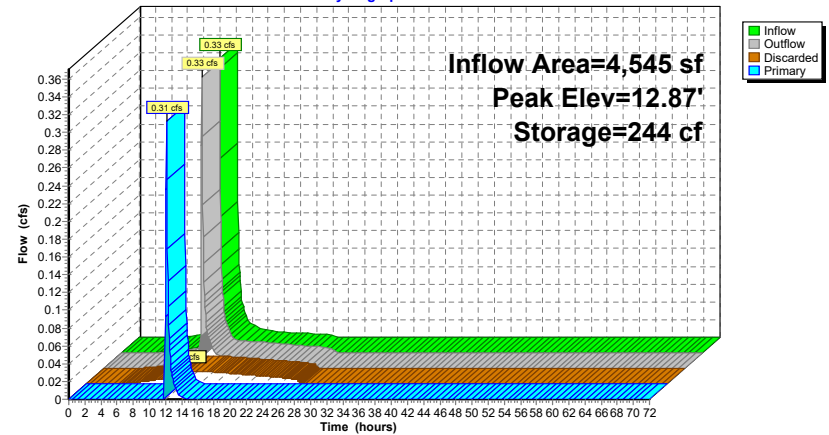
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 252

**Pond 3B-P: BB 3B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 253

**Summary for Pond 4A-P: BB 4A - POND**

Inflow Area = 4,843 sf, 86.37% Impervious, Inflow Depth = 2.84" for NOAA 2-yr event  
 Inflow = 0.37 cfs @ 12.13 hrs, Volume= 1,147 cf  
 Outflow = 0.35 cfs @ 12.15 hrs, Volume= 1,147 cf, Atten= 5%, Lag= 1.7 min  
 Primary = 0.32 cfs @ 12.15 hrs, Volume= 307 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW  
 Secondary = 0.03 cfs @ 12.15 hrs, Volume= 839 cf  
 Routed to Pond 4A-S : BB4A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.02' @ 12.15 hrs Surf.Area= 560 sf Storage= 212 cf

Plug-Flow detention time= 34.1 min calculated for 1,147 cf (100% of inflow)  
 Center-of-Mass det. time= 34.0 min ( 815.2 - 781.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.50'	320 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.50	250	0	0
10.20	664	320	320

Device	Routing	Invert	Outlet Devices
#1	Primary	8.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.00'
#3	Primary	10.10'	<b>5.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)</b>
#4	Device 1	9.95'	<b>24inchDome Grate Capacity</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950

**Primary OutFlow** Max=0.31 cfs @ 12.15 hrs HW=10.02' (Free Discharge)  
 1=Culvert (Passes 0.31 cfs of 4.67 cfs potential flow)  
 4=24inchDome Grate Capacity (Custom Controls 0.31 cfs)  
 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.03 cfs @ 12.15 hrs HW=10.02' (Free Discharge)  
 2=Exfiltration ( Controls 0.03 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

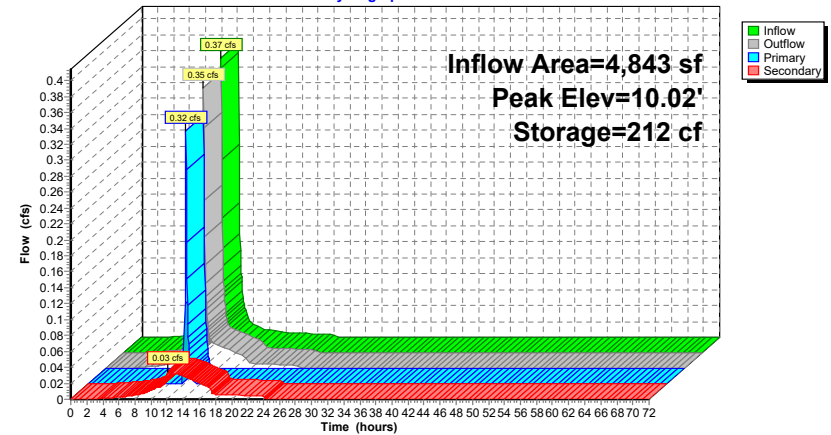
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 254

**Pond 4A-P: BB 4A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 255

**Summary for Pond 4A-S: BB4A-Stone**

Inflow = 0.03 cfs @ 12.15 hrs, Volume= 839 cf  
 Outflow = 0.03 cfs @ 12.20 hrs, Volume= 839 cf, Atten= 1%, Lag= 2.6 min  
 Primary = 0.03 cfs @ 12.20 hrs, Volume= 839 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.12' @ 12.20 hrs Surf.Area= 230 sf Storage= 8 cf

Plug-Flow detention time= 6.2 min calculated for 839 cf (100% of inflow)  
 Center-of-Mass det. time= 6.3 min ( 850.4 - 844.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	138 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 460 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	230	0	0
8.00	230	460	460

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.03 cfs @ 12.20 hrs HW=6.12' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.19 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

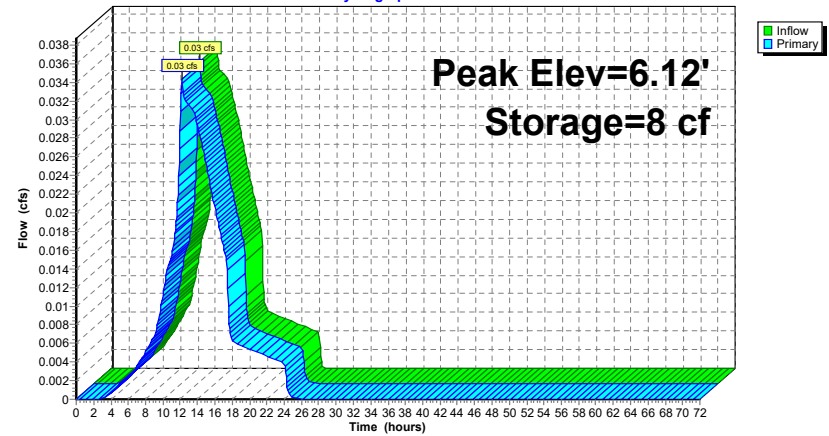
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 256

**Pond 4A-S: BB4A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 257

**Summary for Pond 4B-P: BB 4B - POND**

Inflow Area = 3,048 sf, 86.09% Impervious, Inflow Depth = 2.84" for NOAA 2-yr event  
 Inflow = 0.23 cfs @ 12.13 hrs, Volume= 722 cf  
 Outflow = 0.23 cfs @ 12.15 hrs, Volume= 722 cf, Atten= 2%, Lag= 1.2 min  
 Primary = 0.21 cfs @ 12.15 hrs, Volume= 199 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW  
 Secondary = 0.02 cfs @ 12.15 hrs, Volume= 522 cf  
 Routed to Pond 4B-S : BB 4A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.00' @ 12.15 hrs Surf.Area= 346 sf Storage= 124 cf

Plug-Flow detention time= 33.2 min calculated for 721 cf (100% of inflow)  
 Center-of-Mass det. time= 33.2 min ( 814.4 - 781.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.50'	199 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.50	144	0	0
11.20	424	199	199

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.00' / 8.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.00'
#3	Primary	11.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	10.95'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.20 cfs @ 12.15 hrs HW=11.00' (Free Discharge)

- 1=Culvert (Passes 0.20 cfs of 4.64 cfs potential flow)
- 4=24inch-Dome Grate Capacity (Custom Controls 0.20 cfs)
- 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.02 cfs @ 12.15 hrs HW=11.00' (Free Discharge)

- 2=Exfiltration ( Controls 0.02 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

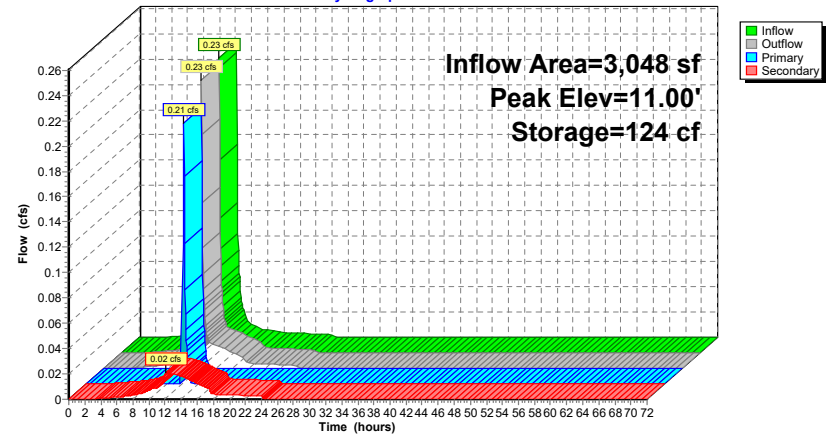
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 258

**Pond 4B-P: BB 4B - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 259

**Summary for Pond 4B-S: BB 4A-Stone**

Inflow = 0.02 cfs @ 12.15 hrs, Volume= 522 cf  
 Outflow = 0.02 cfs @ 12.18 hrs, Volume= 522 cf, Atten= 0%, Lag= 1.7 min  
 Primary = 0.02 cfs @ 12.18 hrs, Volume= 522 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.09' @ 12.18 hrs Surf.Area= 145 sf Storage= 4 cf

Plug-Flow detention time= 4.9 min calculated for 522 cf (100% of inflow)  
 Center-of-Mass det. time= 4.9 min ( 849.5 - 844.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	87 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 290 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	145	0	0
8.00	145	290	290

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.18 hrs HW=6.09' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.02 cfs @ 1.04 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

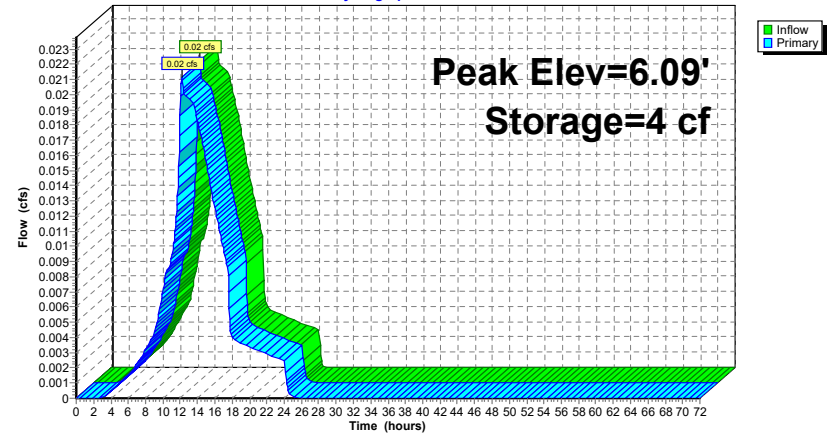
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 260

**Pond 4B-S: BB 4A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 261

**Summary for Pond 5A-P: BB 5A - POND**

Inflow Area = 3,072 sf, 73.44% Impervious, Inflow Depth = 2.54" for NOAA 2-yr event  
 Inflow = 0.22 cfs @ 12.13 hrs, Volume= 651 cf  
 Outflow = 0.04 cfs @ 12.59 hrs, Volume= 651 cf, Atten= 83%, Lag= 27.8 min  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW  
 Secondary = 0.04 cfs @ 12.59 hrs, Volume= 651 cf  
 Routed to Pond 5A-PS : BB 5A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.16' @ 12.59 hrs Surf.Area= 599 sf Storage= 195 cf

Plug-Flow detention time= 38.4 min calculated for 650 cf (100% of inflow)  
 Center-of-Mass det. time= 38.4 min ( 836.6 - 798.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.80'	645 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.80	480	0	0
9.80	810	645	645

Device	Routing	Invert	Outlet Devices
#1	Primary	7.60'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.60' / 7.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.80'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.60'
#3	Device 1	9.50'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.80' (Free Discharge)  
 1=Culvert (Passes 0.00 cfs of 2.86 cfs potential flow)  
 3=24inch-Dome Grate Capacity ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.59 hrs HW=9.16' (Free Discharge)  
 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

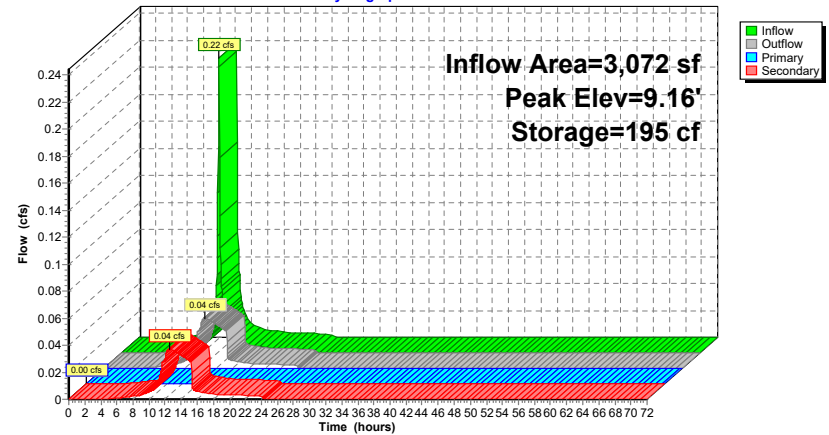
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 262

**Pond 5A-P: BB 5A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 263

**Summary for Pond 5A-PS: BB 5A-Stone**

Inflow = 0.04 cfs @ 12.59 hrs, Volume= 651 cf  
 Outflow = 0.04 cfs @ 12.67 hrs, Volume= 651 cf, Atten= 0%, Lag= 4.7 min  
 Primary = 0.04 cfs @ 12.67 hrs, Volume= 651 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.13' @ 12.67 hrs Surf.Area= 480 sf Storage= 18 cf

Plug-Flow detention time= 13.6 min calculated for 651 cf (100% of inflow)  
 Center-of-Mass det. time= 13.3 min ( 849.8 - 836.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	288 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 960 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	480	0	0
8.00	480	960	960

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.67 hrs HW=6.13' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.21 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

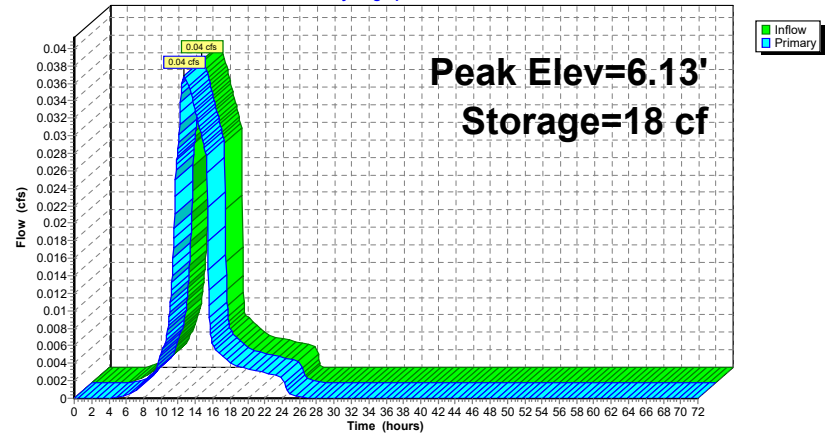
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 264

**Pond 5A-PS: BB 5A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 265

**Summary for Pond 5B-P: BB 5B - POND**

Inflow Area = 34,755 sf, 71.39% Impervious, Inflow Depth = 2.45" for NOAA 2-yr event  
 Inflow = 2.40 cfs @ 12.13 hrs, Volume= 7,086 cf  
 Outflow = 2.36 cfs @ 12.15 hrs, Volume= 7,086 cf, Atten= 1%, Lag= 1.0 min  
 Primary = 0.59 cfs @ 12.15 hrs, Volume= 277 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW  
 Secondary = 0.08 cfs @ 12.15 hrs, Volume= 2,882 cf  
 Routed to Pond 5B-PS : BB 5B-Stone  
 Tertiary = 1.70 cfs @ 12.15 hrs, Volume= 3,926 cf  
 Routed to Reach 6R : ISOLATOR ROW 2

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.97' @ 12.15 hrs Surf.Area= 1,192 sf Storage= 585 cf

Plug-Flow detention time= 31.5 min calculated for 7,081 cf (100% of inflow)  
 Center-of-Mass det. time= 31.5 min ( 834.5 - 803.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.20'	889 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.20	327	0	0
9.20	1,450	889	889

Device	Routing	Invert	Outlet Devices
#1	Primary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.20'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.00'
#3	Device 1	8.90'	<b>24inchDome Grate Capacity X 2.00</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Tertiary	8.70'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.57 cfs @ 12.15 hrs HW=8.97' (Free Discharge)

↳ **1=Culvert** (Passes 0.57 cfs of 4.58 cfs potential flow)

↳ **3=24inchDome Grate Capacity** (Custom Controls 0.57 cfs)

**Secondary OutFlow** Max=0.08 cfs @ 12.15 hrs HW=8.97' (Free Discharge)

↳ **2=Exfiltration** ( Controls 0.08 cfs)

**Tertiary OutFlow** Max=1.68 cfs @ 12.15 hrs HW=8.97' (Free Discharge)

↳ **4=15inch-Dome Grate Capacity** (Custom Controls 1.68 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

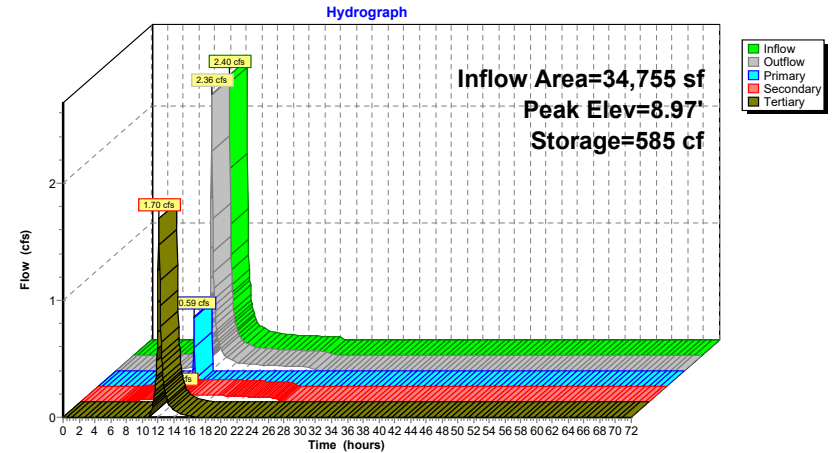
Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 266

**Pond 5B-P: BB 5B - POND**





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 267

**Summary for Pond 5B-PS: BB 5B-Stone**

[44] Hint: Outlet device #1 is below defined storage

Inflow = 0.08 cfs @ 12.15 hrs, Volume= 2,882 cf  
 Outflow = 0.08 cfs @ 12.15 hrs, Volume= 2,882 cf, Atten= 0%, Lag= 0.1 min  
 Primary = 0.08 cfs @ 12.15 hrs, Volume= 2,882 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.00' @ 12.15 hrs Surf.Area= 690 sf Storage= 1 cf

Plug-Flow detention time= 0.1 min calculated for 2,880 cf (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 964.5 - 964.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	414 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 1,380 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	690	0	0
8.00	690	1,380	1,380

Device	Routing	Invert	Outlet Devices
#1	Primary	4.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.57 cfs @ 12.15 hrs HW=6.00' (Free Discharge)  
 1=Orifice/Grate (Orifice Controls 0.57 cfs @ 6.52 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

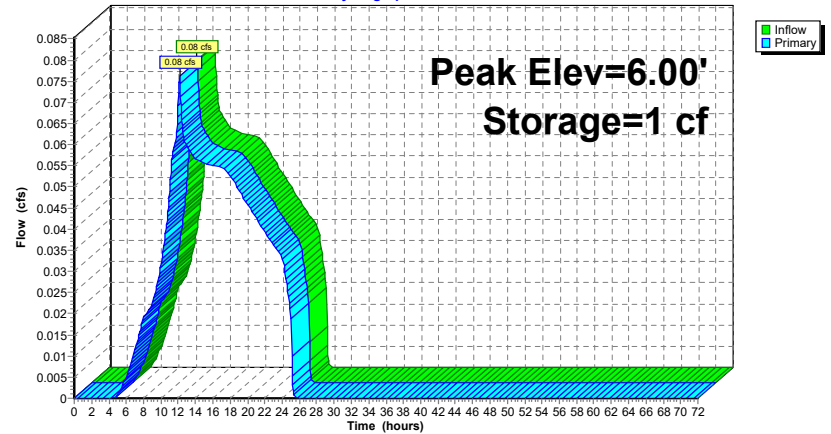
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 268

**Pond 5B-PS: BB 5B-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 269

**Summary for Pond 6A-P: BB 6A - POND**

Inflow Area = 15,148 sf, 46.97% Impervious, Inflow Depth = 1.93" for NOAA 2-yr event  
 Inflow = 0.85 cfs @ 12.13 hrs, Volume= 2,436 cf  
 Outflow = 0.82 cfs @ 12.15 hrs, Volume= 2,436 cf, Atten= 4%, Lag= 1.3 min  
 Primary = 0.77 cfs @ 12.15 hrs, Volume= 996 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW  
 Secondary = 0.04 cfs @ 12.15 hrs, Volume= 1,440 cf  
 Routed to Pond 6A-PS : BB 6A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.94' @ 12.15 hrs Surf.Area= 671 sf Storage= 378 cf

Plug-Flow detention time= 47.5 min calculated for 2,434 cf (100% of inflow)  
 Center-of-Mass det. time= 47.4 min ( 874.1 - 826.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.20'	491 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.20	350	0	0
11.10	740	491	491

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.00' / 8.90' S= 0.0100 '/ S= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.20'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.00'
#3	Device 1	10.80'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	11.00'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.77 cfs @ 12.15 hrs HW=10.94' (Free Discharge)

- 1=Culvert (Passes 0.77 cfs of 4.54 cfs potential flow)
- 3=24inch-Dome Grate Capacity (Custom Controls 0.77 cfs)
- 4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.15 hrs HW=10.94' (Free Discharge)

- 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

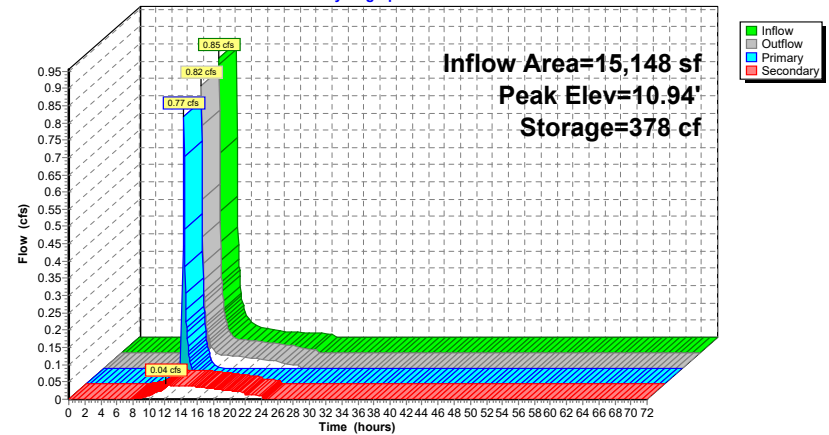
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 270

**Pond 6A-P: BB 6A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 271

**Summary for Pond 6A-PS: BB 6A - STONE**

Inflow = 0.04 cfs @ 12.15 hrs, Volume= 1,440 cf  
 Outflow = 0.04 cfs @ 12.20 hrs, Volume= 1,440 cf, Atten= 1%, Lag= 2.7 min  
 Primary = 0.04 cfs @ 12.20 hrs, Volume= 1,440 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.14' @ 12.20 hrs Surf.Area= 290 sf Storage= 12 cf

Plug-Flow detention time= 5.8 min calculated for 1,439 cf (100% of inflow)  
 Center-of-Mass det. time= 5.8 min ( 970.1 - 964.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	174 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 580 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	290	0	0
8.00	290	580	580

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.20 hrs HW=6.14' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.27 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

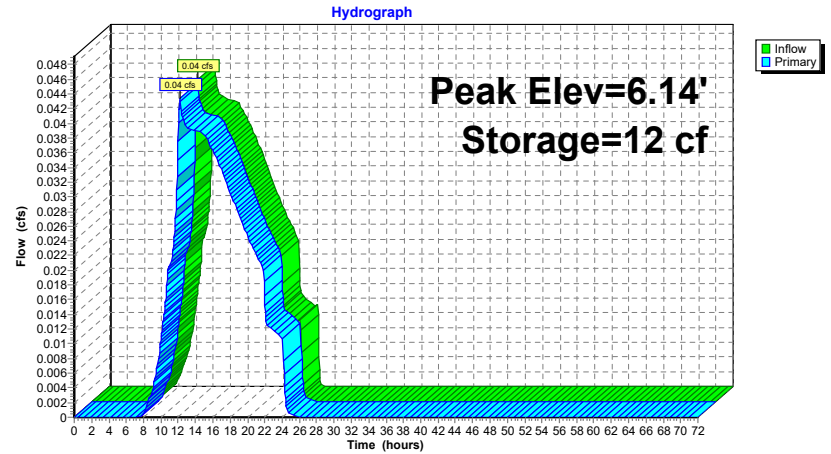
Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 272

**Pond 6A-PS: BB 6A - STONE**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 273

**Summary for Pond 6B-P: BB 6B**

Inflow Area = 6,495 sf, 77.45% Impervious, Inflow Depth = 2.64" for NOAA 2-yr event  
 Inflow = 0.47 cfs @ 12.13 hrs, Volume= 1,428 cf  
 Outflow = 0.44 cfs @ 12.15 hrs, Volume= 1,428 cf, Atten= 6%, Lag= 1.6 min  
 Discarded = 0.02 cfs @ 12.15 hrs, Volume= 871 cf  
 Primary = 0.43 cfs @ 12.15 hrs, Volume= 557 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 12.04' @ 12.15 hrs Surf.Area= 658 sf Storage= 356 cf

Plug-Flow detention time= 135.2 min calculated for 1,427 cf (100% of inflow)  
 Center-of-Mass det. time= 135.3 min ( 928.4 - 793.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	11.20'	394 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
11.20	185	0	0
12.10	690	394	394

Device	Routing	Invert	Outlet Devices
#1	Primary	10.10'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.10' / 10.00' S= 0.0100 ' /' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Discarded	11.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 8.10'
#3	Device 1	11.95'	<b>24inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.02 cfs @ 12.15 hrs HW=12.04' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.02 cfs)

**Primary OutFlow** Max=0.42 cfs @ 12.15 hrs HW=12.04' (Free Discharge)  
 ↳1=Culvert (Passes 0.42 cfs of 4.54 cfs potential flow)  
 ↳3=24inch-Dome Grate Capacity (Custom Controls 0.42 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

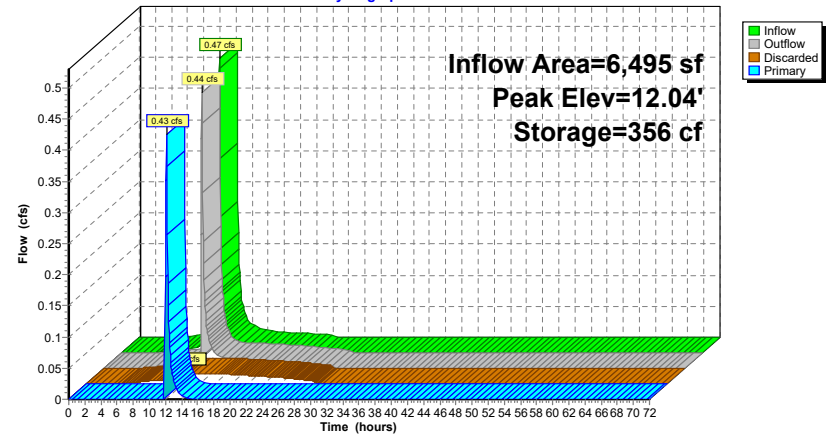
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 274

**Pond 6B-P: BB 6B**

**Hydrograph**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 275

**Summary for Pond 7A-P: BB 7A PONDING**

Inflow Area = 3,165 sf, 87.74% Impervious, Inflow Depth = 2.84" for NOAA 2-yr event  
 Inflow = 0.24 cfs @ 12.13 hrs, Volume= 749 cf  
 Outflow = 0.24 cfs @ 12.15 hrs, Volume= 749 cf, Atten= 1%, Lag= 1.1 min  
 Primary = 0.22 cfs @ 12.15 hrs, Volume= 210 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW  
 Secondary = 0.02 cfs @ 12.14 hrs, Volume= 539 cf  
 Routed to Pond 7A-S : BB 7A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.96' @ 12.14 hrs Surf.Area= 316 sf Storage= 141 cf

Plug-Flow detention time= 43.9 min calculated for 749 cf (100% of inflow)  
 Center-of-Mass det. time= 43.9 min ( 825.1 - 781.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.30'	227 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.30	115	0	0
10.20	390	227	227

Device	Routing	Invert	Outlet Devices
#1	Primary	8.10'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.10' / 8.00' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.30'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.10'
#3	Device 1	9.90'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	10.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.22 cfs @ 12.15 hrs HW=9.96' (Free Discharge)

- 1=Culvert (Passes 0.22 cfs of 4.40 cfs potential flow)
- 3=24inch-Dome Grate Capacity (Custom Controls 0.22 cfs)
- 4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.02 cfs @ 12.14 hrs HW=9.96' (Free Discharge)

- 2=Exfiltration ( Controls 0.02 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

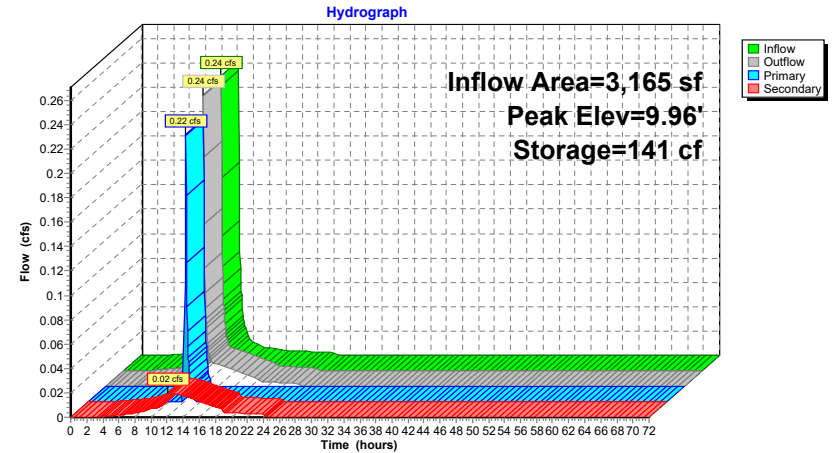
Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 276

**Pond 7A-P: BB 7A PONDING**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 277

**Summary for Pond 7A-S: BB 7A - STONE**

Inflow = 0.02 cfs @ 12.14 hrs, Volume= 539 cf  
 Outflow = 0.02 cfs @ 12.18 hrs, Volume= 539 cf, Atten= 0%, Lag= 1.9 min  
 Primary = 0.02 cfs @ 12.18 hrs, Volume= 539 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5.19' @ 12.18 hrs Surf.Area= 150 sf Storage= 4 cf

Plug-Flow detention time= 5.1 min calculated for 539 cf (100% of inflow)  
 Center-of-Mass det. time= 5.1 min ( 864.8 - 859.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	5.10'	90 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 300 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
5.10	150	0	0
7.10	150	300	300

Device	Routing	Invert	Outlet Devices
#1	Primary	5.10'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.18 hrs HW=5.19' (Free Discharge)  
 ←1=Orifice/Grate (Orifice Controls 0.02 cfs @ 1.03 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

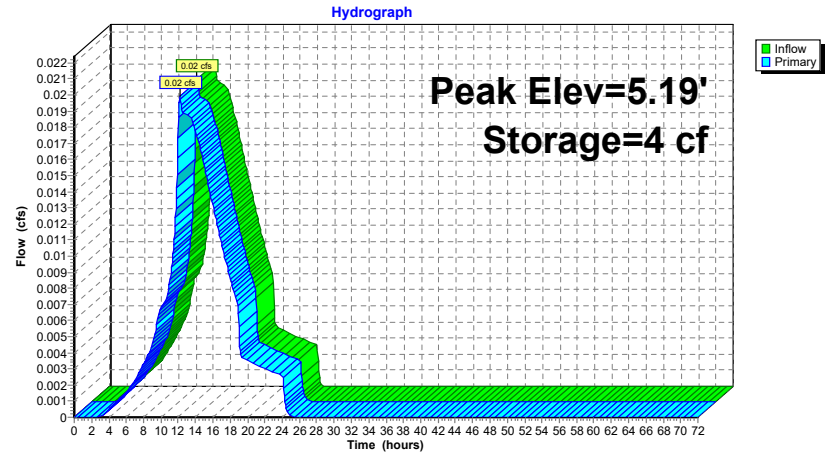
Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 278

**Pond 7A-S: BB 7A - STONE**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 279

**Summary for Pond 7B-P: BB 7B PONDING**

Inflow Area = 4,942 sf, 88.73% Impervious, Inflow Depth = 2.84" for NOAA 2-yr event  
 Inflow = 0.38 cfs @ 12.13 hrs, Volume= 1,170 cf  
 Outflow = 0.37 cfs @ 12.16 hrs, Volume= 1,170 cf, Atten= 3%, Lag= 2.1 min  
 Primary = 0.33 cfs @ 12.16 hrs, Volume= 283 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW  
 Secondary = 0.03 cfs @ 12.16 hrs, Volume= 887 cf  
 Routed to Pond 7B-S : BB 7B - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.68' @ 12.16 hrs Surf.Area= 512 sf Storage= 258 cf

Plug-Flow detention time= 47.2 min calculated for 1,170 cf (100% of inflow)  
 Center-of-Mass det. time= 47.2 min ( 828.4 - 781.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	324 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	250	0	0
10.80	560	324	324

Device	Routing	Invert	Outlet Devices
#1	Primary	8.90'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.90' / 8.80' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.00'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.90'
#3	Device 1	10.60'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.31 cfs @ 12.16 hrs HW=10.67' (Free Discharge)  
 ↳1=Culvert (Passes 0.31 cfs of 4.27 cfs potential flow)  
 ↳3=24inch-Dome Grate Capacity (Custom Controls 0.31 cfs)

**Secondary OutFlow** Max=0.03 cfs @ 12.16 hrs HW=10.67' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.03 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

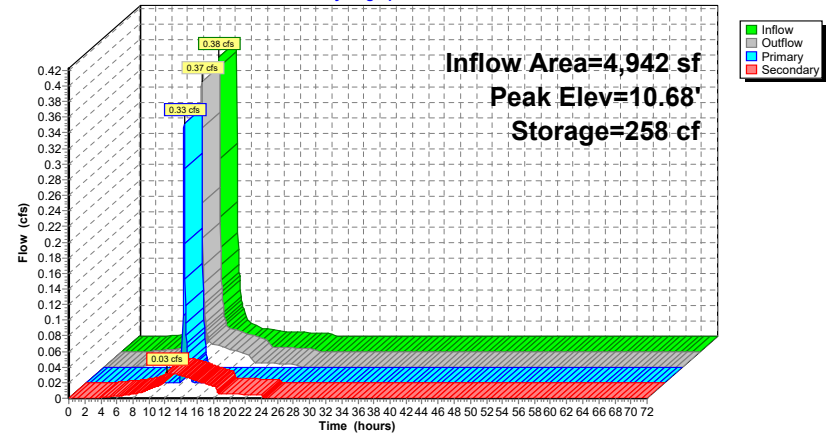
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 280

**Pond 7B-P: BB 7B PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 281

**Summary for Pond 7B-S: BB 7B - STONE**

Inflow = 0.03 cfs @ 12.16 hrs, Volume= 887 cf  
 Outflow = 0.03 cfs @ 12.20 hrs, Volume= 887 cf, Atten= 1%, Lag= 2.1 min  
 Primary = 0.03 cfs @ 12.20 hrs, Volume= 887 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5.22' @ 12.20 hrs Surf.Area= 150 sf Storage= 5 cf

Plug-Flow detention time= 4.1 min calculated for 887 cf (100% of inflow)  
 Center-of-Mass det. time= 4.0 min ( 861.4 - 857.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	5.10'	90 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 300 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
5.10	150	0	0
7.10	150	300	300

Device	Routing	Invert	Outlet Devices
#1	Primary	5.10'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.03 cfs @ 12.20 hrs HW=5.22' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.17 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

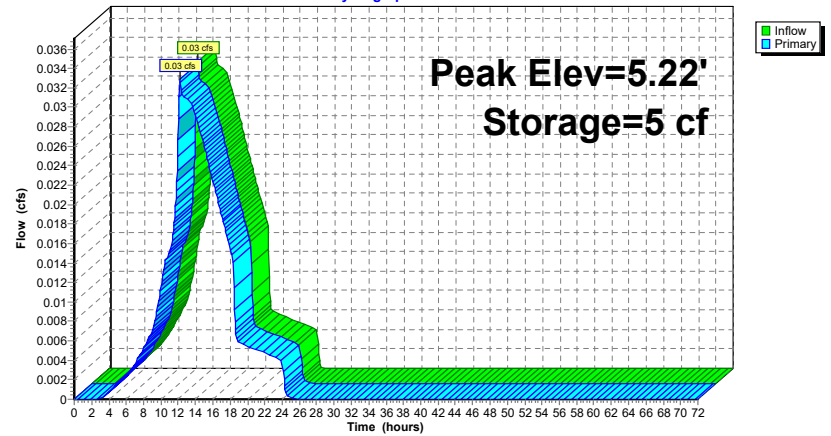
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 282

**Pond 7B-S: BB 7B - STONE**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 283

**Summary for Pond 8a-P: BB 8A PONDING**

Inflow Area = 3,978 sf, 79.99% Impervious, Inflow Depth = 2.64" for NOAA 2-yr event  
 Inflow = 0.29 cfs @ 12.13 hrs, Volume= 875 cf  
 Outflow = 0.14 cfs @ 12.27 hrs, Volume= 875 cf, Atten= 52%, Lag= 8.5 min  
 Primary = 0.10 cfs @ 12.27 hrs, Volume= 79 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE  
 Secondary = 0.04 cfs @ 12.27 hrs, Volume= 795 cf  
 Routed to Pond 8a-s : BB 8A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.03' @ 12.27 hrs Surf.Area= 587 sf Storage= 250 cf

Plug-Flow detention time= 47.4 min calculated for 874 cf (100% of inflow)  
 Center-of-Mass det. time= 47.3 min ( 840.4 - 793.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.50'	575 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.50	360	0	0
9.50	790	575	575

Device	Routing	Invert	Outlet Devices
#1	Primary	7.40'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.40' / 7.30' S= 0.0100 '/ S= 0.0100 ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.40'
#3	Device 1	9.00'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	9.40'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.09 cfs @ 12.27 hrs HW=9.03' (Free Discharge)

- 1=Culvert (Passes 0.09 cfs of 3.99 cfs potential flow)
- 3=24inch-Dome Grate Capacity (Custom Controls 0.09 cfs)
- 4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.27 hrs HW=9.03' (Free Discharge)

- 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

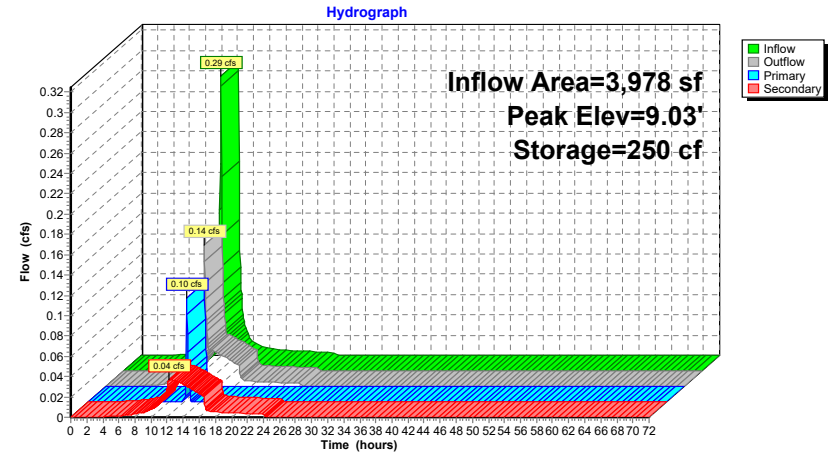
Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 284

**Pond 8a-P: BB 8A PONDING**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 285

**Summary for Pond 8a-s: BB 8A - STONE**

Inflow = 0.04 cfs @ 12.27 hrs, Volume= 795 cf  
 Outflow = 0.04 cfs @ 12.35 hrs, Volume= 795 cf, Atten= 1%, Lag= 5.1 min  
 Primary = 0.04 cfs @ 12.35 hrs, Volume= 795 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 4.53' @ 12.35 hrs Surf.Area= 300 sf Storage= 11 cf

Plug-Flow detention time= 7.9 min calculated for 795 cf (100% of inflow)  
 Center-of-Mass det. time= 7.9 min ( 858.1 - 850.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	4.40'	180 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 600 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
4.40	300	0	0
6.40	300	600	600

Device	Routing	Invert	Outlet Devices
#1	Primary	4.40'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.35 hrs HW=4.53' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.21 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

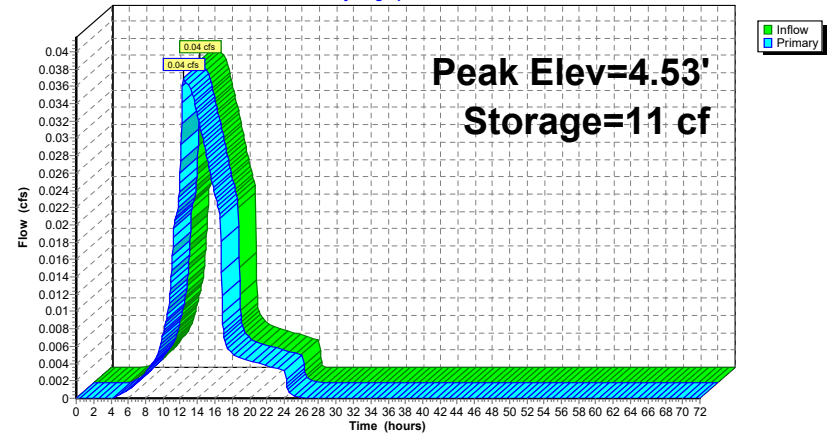
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 286

**Pond 8a-s: BB 8A - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 287

**Summary for Pond 8B-P: BB 8B-PONDING**

Inflow Area = 5,598 sf, 87.78% Impervious, Inflow Depth = 2.84" for NOAA 2-yr event  
 Inflow = 0.43 cfs @ 12.13 hrs, Volume= 1,325 cf  
 Outflow = 0.41 cfs @ 12.15 hrs, Volume= 1,325 cf, Atten= 3%, Lag= 1.2 min  
 Primary = 0.38 cfs @ 12.15 hrs, Volume= 364 cf  
 Routed to Reach H ST : HUDSON STREET DRAINAGE  
 Secondary = 0.04 cfs @ 12.15 hrs, Volume= 962 cf  
 Routed to Pond 8B-S : BB 8B-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.70' @ 12.15 hrs Surf.Area= 615 sf Storage= 242 cf

Plug-Flow detention time= 39.5 min calculated for 1,325 cf (100% of inflow)  
 Center-of-Mass det. time= 39.4 min ( 820.6 - 781.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.10'	306 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.10	190	0	0
9.80	685	306	306

Device	Routing	Invert	Outlet Devices
#1	Primary	7.90'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.90' / 7.80' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.10'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.90'
#3	Device 1	9.65'	<b>24inch-Dome Grate Capacity X 2.00</b>

**Primary OutFlow** Max=0.37 cfs @ 12.15 hrs HW=9.70' (Free Discharge)  
 1=Culvert (Passes 0.37 cfs of 4.31 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 0.37 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.15 hrs HW=9.70' (Free Discharge)  
 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

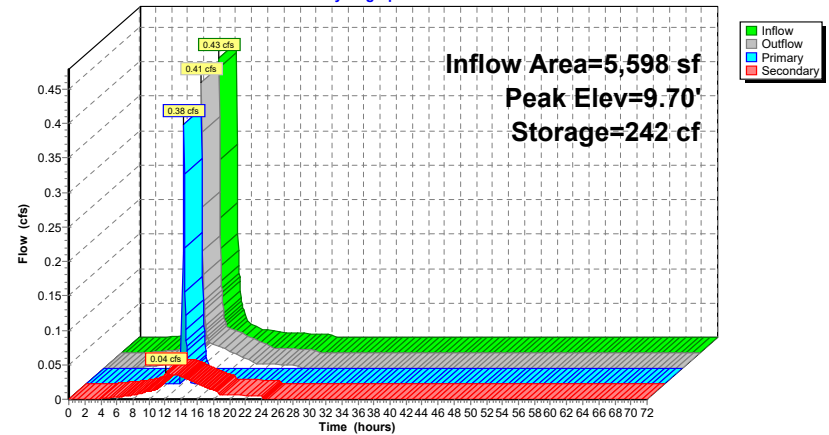
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 288

**Pond 8B-P: BB 8B-PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 289

**Summary for Pond 8B-S: BB 8B-Stone**

Inflow = 0.04 cfs @ 12.15 hrs, Volume= 962 cf  
 Outflow = 0.04 cfs @ 12.20 hrs, Volume= 962 cf, Atten= 1%, Lag= 3.0 min  
 Primary = 0.04 cfs @ 12.20 hrs, Volume= 962 cf  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 4.53' @ 12.20 hrs Surf.Area= 300 sf Storage= 12 cf

Plug-Flow detention time= 7.7 min calculated for 961 cf (100% of inflow)  
 Center-of-Mass det. time= 7.7 min ( 860.6 - 852.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	4.40'	180 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 600 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
4.40	300	0	0
6.40	300	600	600

Device	Routing	Invert	Outlet Devices
#1	Primary	4.40'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.20 hrs HW=4.53' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.22 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

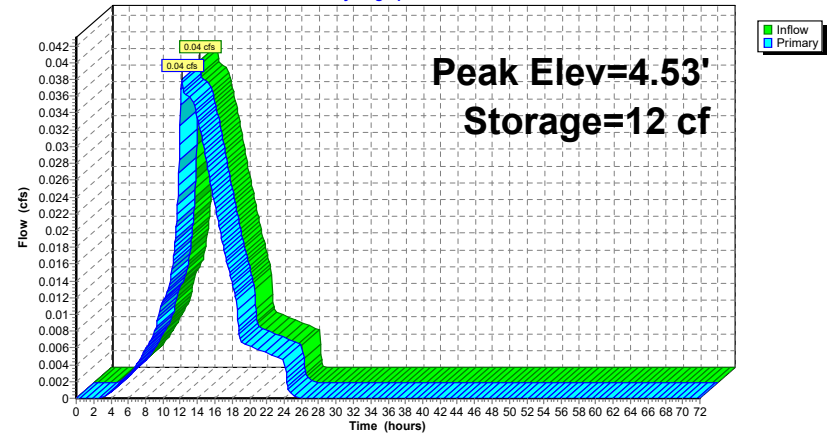
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 290

**Pond 8B-S: BB 8B-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 291

**Summary for Pond 9-P: BB9 - POND**

Inflow Area = 29,651 sf, 74.77% Impervious, Inflow Depth = 2.54" for NOAA 2-yr event  
 Inflow = 2.10 cfs @ 12.13 hrs, Volume= 6,279 cf  
 Outflow = 2.05 cfs @ 12.15 hrs, Volume= 6,279 cf, Atten= 3%, Lag= 1.1 min  
 Primary = 0.01 cfs @ 12.15 hrs, Volume= 2 cf  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW  
 Secondary = 0.04 cfs @ 12.15 hrs, Volume= 1,929 cf  
 Routed to Pond 9-PS : BB9 - STONE  
 Tertiary = 2.00 cfs @ 12.15 hrs, Volume= 4,348 cf  
 Routed to Reach 1R : ISOLATOR ROW C

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.80' @ 12.15 hrs Surf.Area= 664 sf Storage= 343 cf

Plug-Flow detention time= 26.2 min calculated for 6,275 cf (100% of inflow)  
 Center-of-Mass det. time= 26.2 min ( 824.4 - 798.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	485 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	190	0	0
9.00	780	485	485

Device	Routing	Invert	Outlet Devices
#1	Primary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.00'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.00'
#3	Device 1	8.80'	<b>24inchDome Grate Capacity</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Tertiary	8.50'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.01 cfs @ 12.15 hrs HW=8.80' (Free Discharge)

↳ **1=Culvert** (Passes 0.01 cfs of 4.32 cfs potential flow)

↳ **3=24inchDome Grate Capacity** (Custom Controls 0.01 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.15 hrs HW=8.80' (Free Discharge)

↳ **2=Exfiltration** ( Controls 0.04 cfs)

**Tertiary OutFlow** Max=1.98 cfs @ 12.15 hrs HW=8.80' (Free Discharge)

↳ **4=15inch-Dome Grate Capacity** (Custom Controls 1.98 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

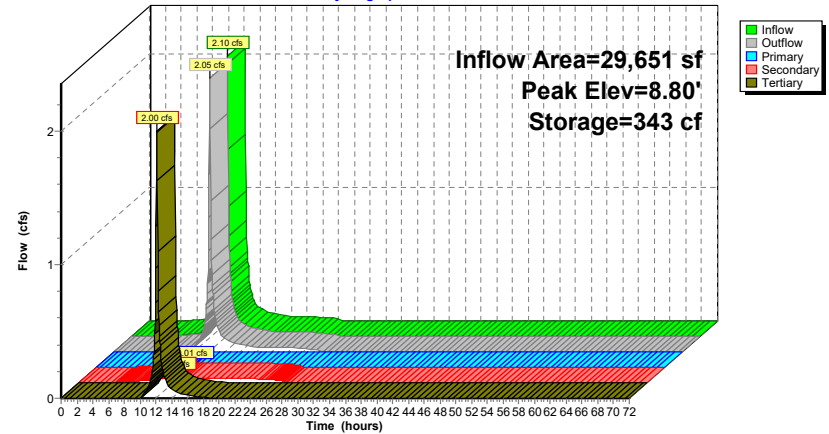
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 292

**Pond 9-P: BB9 - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 293

**Summary for Pond 9-PS: BB9 - STONE**

Inflow = 0.04 cfs @ 12.15 hrs, Volume= 1,929 cf  
 Outflow = 0.04 cfs @ 12.17 hrs, Volume= 1,929 cf, Atten= 1%, Lag= 1.6 min  
 Primary = 0.04 cfs @ 12.17 hrs, Volume= 1,929 cf  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.14' @ 12.17 hrs Surf.Area= 190 sf Storage= 8 cf

Plug-Flow detention time= 3.8 min calculated for 1,928 cf (100% of inflow)  
 Center-of-Mass det. time= 3.8 min ( 988.3 - 984.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	114 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 380 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	190	0	0
8.00	190	380	380

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.17 hrs HW=6.14' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.26 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

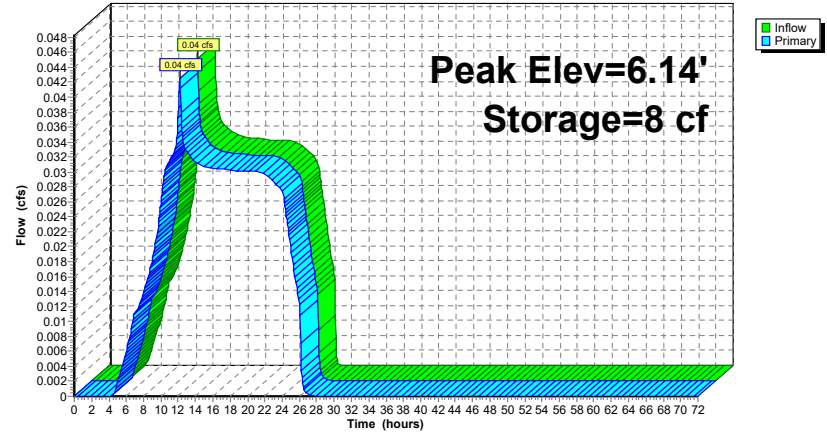
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 294

**Pond 9-PS: BB9 - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 295

**Summary for Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST**

[57] Hint: Peaked at 9.61' (Flood elevation advised)

Inflow Area = 19,582 sf, 58.17% Impervious, Inflow Depth = 2.18" for NOAA 2-yr event  
 Inflow = 1.23 cfs @ 12.13 hrs, Volume= 3,554 cf  
 Outflow = 1.23 cfs @ 12.13 hrs, Volume= 3,554 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 0.57 cfs @ 12.13 hrs, Volume= 2,974 cf  
 Routed to Pond INF-1 : INFILTRATION SYSTEM #1  
 Secondary = 0.66 cfs @ 12.13 hrs, Volume= 580 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.61' @ 12.13 hrs

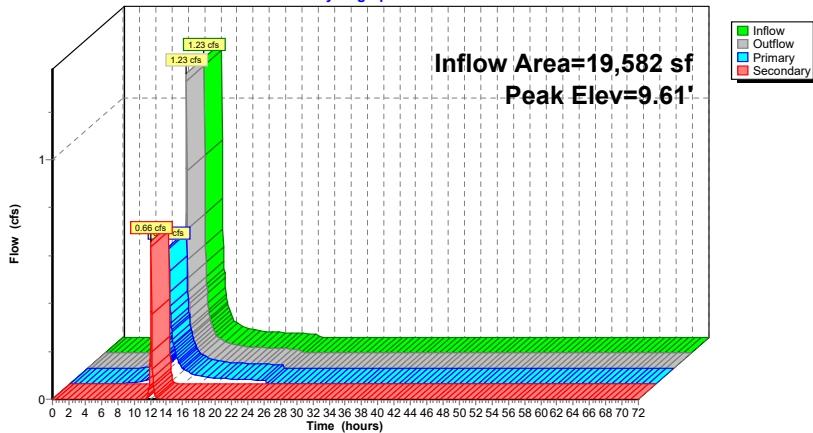
Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>6.0" Vert. WATER QUALITY STORM DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	9.20'	<b>12.0" Vert. LARGE STORM OVEFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.56 cfs @ 12.13 hrs HW=9.60' (Free Discharge)  
 ↳1=WATER QUALITY STORM DIVERSION(Orifice Controls 0.56 cfs @ 2.83 fps)

**Secondary OutFlow** Max=0.62 cfs @ 12.13 hrs HW=9.60' (Free Discharge)  
 ↳2=LARGE STORM OVEFLOW(Orifice Controls 0.62 cfs @ 2.14 fps)

**Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 296

**Summary for Pond DMH2: DIVERSION MANHOLE - HUDSON STREET**

[57] Hint: Peaked at 14.56' (Flood elevation advised)

Inflow Area = 143,309 sf, 49.69% Impervious, Inflow Depth = 2.01" for NOAA 2-yr event  
 Inflow = 8.38 cfs @ 12.13 hrs, Volume= 24,005 cf  
 Outflow = 8.38 cfs @ 12.13 hrs, Volume= 24,005 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 1.83 cfs @ 12.13 hrs, Volume= 15,418 cf  
 Routed to Pond INF-2 : INFILTRATION SYSTEM #2  
 Secondary = 6.55 cfs @ 12.13 hrs, Volume= 8,588 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 14.56' @ 12.13 hrs

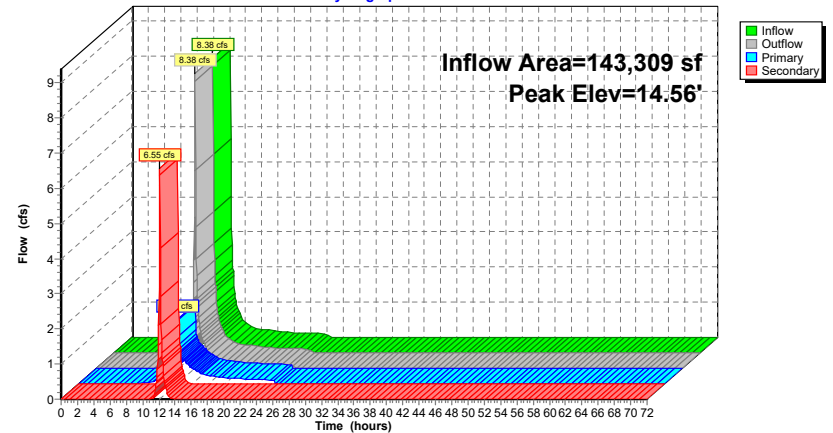
Device	Routing	Invert	Outlet Devices
#1	Primary	10.60'	<b>6.0" Vert. WATER QUALITY STORM DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	11.10'	<b>12.0" Vert. LARGE STORM OVERFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=1.77 cfs @ 12.13 hrs HW=14.35' (Free Discharge)  
 ↳1=WATER QUALITY STORM DIVERSION(Orifice Controls 1.77 cfs @ 9.01 fps)

**Secondary OutFlow** Max=6.27 cfs @ 12.13 hrs HW=14.35' (Free Discharge)  
 ↳2=LARGE STORM OVERFLOW(Orifice Controls 6.27 cfs @ 7.99 fps)

**Pond DMH2: DIVERSION MANHOLE - HUDSON STREET**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 297

**Summary for Pond DMH3: DIVERSION MANHOLE - PORTLAND ST**

[57] Hint: Peaked at 11.54' (Flood elevation advised)

Inflow Area = 19,743 sf, 50.83% Impervious, Inflow Depth = 2.01" for NOAA 2-yr event  
 Inflow = 1.15 cfs @ 12.13 hrs, Volume= 3,307 cf  
 Outflow = 1.15 cfs @ 12.13 hrs, Volume= 3,307 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 0.75 cfs @ 12.13 hrs, Volume= 2,972 cf  
 Routed to Pond INF3 : INFILTRATION SYSTEM #1  
 Secondary = 0.40 cfs @ 12.13 hrs, Volume= 335 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

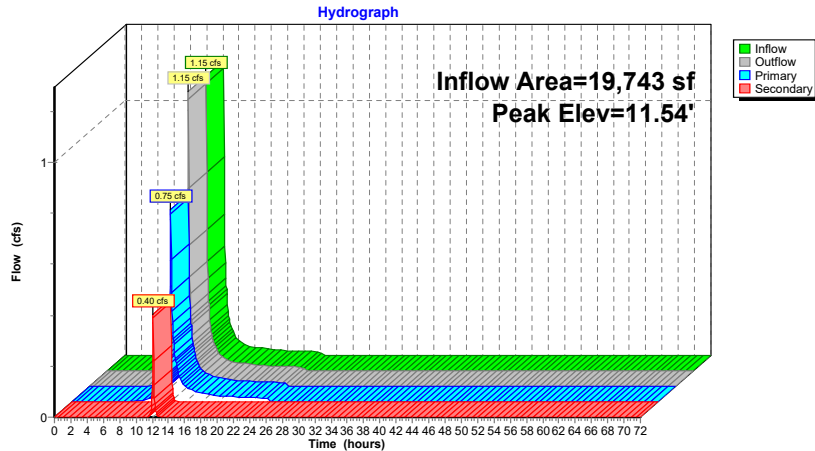
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.54' @ 12.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	11.00'	<b>8.0" Vert. WATER QUALITY DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	11.20'	<b>10.0" Vert. LARGE STORM OVERFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.73 cfs @ 12.13 hrs HW=11.52' (Free Discharge)  
 ↳1=WATER QUALITY DIVERSION(Orifice Controls 0.73 cfs @ 2.47 fps)

**Secondary OutFlow** Max=0.38 cfs @ 12.13 hrs HW=11.52' (Free Discharge)  
 ↳2=LARGE STORM OVERFLOW(Orifice Controls 0.38 cfs @ 1.94 fps)

**Pond DMH3: DIVERSION MANHOLE - PORTLAND ST**



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 298

**Summary for Pond INF-1: INFILTRATION SYSTEM #1**

[79] Warning: Submerged Pond DMH1 Primary device # 1 by 0.01'

Inflow Area = 19,582 sf, 58.17% Impervious, Inflow Depth = 1.82" for NOAA 2-yr event  
 Inflow = 0.57 cfs @ 12.13 hrs, Volume= 2,974 cf  
 Outflow = 0.07 cfs @ 13.69 hrs, Volume= 2,974 cf, Atten= 88%, Lag= 93.8 min  
 Discarded = 0.07 cfs @ 13.69 hrs, Volume= 2,974 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.01' @ 13.69 hrs Surf.Area= 1,772 sf Storage= 1,232 cf

Plug-Flow detention time= 185.7 min calculated for 2,972 cf (100% of inflow)  
 Center-of-Mass det. time= 185.6 min ( 1,018.3 - 832.7 )

Volume	Invert	Avail.Storage	Storage Description
#1A	7.80'	1,091 cf	<b>21.50'W x 81.52'L x 2.33'H Field A</b> 4,090 cf Overall - 973 cf Embedded = 3,117 cf x 35.0% Voids
#2A	8.30'	973 cf	<b>ADS StormTech SC-310 +Cap</b> x 66 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 66 Chambers in 6 Rows
#3	7.80'	137 cf	<b>5.0'D x 7.0'H Vertical Cone/Cylinder</b>
		2,201 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	7.80'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.80'
#2	Primary	8.10'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.10' / 8.00' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.40'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.07 cfs @ 13.69 hrs HW=9.01' (Free Discharge)  
 ↳1=Exfiltration ( Controls 0.07 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=7.80' (Free Discharge)  
 ↳2=Culvert ( Controls 0.00 cfs)  
 ↳3=Sharp-Crested Rectangular Weir( Controls 0.00 cfs)



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 299

**Pond INF-1: INFILTRATION SYSTEM #1 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-310+Cap (ADS StormTech®SC-310 with cap length)**

Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf

Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap

34.0" Wide + 6.0" Spacing = 40.0" C-C Row Spacing

11 Chambers/Row x 7.12' Long +0.60' Cap Length x 2 = 79.52' Row Length +12.0" End Stone x 2 = 81.52' Base Length

6 Rows x 34.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 21.50' Base Width

6.0" Stone Base + 16.0" Chamber Height + 6.0" Stone Cover = 2.33' Field Height

66 Chambers x 14.7 cf = 973.0 cf Chamber Storage

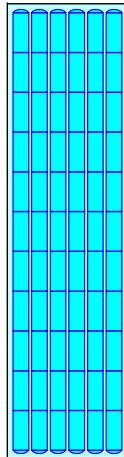
4,089.6 cf Field - 973.0 cf Chambers = 3,116.6 cf Stone x 35.0% Voids = 1,090.8 cf Stone Storage

Chamber Storage + Stone Storage = 2,063.8 cf = 0.047 af

Overall Storage Efficiency = 50.5%

Overall System Size = 81.52' x 21.50' x 2.33'

66 Chambers  
151.5 cy Field  
115.4 cy Stone



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

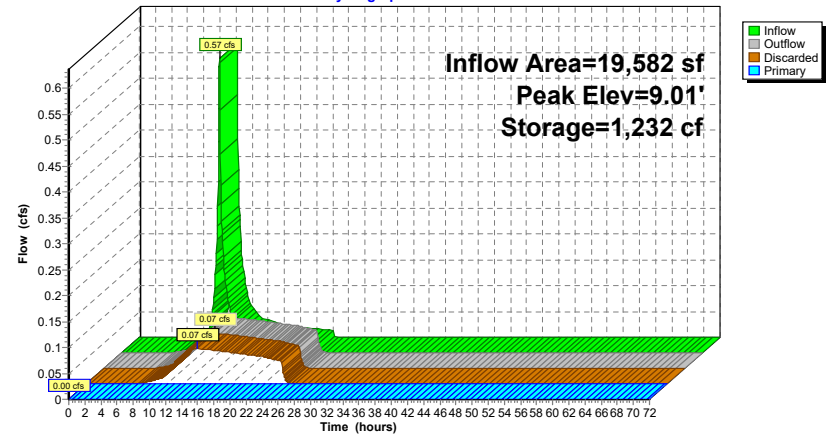
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 300

**Pond INF-1: INFILTRATION SYSTEM #1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 301

**Summary for Pond INF-2: INFILTRATION SYSTEM #2**

Inflow Area = 143,309 sf, 49.69% Impervious, Inflow Depth = 1.29" for NOAA 2-yr event  
 Inflow = 1.83 cfs @ 12.13 hrs, Volume= 15,418 cf  
 Outflow = 1.53 cfs @ 12.20 hrs, Volume= 15,418 cf, Atten= 16%, Lag= 3.8 min  
 Discarded = 0.11 cfs @ 12.20 hrs, Volume= 8,554 cf  
 Primary = 1.42 cfs @ 12.20 hrs, Volume= 6,864 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.70' @ 12.20 hrs Surf.Area= 2,268 sf Storage= 3,280 cf

Plug-Flow detention time= 204.5 min calculated for 15,407 cf (100% of inflow)  
 Center-of-Mass det. time= 204.8 min ( 1,079.0 - 874.2 )

Volume	Invert	Avail.Storage	Storage Description
#1A	7.50'	1,790 cf	<b>25.25'W x 89.06'L x 3.50'H Field A</b> 7,870 cf Overall - 2,756 cf Embedded = 5,114 cf x 35.0% Voids
#2A	8.00'	2,756 cf	<b>ADS_StormTech SC-740 +Cap</b> x 60 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 60 Chambers in 5 Rows
#3	7.50'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		4,684 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	7.50'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.50'
#2	Primary	8.00'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.50'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.11 cfs @ 12.20 hrs HW=9.69' (Free Discharge)  
 ↳1=Exfiltration ( Controls 0.11 cfs)

**Primary OutFlow** Max=1.39 cfs @ 12.20 hrs HW=9.69' (Free Discharge)  
 ↳2=Culvert (Passes 1.39 cfs of 2.97 cfs potential flow)  
 ↳3=Sharp-Crested Rectangular Weir(Weir Controls 1.39 cfs @ 1.44 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 302

**Pond INF-2: INFILTRATION SYSTEM #2 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-740 +Cap (ADS StormTech®SC-740 with cap length)**

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf

Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

12 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 87.06' Row Length +12.0" End Stone x 2 = 89.06' Base Length

5 Rows x 51.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.25' Base Width

6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

60 Chambers x 45.9 cf = 2,756.4 cf Chamber Storage

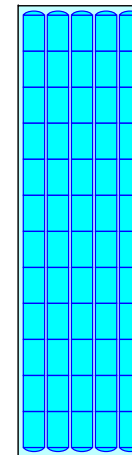
7,870.4 cf Field - 2,756.4 cf Chambers = 5,114.0 cf Stone x 35.0% Voids = 1,789.9 cf Stone Storage

Chamber Storage + Stone Storage = 4,546.3 cf = 0.104 af

Overall Storage Efficiency = 57.8%

Overall System Size = 89.06' x 25.25' x 3.50'

60 Chambers  
 291.5 cy Field  
 189.4 cy Stone



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

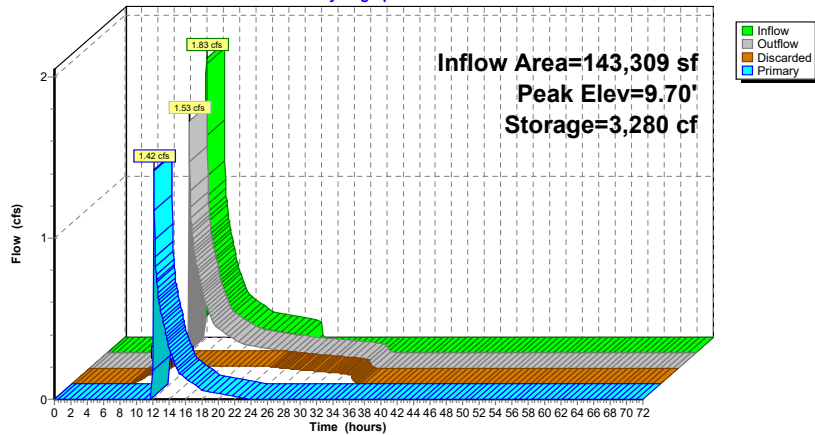
NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Printed 4/6/2022

Page 303

**Pond INF-2: INFILTRATION SYSTEM #2**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Printed 4/6/2022

Page 304

**Summary for Pond INF3: INFILTRATION SYSTEM #1**

Inflow Area = 19,743 sf, 50.83% Impervious, Inflow Depth = 1.81" for NOAA 2-yr event  
 Inflow = 0.75 cfs @ 12.13 hrs, Volume= 2,972 cf  
 Outflow = 0.38 cfs @ 12.32 hrs, Volume= 2,972 cf, Atten= 50%, Lag= 11.1 min  
 Discarded = 0.05 cfs @ 12.32 hrs, Volume= 2,339 cf  
 Primary = 0.33 cfs @ 12.32 hrs, Volume= 633 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.57' @ 12.32 hrs Surf.Area= 1,113 sf Storage= 944 cf

Plug-Flow detention time= 182.2 min calculated for 2,970 cf (100% of inflow)  
 Center-of-Mass det. time= 182.2 min ( 1,016.0 - 833.8 )

Volume	Invert	Avail.Storage	Storage Description
#1A	8.10'	686 cf	<b>18.17'W x 60.16'L x 2.33'H Field A</b> 2,550 cf Overall - 590 of Embedded = 1,960 cf x 35.0% Voids
#2A	8.60'	590 cf	<b>ADS_StormTech SC-310 +Cap x 40 Inside #1</b> Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 40 Chambers in 5 Rows
#3	8.10'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		1,413 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	8.10'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.10'
#2	Primary	8.40'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.40' / 8.30' S= 0.0100'/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.50'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.05 cfs @ 12.32 hrs HW=9.57' (Free Discharge)  
 ↳1=Exfiltration ( Controls 0.05 cfs)

**Primary OutFlow** Max=0.31 cfs @ 12.32 hrs HW=9.57' (Free Discharge)  
 ↳2=Culvert (Passes 0.31 cfs of 2.13 cfs potential flow)  
 ↳3=Sharp-Crested Rectangular Weir(Weir Controls 0.31 cfs @ 0.87 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 305

**Pond INF3: INFILTRATION SYSTEM #1 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-310+Cap (ADS StormTech®SC-310 with cap length)**

Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf

Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap

34.0" Wide + 6.0" Spacing = 40.0" C-C Row Spacing

8 Chambers/Row x 7.12' Long +0.60' Cap Length x 2 = 58.16' Row Length +12.0" End Stone x 2 = 60.16' Base Length

5 Rows x 34.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 18.17' Base Width

6.0" Stone Base + 16.0" Chamber Height + 6.0" Stone Cover = 2.33' Field Height

40 Chambers x 14.7 cf = 589.7 cf Chamber Storage

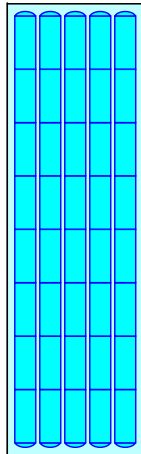
2,550.1 cf Field - 589.7 cf Chambers = 1,960.4 cf Stone x 35.0% Voids = 686.2 cf Stone Storage

Chamber Storage + Stone Storage = 1,275.8 cf = 0.029 af

Overall Storage Efficiency = 50.0%

Overall System Size = 60.16' x 18.17' x 2.33'

40 Chambers  
94.4 cy Field  
72.6 cy Stone



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 2-yr Rainfall=3.40"

Prepared by {enter your company name here}

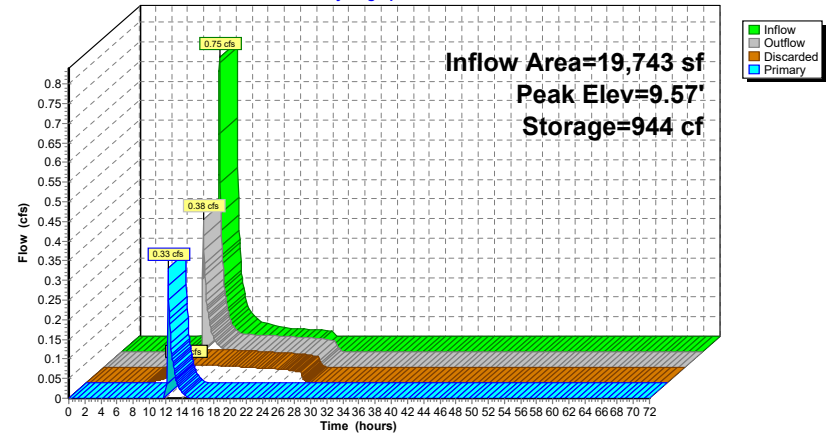
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 306

**Pond INF3: INFILTRATION SYSTEM #1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 307

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: BB-1</b>	Runoff Area=38,826 sf 51.66% Impervious Runoff Depth=4.45" Tc=6.0 min CN=86 Runoff=4.84 cfs 14,390 cf
<b>Subcatchment2a: BB-2a</b>	Runoff Area=3,116 sf 92.62% Impervious Runoff Depth=5.57" Tc=6.0 min CN=96 Runoff=0.44 cfs 1,446 cf
<b>Subcatchment2b: BB-2b</b>	Runoff Area=21,490 sf 80.50% Impervious Runoff Depth=5.22" Tc=6.0 min CN=93 Runoff=2.97 cfs 9,352 cf
<b>Subcatchment3A: BB-3A</b>	Runoff Area=10,987 sf 58.16% Impervious Runoff Depth=4.66" Tc=6.0 min CN=88 Runoff=1.42 cfs 4,271 cf
<b>Subcatchment3B: BB-3B</b>	Runoff Area=4,545 sf 77.34% Impervious Runoff Depth=5.22" Tc=6.0 min CN=93 Runoff=0.63 cfs 1,978 cf
<b>Subcatchment4A: BB-4A</b>	Runoff Area=4,843 sf 86.37% Impervious Runoff Depth=5.45" Tc=6.0 min CN=95 Runoff=0.68 cfs 2,200 cf
<b>Subcatchment4B: BB-4B</b>	Runoff Area=3,048 sf 86.09% Impervious Runoff Depth=5.45" Tc=6.0 min CN=95 Runoff=0.43 cfs 1,385 cf
<b>Subcatchment5A: BB-5A</b>	Runoff Area=3,072 sf 73.44% Impervious Runoff Depth=5.11" Tc=6.0 min CN=92 Runoff=0.42 cfs 1,308 cf
<b>Subcatchment5B: BB-5B</b>	Runoff Area=34,755 sf 71.39% Impervious Runoff Depth=5.00" Tc=6.0 min CN=91 Runoff=4.69 cfs 14,471 cf
<b>Subcatchment6A: BB-6A</b>	Runoff Area=15,148 sf 46.97% Impervious Runoff Depth=4.34" Tc=6.0 min CN=85 Runoff=1.85 cfs 5,479 cf
<b>Subcatchment6B: BB-6B</b>	Runoff Area=6,495 sf 77.45% Impervious Runoff Depth=5.22" Tc=6.0 min CN=93 Runoff=0.90 cfs 2,826 cf
<b>Subcatchment7A: BB-7A</b>	Runoff Area=3,165 sf 87.74% Impervious Runoff Depth=5.45" Tc=6.0 min CN=95 Runoff=0.45 cfs 1,438 cf
<b>Subcatchment7B: BB-7B</b>	Runoff Area=4,942 sf 88.73% Impervious Runoff Depth=5.45" Tc=6.0 min CN=95 Runoff=0.70 cfs 2,245 cf
<b>Subcatchment8A: BB-8A</b>	Runoff Area=3,978 sf 79.99% Impervious Runoff Depth=5.22" Tc=6.0 min CN=93 Runoff=0.55 cfs 1,731 cf
<b>Subcatchment8B: BB-8B</b>	Runoff Area=5,598 sf 87.78% Impervious Runoff Depth=5.45" Tc=6.0 min CN=95 Runoff=0.79 cfs 2,543 cf
<b>Subcatchment9: BB-9</b>	Runoff Area=29,651 sf 74.77% Impervious Runoff Depth=5.11" Tc=6.0 min CN=92 Runoff=4.05 cfs 12,623 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 308

<b>SubcatchmentCB-1: New CB South</b>	Runoff Area=19,582 sf 58.17% Impervious Runoff Depth=4.66" Flow Length=512' Tc=6.0 min CN=88 Runoff=2.53 cfs 7,612 cf
<b>SubcatchmentCB-5: PORTLANDST</b>	Runoff Area=19,743 sf 50.83% Impervious Runoff Depth=4.45" Flow Length=574' Tc=6.0 min CN=86 Runoff=2.46 cfs 7,317 cf
<b>SubcatchmentCB3: NEW CB SOUTH-</b>	Runoff Area=25,183 sf 51.84% Impervious Runoff Depth=4.45" Flow Length=635' Tc=6.0 min CN=86 Runoff=3.14 cfs 9,333 cf
<b>SubcatchmentCB4: NEW CB NOTH -</b>	Runoff Area=118,126 sf 49.24% Impervious Runoff Depth=4.45" Flow Length=822' Tc=6.0 min CN=86 Runoff=14.73 cfs 43,780 cf
<b>Reach 1R: ISOLATORROW C</b>	Inflow=2.68 cfs 9,749 cf Outflow=2.68 cfs 9,749 cf
<b>Reach 6R: ISOLATORROW 2</b>	Inflow=2.33 cfs 8,999 cf Outflow=2.33 cfs 8,999 cf
<b>Reach 15R: ISOLATORROW 1</b>	Inflow=5.27 cfs 20,413 cf Outflow=5.27 cfs 20,413 cf
<b>Reach B: PARKING LOT B OVERFLOW</b>	Inflow=24.91 cfs 66,837 cf Outflow=24.91 cfs 66,837 cf
<b>Reach BMP4_O: BMP-4 OVERFLOW</b>	Inflow=0.46 cfs 2,648 cf Outflow=0.46 cfs 2,648 cf
<b>Reach BMP6_O: BMP-6 OVERFLOW</b>	Inflow=2.67 cfs 7,136 cf Outflow=2.67 cfs 7,136 cf
<b>Reach BMP7_O: BMP-7 OVERFLOW</b>	Inflow=1.12 cfs 3,683 cf Outflow=1.12 cfs 3,683 cf
<b>Reach BMP9_O: BMP-9 OVERFLOW</b>	Inflow=3.97 cfs 12,623 cf Outflow=3.97 cfs 12,623 cf
<b>Reach BMP_3: BMP-3_OVERFLOW</b>	Inflow=2.56 cfs 4,877 cf Outflow=2.56 cfs 4,877 cf
<b>Reach DP-1: French Rodney Blvd 14" Outfall</b>	Inflow=9.67 cfs 25,417 cf Outflow=9.67 cfs 25,417 cf
<b>Reach DP-2: NORTHERN OUTFALL</b>	Inflow=36.33 cfs 98,692 cf Outflow=36.33 cfs 98,692 cf
<b>Reach H ST: HUDSON STREET DRAINAGE</b>	Inflow=29.64 cfs 82,004 cf Outflow=29.64 cfs 82,004 cf
<b>Reach P ST: PORTLAND STREET DRAINAGE</b>	Inflow=6.69 cfs 16,689 cf Outflow=6.69 cfs 16,689 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 309

**Pond 1-P: BB 1** Peak Elev=10.18' Storage=1,085 cf Inflow=4.84 cfs 14,390 cf  
Discarded=0.05 cfs 2,687 cf Primary=2.39 cfs 1,961 cf Secondary=2.27 cfs 9,742 cf Outflow=4.72 cfs 14,390 cf

**Pond 2a-P: BB 2a** Peak Elev=8.20' Storage=82 cf Inflow=0.44 cfs 1,446 cf  
Primary=0.43 cfs 1,410 cf Secondary=0.00 cfs 0 cf Outflow=0.43 cfs 1,410 cf

**Pond 2b-P: BB 2b** Peak Elev=8.54' Storage=288 cf Inflow=2.97 cfs 9,352 cf  
Primary=2.57 cfs 9,261 cf Secondary=0.30 cfs 55 cf Outflow=2.87 cfs 9,316 cf

**Pond 3A-P: BB 3A** Peak Elev=11.10' Storage=472 cf Inflow=1.42 cfs 4,271 cf  
Discarded=0.03 cfs 1,429 cf Primary=1.34 cfs 2,842 cf Outflow=1.36 cfs 4,271 cf

**Pond 3B-P: BB 3B** Peak Elev=12.90' Storage=260 cf Inflow=0.63 cfs 1,978 cf  
Discarded=0.02 cfs 880 cf Primary=0.60 cfs 1,098 cf Outflow=0.62 cfs 1,978 cf

**Pond 4A-P: BB 4A - POND** Peak Elev=10.07' Storage=239 cf Inflow=0.68 cfs 2,200 cf  
Primary=0.63 cfs 937 cf Secondary=0.04 cfs 1,263 cf Outflow=0.66 cfs 2,200 cf

**Pond 4A-S: BB4A-Stone** Peak Elev=6.13' Storage=9 cf Inflow=0.04 cfs 1,263 cf  
Outflow=0.04 cfs 1,263 cf

**Pond 4B-P: BB 4B - POND** Peak Elev=11.04' Storage=136 cf Inflow=0.43 cfs 1,385 cf  
Primary=0.40 cfs 597 cf Secondary=0.02 cfs 787 cf Outflow=0.42 cfs 1,385 cf

**Pond 4B-S: BB 4A-Stone** Peak Elev=6.10' Storage=4 cf Inflow=0.02 cfs 787 cf  
Outflow=0.02 cfs 787 cf

**Pond 5A-P: BB 5A - POND** Peak Elev=9.52' Storage=431 cf Inflow=0.42 cfs 1,308 cf  
Primary=0.07 cfs 72 cf Secondary=0.05 cfs 1,236 cf Outflow=0.12 cfs 1,308 cf

**Pond 5A-PS: BB 5A-Stone** Peak Elev=6.14' Storage=21 cf Inflow=0.05 cfs 1,236 cf  
Outflow=0.05 cfs 1,236 cf

**Pond 5B-P: BB 5B - POND** Peak Elev=9.07' Storage=716 cf Inflow=4.69 cfs 14,471 cf  
Primary=2.20 cfs 1,514 cf Secondary=0.08 cfs 3,958 cf Tertiary=2.33 cfs 8,999 cf Outflow=4.61 cfs 14,471 cf

**Pond 5B-PS: BB 5B-Stone** Peak Elev=6.00' Storage=1 cf Inflow=0.08 cfs 3,958 cf  
Outflow=0.08 cfs 3,958 cf

**Pond 6A-P: BB 6A - POND** Peak Elev=11.03' Storage=443 cf Inflow=1.85 cfs 5,479 cf  
Primary=1.78 cfs 3,305 cf Secondary=0.05 cfs 2,174 cf Outflow=1.83 cfs 5,479 cf

**Pond 6A-PS: BB 6A - STONE** Peak Elev=6.14' Storage=13 cf Inflow=0.05 cfs 2,174 cf  
Outflow=0.05 cfs 2,174 cf

**Pond 6B-P: BB 6B** Peak Elev=12.10' Storage=393 cf Inflow=0.90 cfs 2,826 cf  
Discarded=0.02 cfs 1,170 cf Primary=0.84 cfs 1,657 cf Outflow=0.86 cfs 2,826 cf

**Pond 7A-P: BB 7A PONDING** Peak Elev=9.99' Storage=153 cf Inflow=0.45 cfs 1,438 cf  
Primary=0.42 cfs 638 cf Secondary=0.02 cfs 800 cf Outflow=0.44 cfs 1,438 cf

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 310

**Pond 7A-S: BB 7A - STONE** Peak Elev=5.19' Storage=4 cf Inflow=0.02 cfs 800 cf  
Outflow=0.02 cfs 800 cf

**Pond 7B-P: BB 7B PONDING** Peak Elev=10.72' Storage=282 cf Inflow=0.70 cfs 2,245 cf  
Primary=0.65 cfs 940 cf Secondary=0.03 cfs 1,305 cf Outflow=0.68 cfs 2,245 cf

**Pond 7B-S: BB 7B - STONE** Peak Elev=5.22' Storage=5 cf Inflow=0.03 cfs 1,305 cf  
Outflow=0.03 cfs 1,305 cf

**Pond 8a-P: BB 8A PONDING** Peak Elev=9.10' Storage=295 cf Inflow=0.55 cfs 1,731 cf  
Primary=0.48 cfs 547 cf Secondary=0.04 cfs 1,184 cf Outflow=0.52 cfs 1,731 cf

**Pond 8a-s: BB 8A - STONE** Peak Elev=4.53' Storage=12 cf Inflow=0.04 cfs 1,184 cf  
Outflow=0.04 cfs 1,184 cf

**Pond 8B-P: BB 8B-PONDING** Peak Elev=9.73' Storage=262 cf Inflow=0.79 cfs 2,543 cf  
Primary=0.74 cfs 1,102 cf Secondary=0.04 cfs 1,441 cf Outflow=0.78 cfs 2,543 cf

**Pond 8B-S: BB 8B-Stone** Peak Elev=4.53' Storage=12 cf Inflow=0.04 cfs 1,441 cf  
Outflow=0.04 cfs 1,441 cf

**Pond 9-P: BB9 - POND** Peak Elev=8.99' Storage=476 cf Inflow=4.05 cfs 12,623 cf  
Primary=1.24 cfs 542 cf Secondary=0.05 cfs 2,332 cf Tertiary=2.68 cfs 9,749 cf Outflow=3.97 cfs 12,623 cf

**Pond 9-PS: BB9 - STONE** Peak Elev=6.15' Storage=9 cf Inflow=0.05 cfs 2,332 cf  
Outflow=0.05 cfs 2,332 cf

**Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST** Peak Elev=9.92' Inflow=2.53 cfs 7,612 cf  
Primary=0.78 cfs 5,625 cf Secondary=1.75 cfs 1,987 cf Outflow=2.53 cfs 7,612 cf

**Pond DMH2: DIVERSION MANHOLE - HUDSON STREET** Peak Elev=25.58' Inflow=17.88 cfs 53,113 cf  
Primary=3.65 cfs 28,364 cf Secondary=14.23 cfs 24,749 cf Outflow=17.88 cfs 53,113 cf

**Pond DMH3: DIVERSION MANHOLE - PORTLAND ST** Peak Elev=11.85' Inflow=2.46 cfs 7,317 cf  
Primary=1.21 cfs 6,020 cf Secondary=1.25 cfs 1,297 cf Outflow=2.46 cfs 7,317 cf

**Pond INF-1: INFILTRATIONSYSTEM#1** Peak Elev=9.48' Storage=1,689 cf Inflow=0.78 cfs 5,625 cf  
Discarded=0.08 cfs 4,624 cf Primary=0.37 cfs 1,001 cf Outflow=0.45 cfs 5,625 cf

**Pond INF-2: INFILTRATIONSYSTEM#2** Peak Elev=9.87' Storage=3,518 cf Inflow=3.65 cfs 28,364 cf  
Discarded=0.12 cfs 9,579 cf Primary=3.18 cfs 18,785 cf Outflow=3.30 cfs 28,364 cf

**Pond INF3: INFILTRATIONSYSTEM#1** Peak Elev=9.67' Storage=1,000 cf Inflow=1.21 cfs 6,020 cf  
Discarded=0.05 cfs 3,178 cf Primary=1.15 cfs 2,842 cf Outflow=1.20 cfs 6,020 cf

**Total Runoff Area = 376,293 sf Runoff Volume = 147,727 cf Average Runoff Depth = 4.71"**  
**39.88% Pervious = 150,053 sf 60.12% Impervious = 226,240 sf**

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 311

**Summary for Subcatchment 1: BB-1**

Runoff = 4.84 cfs @ 12.13 hrs, Volume= 14,390 cf, Depth= 4.45"  
Routed to Pond 1-P : BB 1

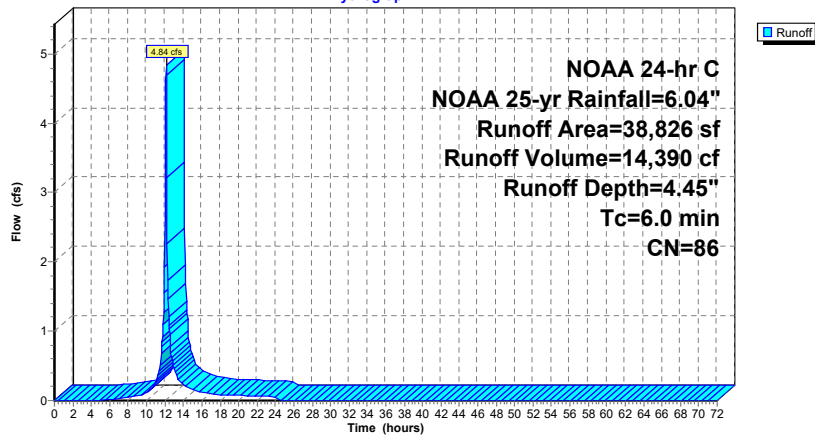
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
27,309	83	1/4 acre lots, 38% imp, HSG C
1,838	74	>75% Grass cover, Good, HSG C
9,679	98	Paved parking, HSG C
38,826	86	Weighted Average
18,770		48.34% Pervious Area
20,056		51.66% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 1: BB-1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 312

**Summary for Subcatchment 2a: BB-2a**

Runoff = 0.44 cfs @ 12.13 hrs, Volume= 1,446 cf, Depth= 5.57"  
Routed to Pond 2a-P : BB 2a

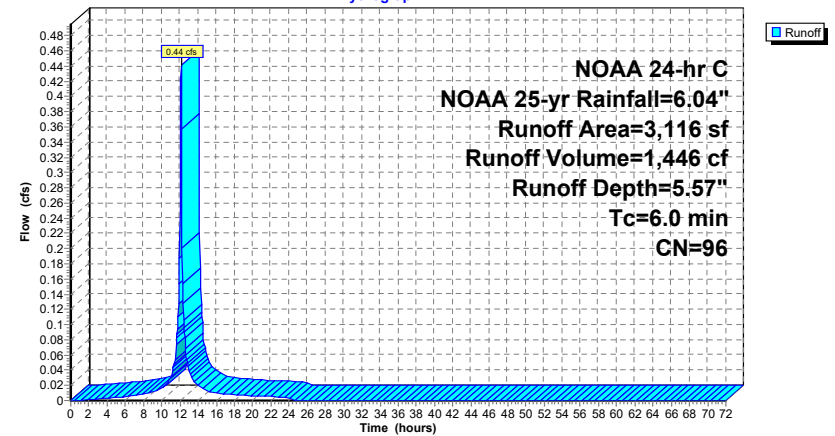
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
230	74	>75% Grass cover, Good, HSG C
2,886	98	Paved parking, HSG C
3,116	96	Weighted Average
230		7.38% Pervious Area
2,886		92.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2a: BB-2a**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 313

**Summary for Subcatchment 2b: BB-2b**

Runoff = 2.97 cfs @ 12.13 hrs, Volume= 9,352 cf, Depth= 5.22"  
Routed to Pond 2b-P : BB 2b

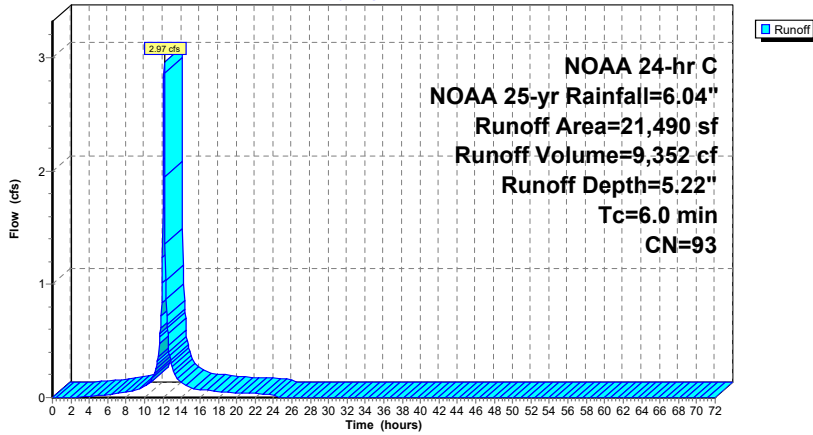
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
3,097	83	1/4 acre lots, 38% imp, HSG C
2,270	74	>75% Grass cover, Good, HSG C
16,123	98	Paved parking, HSG C
21,490	93	Weighted Average
4,190		19.50% Pervious Area
17,300		80.50% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 2b: BB-2b**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 314

**Summary for Subcatchment 3A: BB-3A**

Runoff = 1.42 cfs @ 12.13 hrs, Volume= 4,271 cf, Depth= 4.66"  
Routed to Pond 3A-P : BB 3A

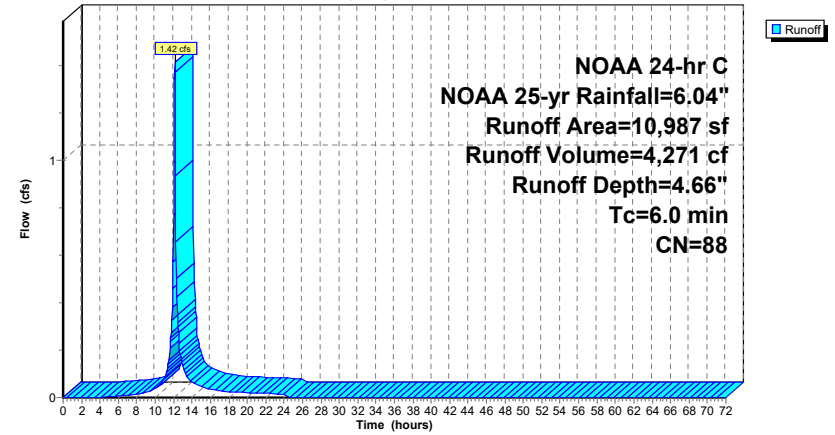
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
5,791	83	1/4 acre lots, 38% imp, HSG C
1,007	74	>75% Grass cover, Good, HSG C
4,189	98	Paved parking, HSG C
10,987	88	Weighted Average
4,597		41.84% Pervious Area
6,390		58.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3A: BB-3A**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 315

**Summary for Subcatchment 3B: BB-3B**

Runoff = 0.63 cfs @ 12.13 hrs, Volume= 1,978 cf, Depth= 5.22"  
Routed to Pond 3B-P : BB 3B

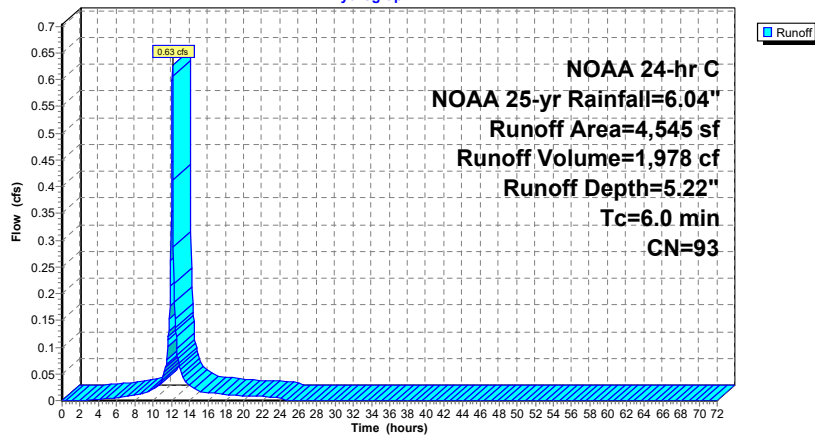
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
* 1,030	74	>75% Grass cover, Good, HSG C
3,515	98	Paved parking, HSG C
4,545	93	Weighted Average
1,030		22.66% Pervious Area
3,515		77.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 3B: BB-3B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 316

**Summary for Subcatchment 4A: BB-4A**

Runoff = 0.68 cfs @ 12.13 hrs, Volume= 2,200 cf, Depth= 5.45"  
Routed to Pond 4A-P : BB 4A - POND

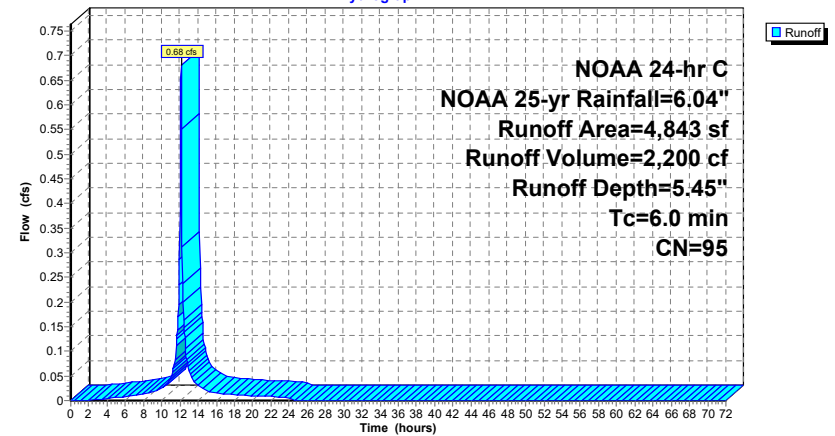
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
660	74	>75% Grass cover, Good, HSG C
4,183	98	Paved parking, HSG C
4,843	95	Weighted Average
660		13.63% Pervious Area
4,183		86.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 4A: BB-4A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 317

**Summary for Subcatchment 4B: BB-4B**

Runoff = 0.43 cfs @ 12.13 hrs, Volume= 1,385 cf, Depth= 5.45"  
Routed to Pond 4B-P : BB 4B - POND

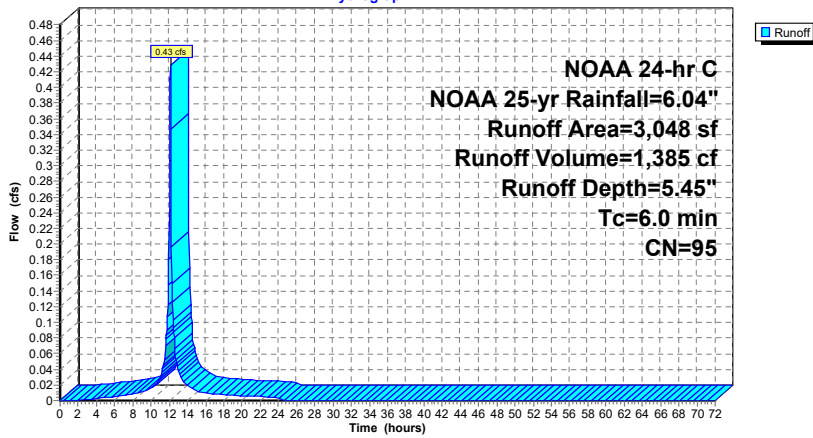
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
424	74	>75% Grass cover, Good, HSG C
2,624	98	Paved parking, HSG C
3,048	95	Weighted Average
424		13.91% Pervious Area
2,624		86.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 4B: BB-4B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 318

**Summary for Subcatchment 5A: BB-5A**

Runoff = 0.42 cfs @ 12.13 hrs, Volume= 1,308 cf, Depth= 5.11"  
Routed to Pond 5A-P : BB 5A - POND

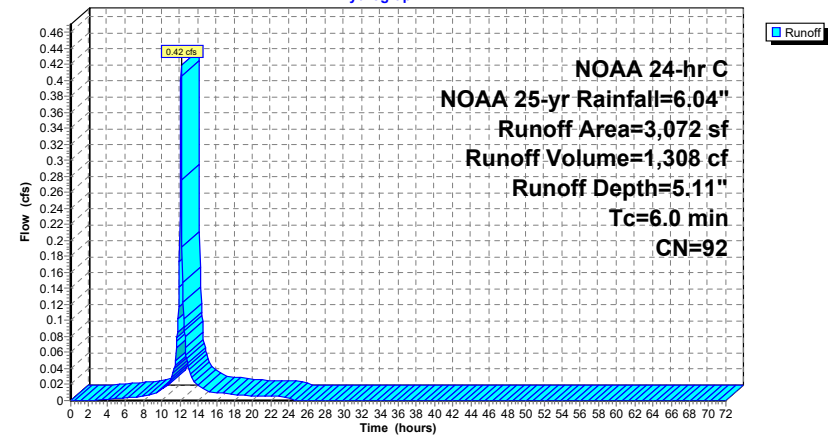
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
816	74	>75% Grass cover, Good, HSG C
2,256	98	Paved parking, HSG C
3,072	92	Weighted Average
816		26.56% Pervious Area
2,256		73.44% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5A: BB-5A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 319

**Summary for Subcatchment 5B: BB-5B**

Runoff = 4.69 cfs @ 12.13 hrs, Volume= 14,471 cf, Depth= 5.00"  
Routed to Pond 5B-P : BB 5B - POND

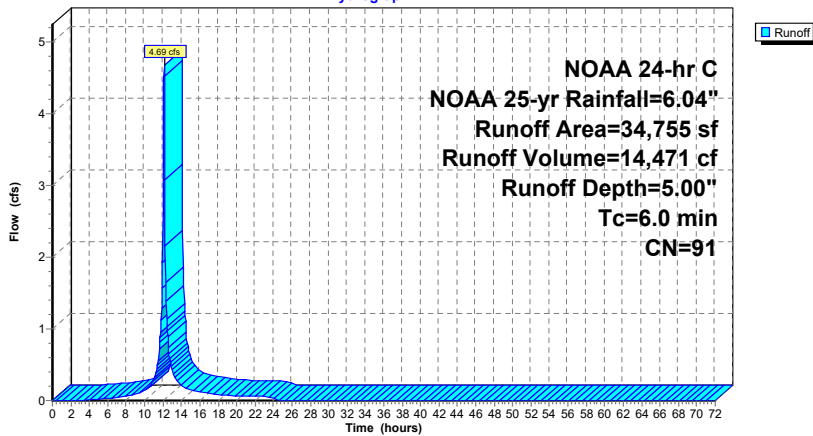
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
12,062	83	1/4 acre lots, 38% imp, HSG C
2,464	74	>75% Grass cover, Good, HSG C
20,229	98	Paved parking, HSG C
34,755	91	Weighted Average
9,942		28.61% Pervious Area
24,813		71.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 5B: BB-5B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 320

**Summary for Subcatchment 6A: BB-6A**

Runoff = 1.85 cfs @ 12.13 hrs, Volume= 5,479 cf, Depth= 4.34"  
Routed to Pond 6A-P : BB 6A - POND

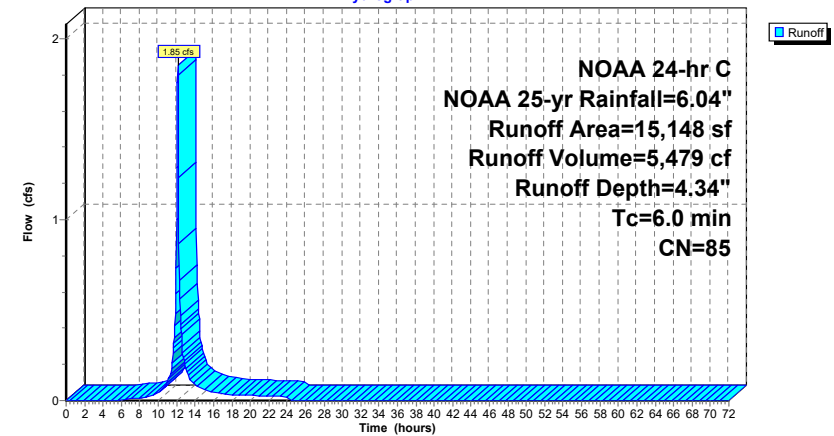
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
11,763	83	1/4 acre lots, 38% imp, HSG C
740	74	>75% Grass cover, Good, HSG C
2,645	98	Paved parking, HSG C
15,148	85	Weighted Average
8,033		53.03% Pervious Area
7,115		46.97% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6A: BB-6A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 321

**Summary for Subcatchment 6B: BB-6B**

Runoff = 0.90 cfs @ 12.13 hrs, Volume= 2,826 cf, Depth= 5.22"  
Routed to Pond 6B-P : BB 6B

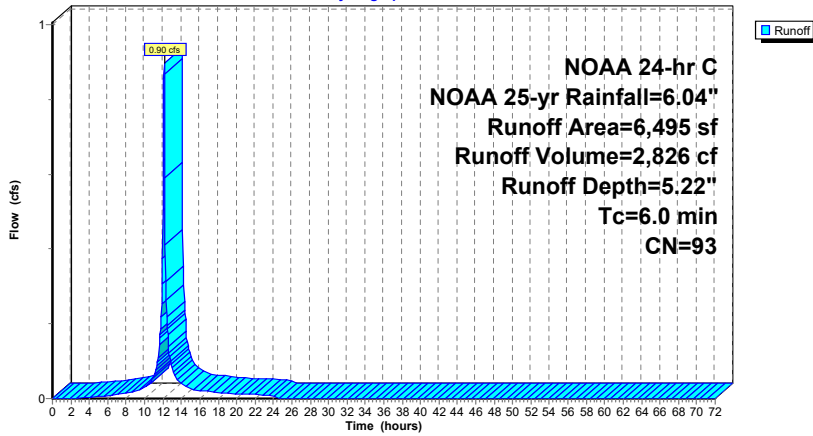
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
1,259	83	1/4 acre lots, 38% imp, HSG C
684	74	>75% Grass cover, Good, HSG C
4,552	98	Paved parking, HSG C
6,495	93	Weighted Average
1,465		22.55% Pervious Area
5,030		77.45% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 6B: BB-6B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 322

**Summary for Subcatchment 7A: BB-7A**

Runoff = 0.45 cfs @ 12.13 hrs, Volume= 1,438 cf, Depth= 5.45"  
Routed to Pond 7A-P : BB 7A PONDING

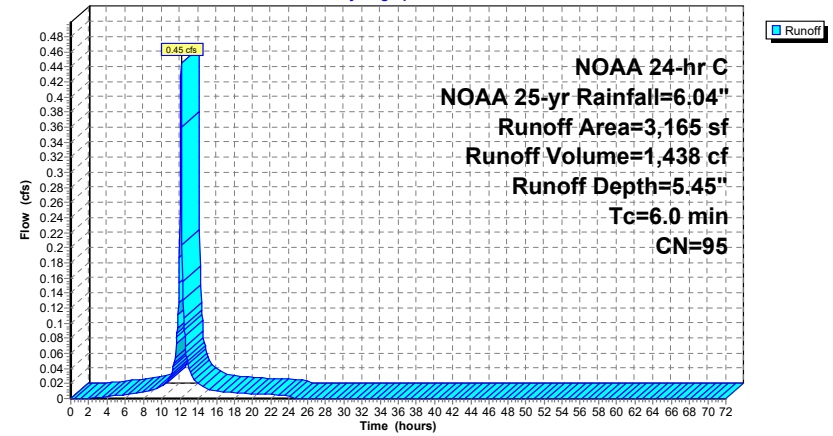
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
388	74	>75% Grass cover, Good, HSG C
2,777	98	Paved parking, HSG C
3,165	95	Weighted Average
388		12.26% Pervious Area
2,777		87.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 7A: BB-7A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 323

**Summary for Subcatchment 7B: BB-7B**

Runoff = 0.70 cfs @ 12.13 hrs, Volume= 2,245 cf, Depth= 5.45"  
 Routed to Pond 7B-P : BB 7B PONDING

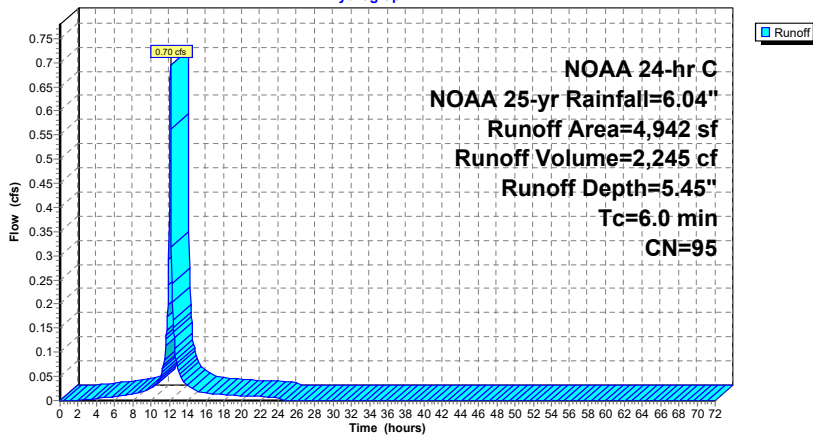
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
557	74	>75% Grass cover, Good, HSG C
4,385	98	Paved parking, HSG C
4,942	95	Weighted Average
557		11.27% Pervious Area
4,385		88.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 7B: BB-7B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 324

**Summary for Subcatchment 8A: BB-8A**

Runoff = 0.55 cfs @ 12.13 hrs, Volume= 1,731 cf, Depth= 5.22"  
 Routed to Pond 8a-P : BB 8A PONDING

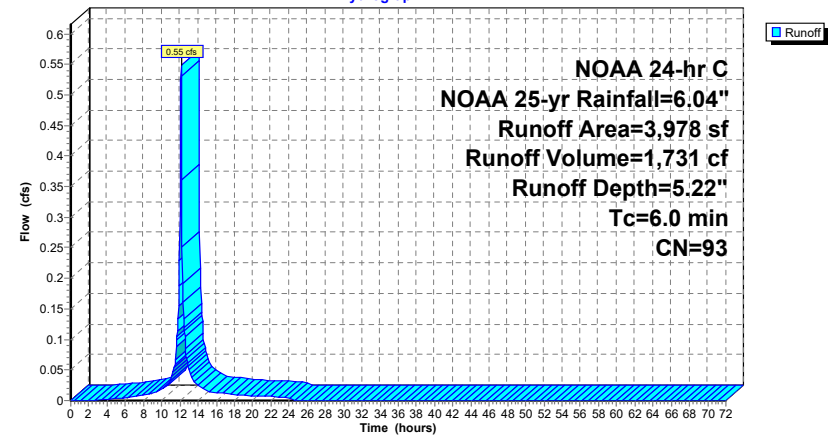
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
796	74	>75% Grass cover, Good, HSG C
3,182	98	Paved parking, HSG C
3,978	93	Weighted Average
796		20.01% Pervious Area
3,182		79.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8A: BB-8A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 325

**Summary for Subcatchment 8B: BB-8B**

Runoff = 0.79 cfs @ 12.13 hrs, Volume= 2,543 cf, Depth= 5.45"  
 Routed to Pond 8B-P : BB 8B-PONDING

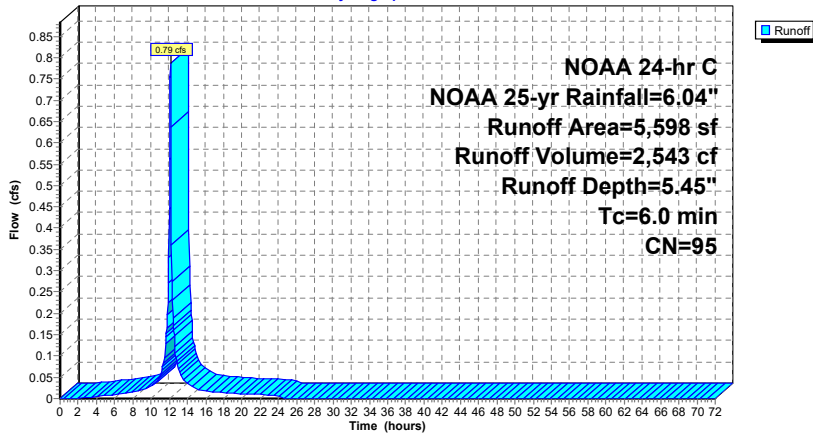
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
0	83	1/4 acre lots, 38% imp, HSG C
684	74	>75% Grass cover, Good, HSG C
4,914	98	Paved parking, HSG C
5,598	95	Weighted Average
684		12.22% Pervious Area
4,914		87.78% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 8B: BB-8B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 326

**Summary for Subcatchment 9: BB-9**

Runoff = 4.05 cfs @ 12.13 hrs, Volume= 12,623 cf, Depth= 5.11"  
 Routed to Pond 9-P : BB9 - POND

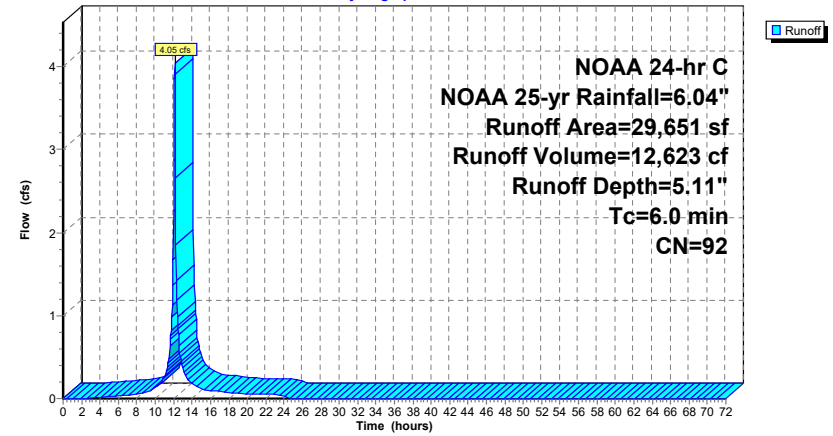
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Area (sf)	CN	Description
8,550	83	1/4 acre lots, 38% imp, HSG C
2,179	74	>75% Grass cover, Good, HSG C
18,922	98	Paved parking, HSG C
29,651	92	Weighted Average
7,480		25.23% Pervious Area
22,171		74.77% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, residential & parking areas

**Subcatchment 9: BB-9**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 327

**Summary for Subcatchment CB-1: New CB South (Seymour Street)**

Runoff = 2.53 cfs @ 12.13 hrs, Volume= 7,612 cf, Depth= 4.66"  
 Routed to Pond DMH1 : DIVERSION MANHOLE - SEYMOUR ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

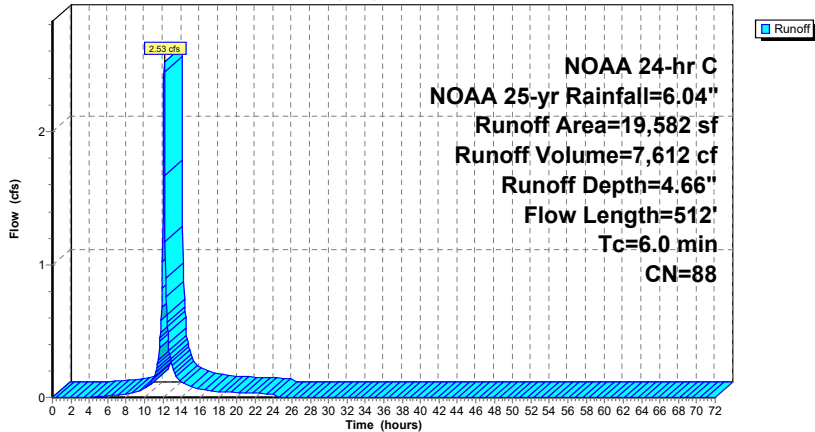
Area (sf)	CN	Description
13,211	83	1/4 acre lots, 38% imp, HSG C
* 6,371	98	Roadway
19,582	88	Weighted Average
8,191		41.83% Pervious Area
11,391		58.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	50	0.0300	1.45		<b>Sheet Flow, A-B</b> Smooth surfaces n= 0.011 P2= 3.40"
2.4	462	0.0249	3.20		<b>Shallow Concentrated Flow, Paved</b> Paved Kv= 20.3 fps
3.0					<b>Direct Entry, Direct entry to 6</b>
6.0	512	Total			

**Subcatchment CB-1: New CB South (Seymour Street)**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 328

**Summary for Subcatchment CB-5: PORTLAND ST SOUTH OFFSITE**

Runoff = 2.46 cfs @ 12.13 hrs, Volume= 7,317 cf, Depth= 4.45"  
 Routed to Pond DMH3 : DIVERSION MANHOLE - PORTLAND ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

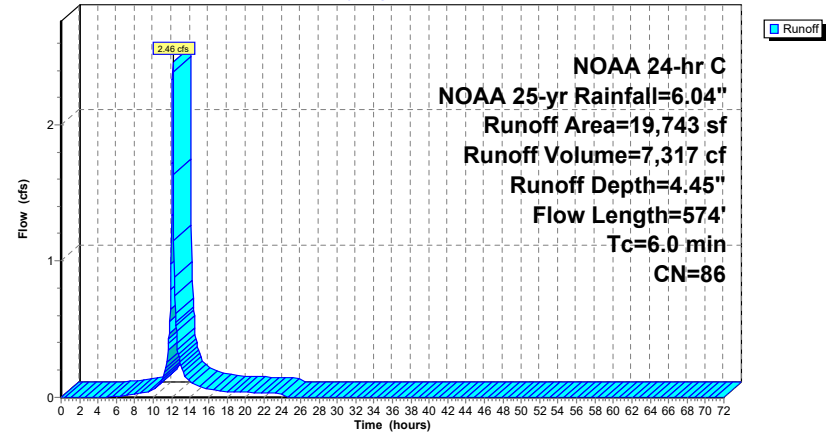
Area (sf)	CN	Description
15,657	83	1/4 acre lots, 38% imp, HSG C
* 4,086	98	Roadway
19,743	86	Weighted Average
9,707		49.17% Pervious Area
10,036		50.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		<b>Sheet Flow, A-B (sheet flow)</b> Smooth surfaces n= 0.011 P2= 3.40"
2.3	524	0.0346	3.78		<b>Shallow Concentrated Flow, B-C (shallow conc.)</b> Paved Kv= 20.3 fps
3.2					<b>Direct Entry, direct to 6</b>
6.0	574	Total			

**Subcatchment CB-5: PORTLAND ST SOUTH OFFSITE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 329

**Summary for Subcatchment CB3: NEW CB SOUTH- HUDSON ST**

Runoff = 3.14 cfs @ 12.13 hrs, Volume= 9,333 cf, Depth= 4.45"  
 Routed to Pond DMH2 : DIVERSION MANHOLE - HUDSON STREET

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

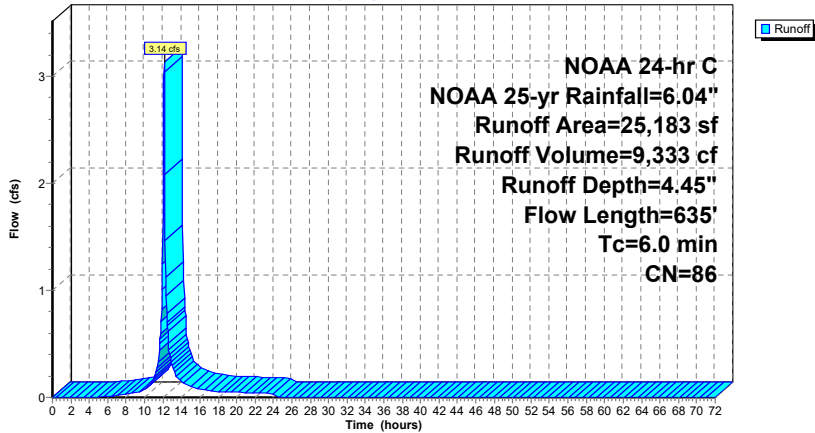
Area (sf)	CN	Description
19,562	83	1/4 acre lots, 38% imp, HSG C
* 5,621	98	Roadway
25,183	86	Weighted Average
12,128		48.16% Pervious Area
13,055		51.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0444	1.70		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
3.0	585	0.0256	3.25		Shallow Concentrated Flow, B-C Paved Kv= 20.3 fps
2.5					Direct Entry, direct entry to 6
6.0	635	Total			

**Subcatchment CB3: NEW CB SOUTH- HUDSON ST**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 330

**Summary for Subcatchment CB4: NEW CB NOTH - HUDSON STREET**

Runoff = 14.73 cfs @ 12.13 hrs, Volume= 43,780 cf, Depth= 4.45"  
 Routed to Pond DMH2 : DIVERSION MANHOLE - HUDSON STREET

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

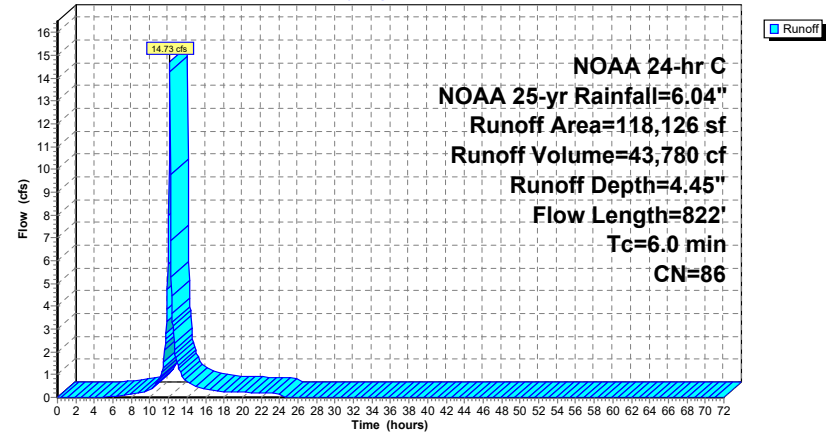
Area (sf)	CN	Description
96,716	83	1/4 acre lots, 38% imp, HSG C
* 21,410	98	Roadway
118,126	86	Weighted Average
59,964		50.76% Pervious Area
58,162		49.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	50	0.0500	1.78		Sheet Flow, A-B (sheet flow) Smooth surfaces n= 0.011 P2= 3.40"
4.0	772	0.0245	3.18		Shallow Concentrated Flow, B-C (shallow concentrated) Paved Kv= 20.3 fps
1.5					Direct Entry, direct entry to 6
6.0	822	Total			

**Subcatchment CB4: NEW CB NOTH - HUDSON STREET**

Hydrograph





**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 331

**Summary for Reach 1R: ISOLATOR ROW C**

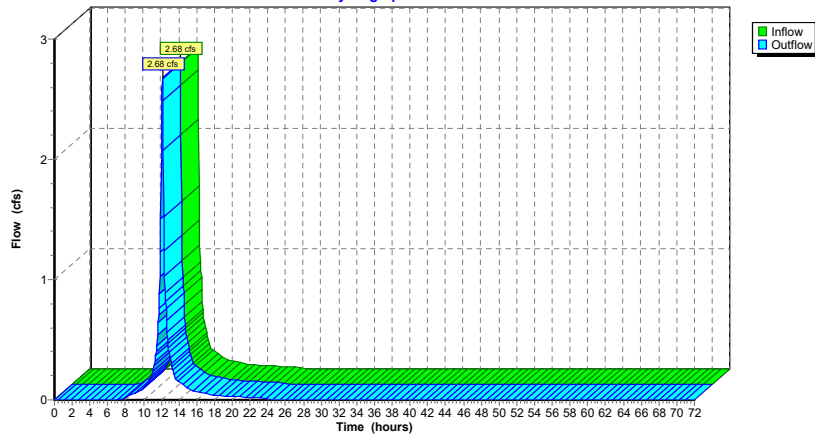
[40] Hint: Not Described (Outflow=Inflow)

Inflow = 2.68 cfs @ 12.14 hrs, Volume= 9,749 cf  
Outflow = 2.68 cfs @ 12.14 hrs, Volume= 9,749 cf, Atten= 0%, Lag= 0.0 min  
Routed to Reach BMP9\_O : BMP-9 OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 1R: ISOLATOR ROW C**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 332

**Summary for Reach 6R: ISOLATOR ROW 2**

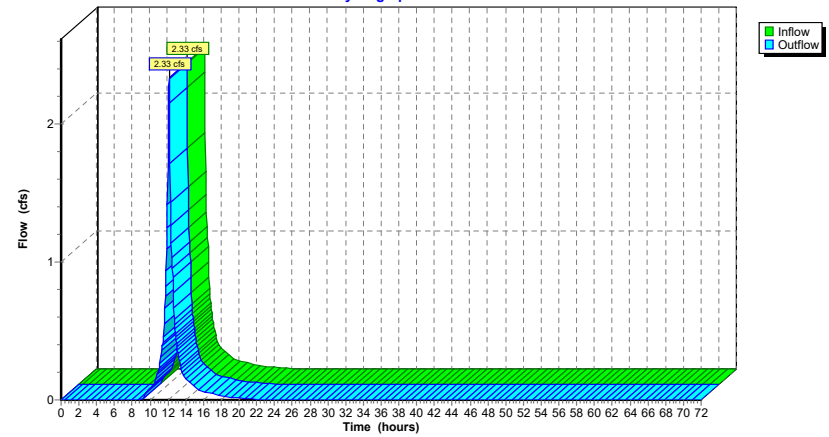
[40] Hint: Not Described (Outflow=Inflow)

Inflow = 2.33 cfs @ 12.14 hrs, Volume= 8,999 cf  
Outflow = 2.33 cfs @ 12.14 hrs, Volume= 8,999 cf, Atten= 0%, Lag= 0.0 min  
Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 6R: ISOLATOR ROW 2**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 333

**Summary for Reach 15R: ISOLATOR ROW 1**

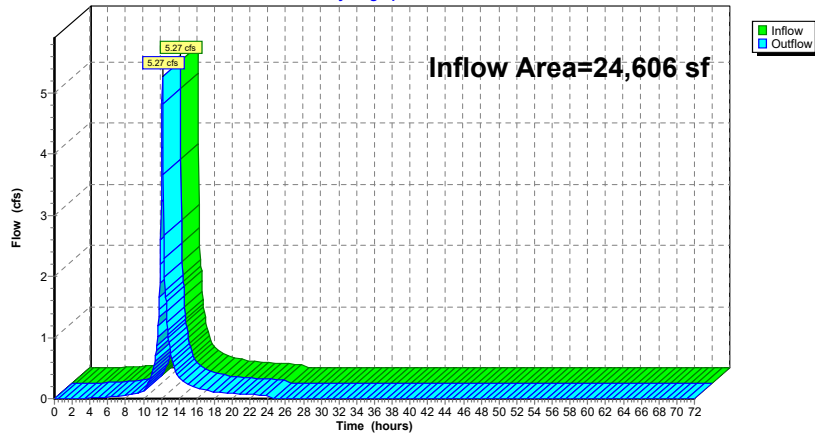
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 24,606 sf, 82.04% Impervious, Inflow Depth = 9.96" for NOAA 25-yr event  
 Inflow = 5.27 cfs @ 12.15 hrs, Volume= 20,413 cf  
 Outflow = 5.27 cfs @ 12.15 hrs, Volume= 20,413 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach 15R: ISOLATOR ROW 1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 334

**Summary for Reach B: PARKING LOT B OVERFLOW**

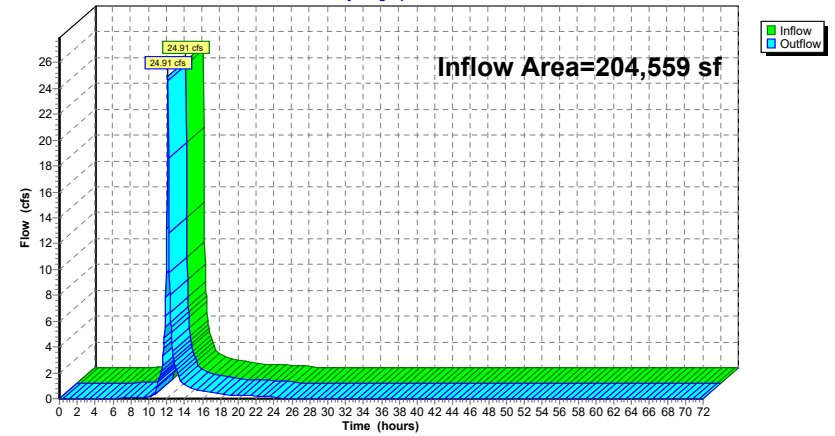
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 204,559 sf, 56.22% Impervious, Inflow Depth = 3.92" for NOAA 25-yr event  
 Inflow = 24.91 cfs @ 12.14 hrs, Volume= 66,837 cf  
 Outflow = 24.91 cfs @ 12.14 hrs, Volume= 66,837 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach B: PARKING LOT B OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 335

**Summary for Reach BMP4\_O: BMP-4 OVERFLOW**

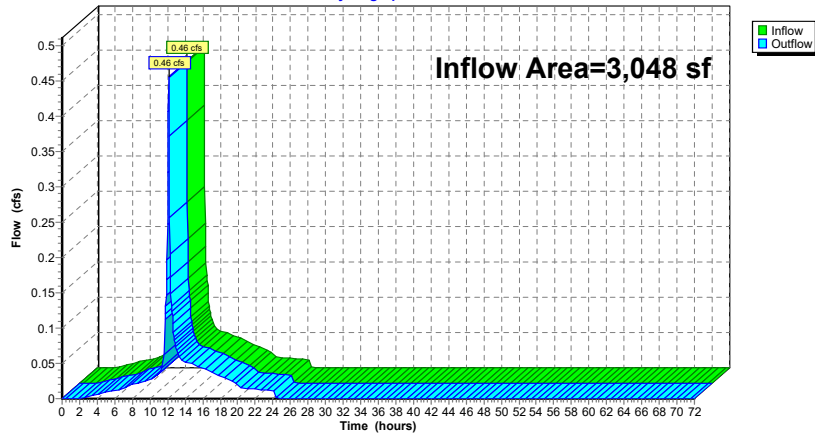
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3,048 sf, 86.09% Impervious, Inflow Depth = 10.42" for NOAA 25-yr event  
Inflow = 0.46 cfs @ 12.14 hrs, Volume= 2,648 cf  
Outflow = 0.46 cfs @ 12.14 hrs, Volume= 2,648 cf, Atten= 0%, Lag= 0.0 min  
Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP4\_O: BMP-4 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 336

**Summary for Reach BMP6\_O: BMP-6 OVERFLOW**

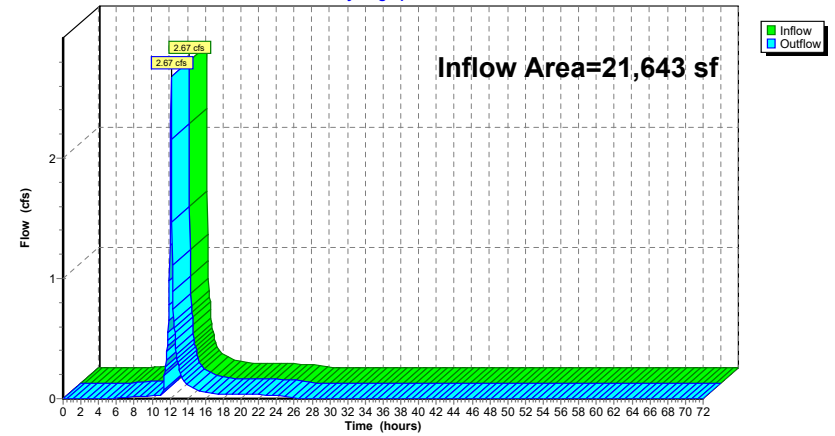
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 21,643 sf, 56.12% Impervious, Inflow Depth = 3.96" for NOAA 25-yr event  
Inflow = 2.67 cfs @ 12.15 hrs, Volume= 7,136 cf  
Outflow = 2.67 cfs @ 12.15 hrs, Volume= 7,136 cf, Atten= 0%, Lag= 0.0 min  
Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP6\_O: BMP-6 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 337

**Summary for Reach BMP7\_O: BMP-7 OVERFLOW**

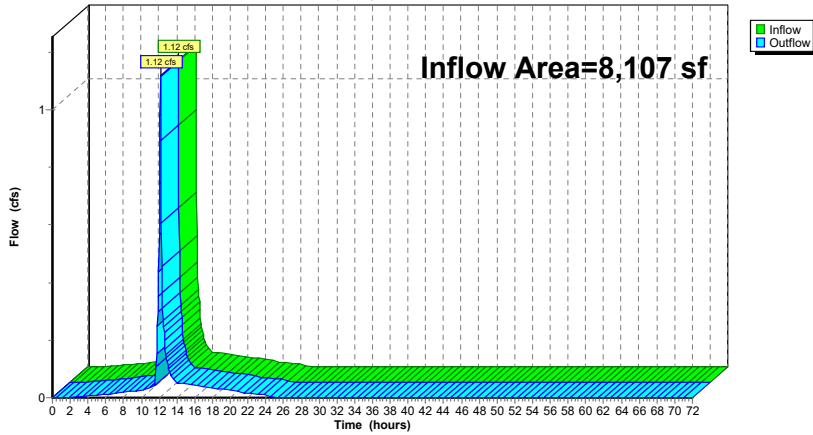
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8,107 sf, 88.34% Impervious, Inflow Depth = 5.45" for NOAA 25-yr event  
 Inflow = 1.12 cfs @ 12.14 hrs, Volume= 3,683 cf  
 Outflow = 1.12 cfs @ 12.14 hrs, Volume= 3,683 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP7\_O: BMP-7 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 338

**Summary for Reach BMP9\_O: BMP-9 OVERFLOW**

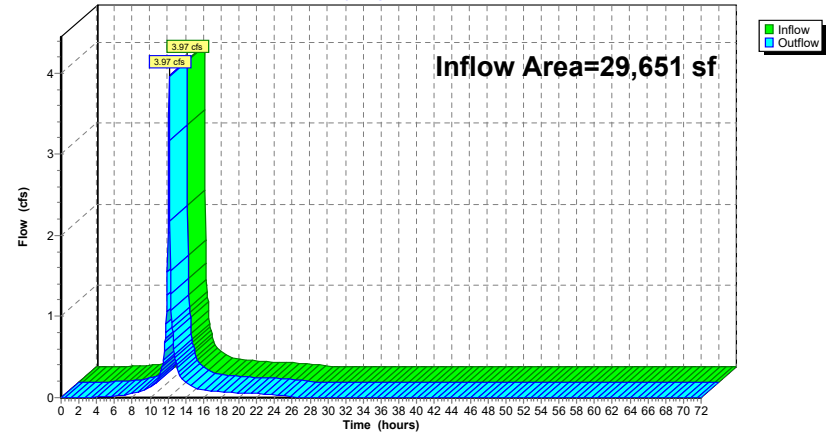
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 29,651 sf, 74.77% Impervious, Inflow Depth = 5.11" for NOAA 25-yr event  
 Inflow = 3.97 cfs @ 12.14 hrs, Volume= 12,623 cf  
 Outflow = 3.97 cfs @ 12.14 hrs, Volume= 12,623 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP9\_O: BMP-9 OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 339

**Summary for Reach BMP\_3: BMP-3\_OVERFLOW**

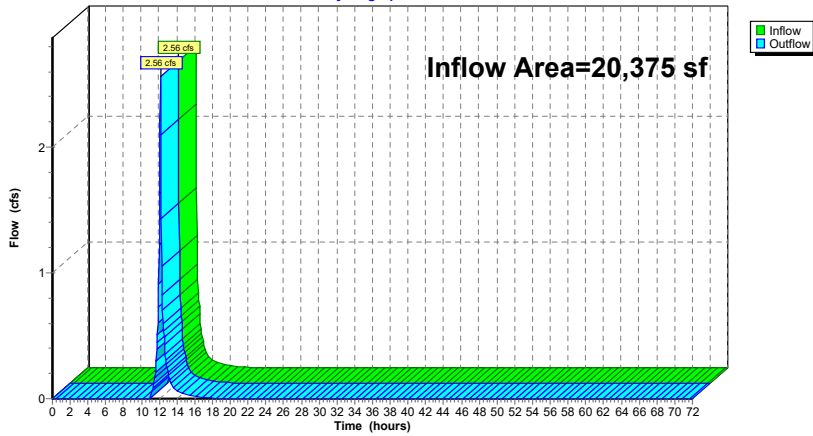
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 20,375 sf, 69.14% Impervious, Inflow Depth = 2.87" for NOAA 25-yr event  
 Inflow = 2.56 cfs @ 12.15 hrs, Volume= 4,877 cf  
 Outflow = 2.56 cfs @ 12.15 hrs, Volume= 4,877 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach BMP\_3: BMP-3\_OVERFLOW**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 340

**Summary for Reach DP-1: French Rodney Blvd 14" Outfall**

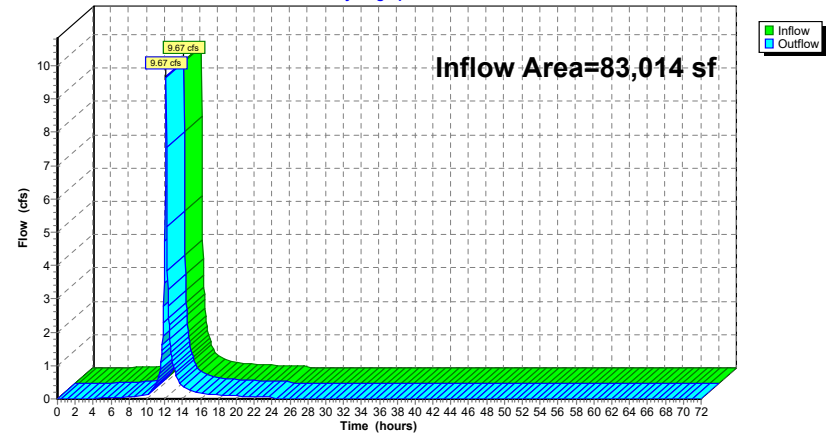
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 83,014 sf, 62.20% Impervious, Inflow Depth = 3.67" for NOAA 25-yr event  
 Inflow = 9.67 cfs @ 12.14 hrs, Volume= 25,417 cf  
 Outflow = 9.67 cfs @ 12.14 hrs, Volume= 25,417 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-1: French Rodney Blvd 14" Outfall**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 341

**Summary for Reach DP-2: NORTHERN OUTFALL**

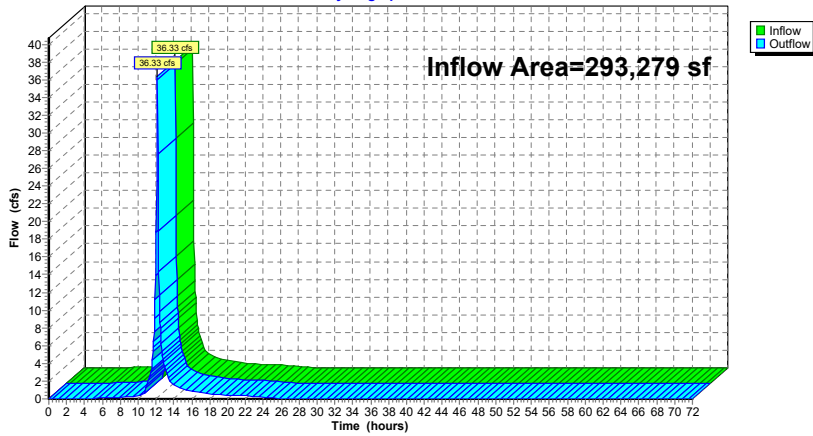
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =	293,279 sf,	59.54% Impervious,	Inflow Depth = 4.04"	for NOAA 25-yr event
Inflow =	36.33 cfs @	12.14 hrs,	Volume=	98,692 cf
Outflow =	36.33 cfs @	12.14 hrs,	Volume=	98,692 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach DP-2: NORTHERN OUTFALL**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 342

**Summary for Reach H ST: HUDSON STREET DRAINAGE**

[40] Hint: Not Described (Outflow=Inflow)

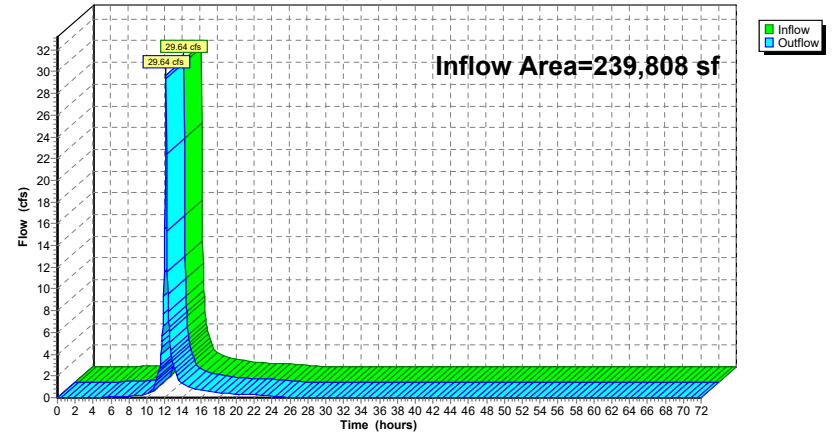
Inflow Area =	239,808 sf,	59.25% Impervious,	Inflow Depth = 4.10"	for NOAA 25-yr event
Inflow =	29.64 cfs @	12.14 hrs,	Volume=	82,004 cf
Outflow =	29.64 cfs @	12.14 hrs,	Volume=	82,004 cf, Atten= 0%, Lag= 0.0 min

Routed to Reach DP-2 : NORTHERN OUTFALL

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach H ST: HUDSON STREET DRAINAGE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 343

**Summary for Reach P ST: PORTLAND STREET DRAINAGE**

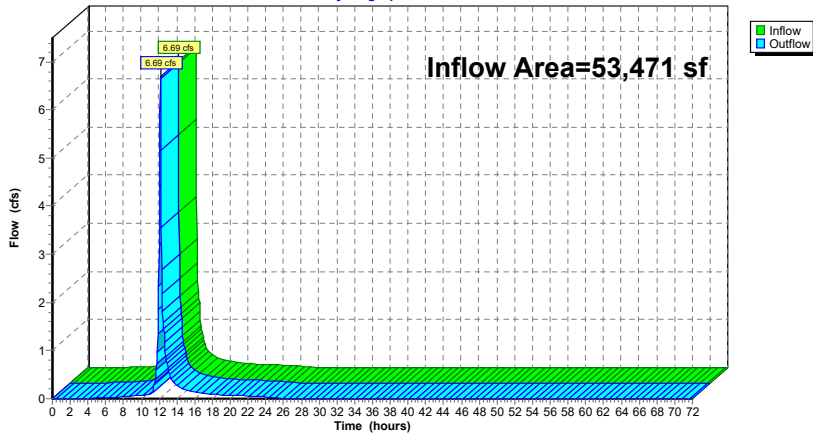
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 53,471 sf, 60.83% Impervious, Inflow Depth = 3.75" for NOAA 25-yr event  
 Inflow = 6.69 cfs @ 12.14 hrs, Volume= 16,689 cf  
 Outflow = 6.69 cfs @ 12.14 hrs, Volume= 16,689 cf, Atten= 0%, Lag= 0.0 min  
 Routed to Reach DP-2 : NORTHERN OUTFALL

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Reach P ST: PORTLAND STREET DRAINAGE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 344

**Summary for Pond 1-P: BB 1**

Inflow Area = 38,826 sf, 51.66% Impervious, Inflow Depth = 4.45" for NOAA 25-yr event  
 Inflow = 4.84 cfs @ 12.13 hrs, Volume= 14,390 cf  
 Outflow = 4.72 cfs @ 12.15 hrs, Volume= 14,390 cf, Atten= 3%, Lag= 1.1 min  
 Discarded = 0.05 cfs @ 12.15 hrs, Volume= 2,687 cf  
 Primary = 2.39 cfs @ 12.15 hrs, Volume= 1,961 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall  
 Secondary = 2.27 cfs @ 12.15 hrs, Volume= 9,742 cf  
 Routed to Reach 15R : ISOLATOR ROW 1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.18' @ 12.15 hrs Surf.Area= 1,807 sf Storage= 1,085 cf

Plug-Flow detention time= 44.4 min calculated for 14,380 cf (100% of inflow)  
 Center-of-Mass det. time= 44.6 min ( 844.8 - 800.1 )

Volume #1	Invert	Avail.Storage	Storage Description
	9.20'	1,114 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.20	490	0	0
9.50	800	194	194
10.20	1,830	920	1,114

Device	Routing	Invert	Outlet Devices
#1	Primary	8.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100' /' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Discarded	9.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.00'
#3	Device 1	10.00'	<b>24inch-Dome Gate Capacity X 2.00</b>
#4	Secondary	9.83'	<b>15inch-Dome Gate Capacity</b>

**Discarded OutFlow** Max=0.05 cfs @ 12.15 hrs HW=10.18' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.05 cfs)

**Primary OutFlow** Max=2.36 cfs @ 12.15 hrs HW=10.18' (Free Discharge)  
 ↳1=Culvert (Passes 2.36 cfs of 4.90 cfs potential flow)  
 ↳3=24inch-Dome Gate Capacity (Custom Controls 2.36 cfs)

**Secondary OutFlow** Max=2.27 cfs @ 12.15 hrs HW=10.18' (Free Discharge)  
 ↳4=15inch-Dome Gate Capacity (Custom Controls 2.27 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

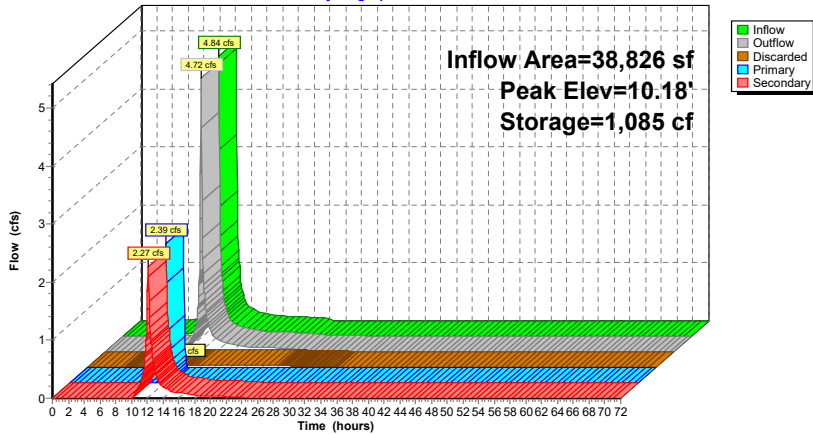
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 345

**Pond 1-P: BB 1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 346

**Summary for Pond 2a-P: BB 2a**

Inflow Area = 3,116 sf, 92.62% Impervious, Inflow Depth = 5.57" for NOAA 25-yr event  
 Inflow = 0.44 cfs @ 12.13 hrs, Volume= 1,446 cf  
 Outflow = 0.43 cfs @ 12.15 hrs, Volume= 1,410 cf, Atten= 3%, Lag= 1.2 min  
 Primary = 0.43 cfs @ 12.15 hrs, Volume= 1,410 cf  
 Routed to Reach 15R : ISOLATOR ROW 1  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.20' @ 12.15 hrs Surf.Area= 480 sf Storage= 82 cf

Plug-Flow detention time= 31.7 min calculated for 1,409 cf (97% of inflow)  
 Center-of-Mass det. time= 16.0 min ( 775.3 - 759.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	710 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	320	0	0
9.00	1,100	710	710

Device	Routing	Invert	Outlet Devices
#1	Secondary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/ S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	8.50'	<b>24inch-Dome Grate Capacity X 2.00</b>
#3	Primary	8.10'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.42 cfs @ 12.15 hrs HW=8.20' (Free Discharge)  
 ↳3=15inch-Dome Grate Capacity(Custom Controls 0.42 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=8.00' (Free Discharge)  
 ↳1=Culvert (Passes 0.00 cfs of 2.27 cfs potential flow)  
 ↳2=24inch-Dome Grate Capacity ( Controls 0.00 cfs)



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

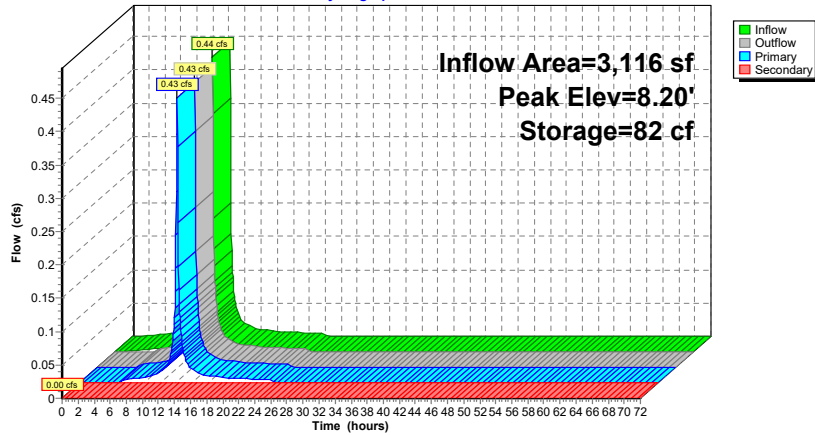
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 347

**Pond 2a-P: BB 2a**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 348

**Summary for Pond 2b-P: BB 2b**

Inflow Area = 21,490 sf, 80.50% Impervious, Inflow Depth = 5.22" for NOAA 25-yr event  
 Inflow = 2.97 cfs @ 12.13 hrs, Volume= 9,352 cf  
 Outflow = 2.87 cfs @ 12.15 hrs, Volume= 9,316 cf, Atten= 3%, Lag= 1.4 min  
 Primary = 2.57 cfs @ 12.15 hrs, Volume= 9,261 cf  
 Routed to Reach 15R : ISOLATOR ROW 1  
 Secondary = 0.30 cfs @ 12.15 hrs, Volume= 55 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.54' @ 12.15 hrs Surf.Area= 743 sf Storage= 288 cf

Plug-Flow detention time= 6.8 min calculated for 9,316 cf (100% of inflow)  
 Center-of-Mass det. time= 4.3 min ( 778.9 - 774.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	710 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)
	8.00	320	0
	9.00	1,100	710

Device	Routing	Invert	Outlet Devices
#1	Secondary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/ S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	8.50'	<b>24inch-Dome Grate Capacity X 2.00</b>
#3	Primary	8.10'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=2.56 cfs @ 12.15 hrs HW=8.54' (Free Discharge)  
 ↳3=15inch-Dome Grate Capacity(Custom Controls 2.56 cfs)

**Secondary OutFlow** Max=0.30 cfs @ 12.15 hrs HW=8.54' (Free Discharge)  
 ↳1=Culvert (Passes 0.30 cfs of 3.75 cfs potential flow)  
 ↳2=24inch-Dome Grate Capacity(Custom Controls 0.30 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

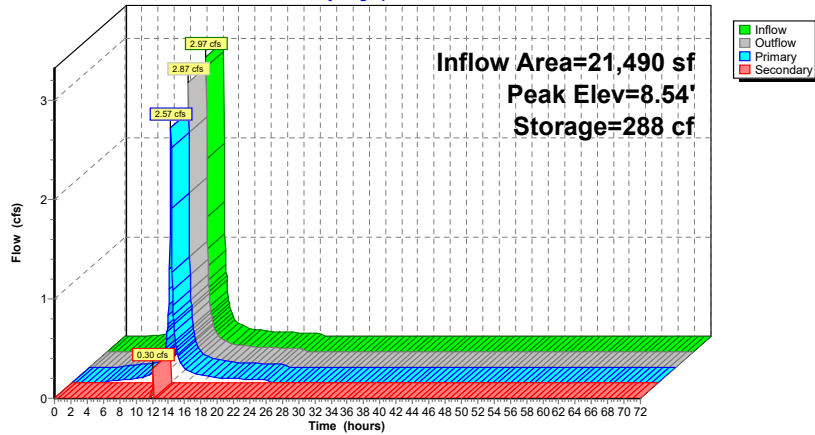
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 349

**Pond 2b-P: BB 2b**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 350

**Summary for Pond 3A-P: BB 3A**

Inflow Area = 10,987 sf, 58.16% Impervious, Inflow Depth = 4.66" for NOAA 25-yr event  
 Inflow = 1.42 cfs @ 12.13 hrs, Volume= 4,271 cf  
 Outflow = 1.36 cfs @ 12.15 hrs, Volume= 4,271 cf, Atten= 4%, Lag= 1.2 min  
 Discarded = 0.03 cfs @ 12.15 hrs, Volume= 1,429 cf  
 Primary = 1.34 cfs @ 12.15 hrs, Volume= 2,842 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.10' @ 12.15 hrs Surf.Area= 917 sf Storage= 472 cf

Plug-Flow detention time= 74.1 min calculated for 4,268 cf (100% of inflow)  
 Center-of-Mass det. time= 74.4 min ( 868.1 - 793.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.25'	622 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.25	271	0	0
10.45	350	62	62
11.25	1,050	560	622

Device	Routing	Invert	Outlet Devices
#1	Primary	9.30'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.30' / 9.20' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.55 sf
#2	Discarded	10.25'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.30'
#3	Primary	11.15'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	10.90'	<b>24inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.03 cfs @ 12.15 hrs HW=11.10' (Free Discharge)  
 2=Exfiltration ( Controls 0.03 cfs)

**Primary OutFlow** Max=1.33 cfs @ 12.15 hrs HW=11.10' (Free Discharge)  
 1=Culvert (Passes 1.33 cfs of 3.09 cfs potential flow)  
 4=24inch-Dome Grate Capacity (Custom Controls 1.33 cfs)  
 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

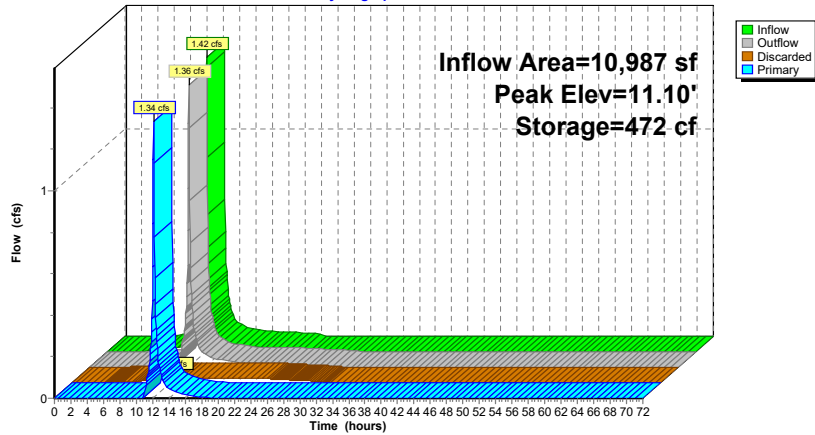
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 351

**Pond 3A-P: BB 3A**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 352

**Summary for Pond 3B-P: BB 3B**

Inflow Area = 4,545 sf, 77.34% Impervious, Inflow Depth = 5.22" for NOAA 25-yr event  
 Inflow = 0.63 cfs @ 12.13 hrs, Volume= 1,978 cf  
 Outflow = 0.62 cfs @ 12.14 hrs, Volume= 1,978 cf, Atten= 1%, Lag= 0.9 min  
 Discarded = 0.02 cfs @ 12.14 hrs, Volume= 880 cf  
 Primary = 0.60 cfs @ 12.14 hrs, Volume= 1,098 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 12.90' @ 12.14 hrs Surf.Area= 568 sf Storage= 260 cf

Plug-Flow detention time= 88.8 min calculated for 1,976 cf (100% of inflow)  
 Center-of-Mass det. time= 88.9 min ( 863.6 - 774.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	12.20'	263 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
12.20	180	0	0
12.90	570	263	263

Device	Routing	Invert	Outlet Devices
#1	Primary	10.70'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.70' / 10.60' S= 0.0100 ' / Cc= 0.900 n= 0.013, Flow Area= 0.55 sf
#2	Discarded	12.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 8.70'
#3	Device 1	12.80'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	12.85'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.02 cfs @ 12.14 hrs HW=12.90' (Free Discharge)  
 ↳ **2=Exfiltration** ( Controls 0.02 cfs)

**Primary OutFlow** Max=0.59 cfs @ 12.14 hrs HW=12.90' (Free Discharge)  
 ↳ **1=Culvert** (Passes 0.43 cfs of 3.50 cfs potential flow)  
 ↳ **3=24inch-Dome Grate Capacity** (Custom Controls 0.43 cfs)  
 ↳ **4=Sharp-Crested Rectangular Weir** (Weir Controls 0.16 cfs @ 0.69 fps)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

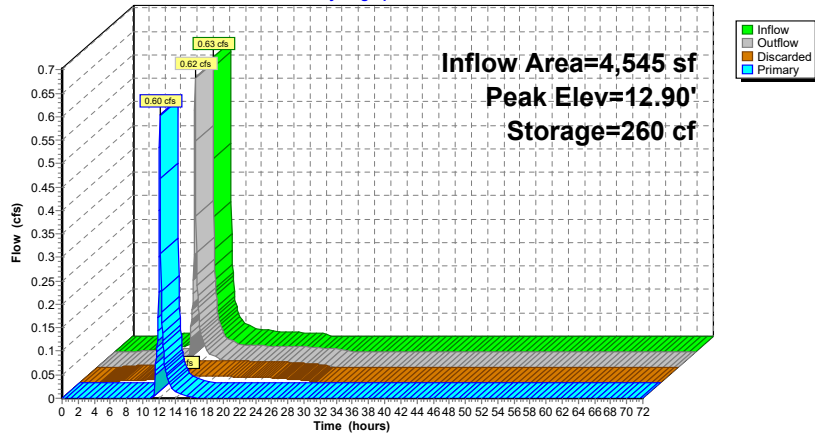
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 353

**Pond 3B-P: BB 3B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 354

**Summary for Pond 4A-P: BB 4A - POND**

Inflow Area = 4,843 sf, 86.37% Impervious, Inflow Depth = 5.45" for NOAA 25-yr event  
 Inflow = 0.68 cfs @ 12.13 hrs, Volume= 2,200 cf  
 Outflow = 0.66 cfs @ 12.15 hrs, Volume= 2,200 cf, Atten= 3%, Lag= 1.2 min  
 Primary = 0.63 cfs @ 12.15 hrs, Volume= 937 cf  
 Routed to Reach BMP\_3 : BMP-3\_OVERFLOW  
 Secondary = 0.04 cfs @ 12.15 hrs, Volume= 1,263 cf  
 Routed to Pond 4A-S : BB4A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.07' @ 12.15 hrs Surf.Area= 588 sf Storage= 239 cf

Plug-Flow detention time= 29.7 min calculated for 2,200 cf (100% of inflow)  
 Center-of-Mass det. time= 29.7 min ( 794.6 - 764.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.50'	320 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)
	9.50	250	0
	10.20	664	320

Device	Routing	Invert	Outlet Devices
#1	Primary	8.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.00'
#3	Primary	10.10'	<b>5.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)</b>
#4	Device 1	9.95'	<b>24inchDome Grate Capacity</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950

**Primary OutFlow** Max=0.62 cfs @ 12.15 hrs HW=10.07' (Free Discharge)  
 1=Culvert (Passes 0.62 cfs of 4.74 cfs potential flow)  
 4=24inchDome Grate Capacity (Custom Controls 0.62 cfs)  
 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.15 hrs HW=10.07' (Free Discharge)  
 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

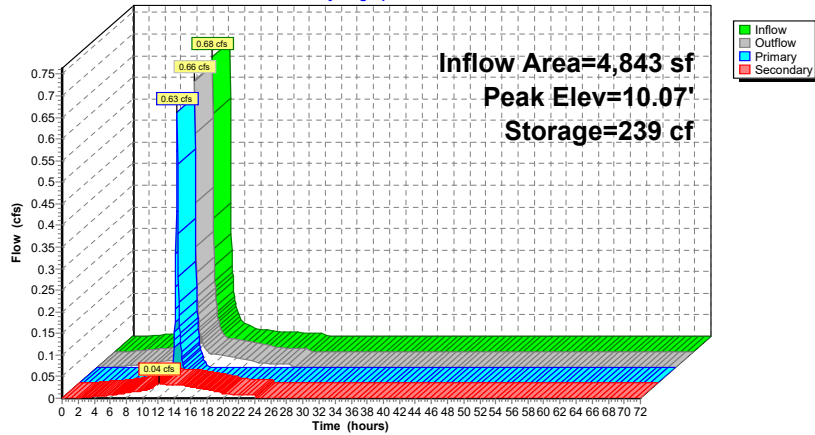
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 355

**Pond 4A-P: BB 4A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 356

**Summary for Pond 4A-S: BB4A-Stone**

Inflow = 0.04 cfs @ 12.15 hrs, Volume= 1,263 cf  
 Outflow = 0.04 cfs @ 12.18 hrs, Volume= 1,263 cf, Atten= 1%, Lag= 1.9 min  
 Primary = 0.04 cfs @ 12.18 hrs, Volume= 1,263 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.13' @ 12.18 hrs Surf.Area= 230 sf Storage= 9 cf

Plug-Flow detention time= 5.7 min calculated for 1,263 cf (100% of inflow)  
 Center-of-Mass det. time= 5.6 min ( 845.0 - 839.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	138 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 460 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	230	0	0
8.00	230	460	460

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.18 hrs HW=6.13' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.20 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

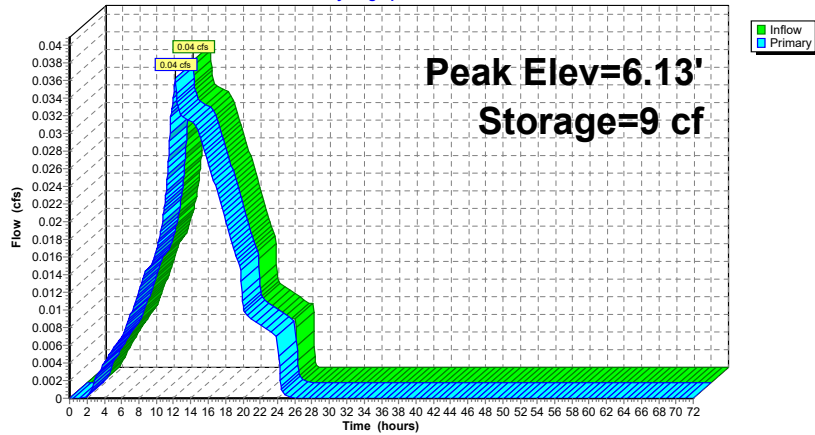
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 357

**Pond 4A-S: BB4A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 358

**Summary for Pond 4B-P: BB 4B - POND**

Inflow Area = 3,048 sf, 86.09% Impervious, Inflow Depth = 5.45" for NOAA 25-yr event  
 Inflow = 0.43 cfs @ 12.13 hrs, Volume= 1,385 cf  
 Outflow = 0.42 cfs @ 12.14 hrs, Volume= 1,385 cf, Atten= 2%, Lag= 1.0 min  
 Primary = 0.40 cfs @ 12.14 hrs, Volume= 597 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW  
 Secondary = 0.02 cfs @ 12.14 hrs, Volume= 787 cf  
 Routed to Pond 4B-S : BB 4A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.04' @ 12.14 hrs Surf.Area= 360 sf Storage= 136 cf

Plug-Flow detention time= 29.1 min calculated for 1,384 cf (100% of inflow)  
 Center-of-Mass det. time= 29.1 min ( 794.0 - 764.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.50'	199 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.50	144	0	0
11.20	424	199	199

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.00' / 8.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.00'
#3	Primary	11.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)
#4	Device 1	10.95'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.39 cfs @ 12.14 hrs HW=11.04' (Free Discharge)  
 1=Culvert (Passes 0.39 cfs of 4.69 cfs potential flow)  
 4=24inch-Dome Grate Capacity (Custom Controls 0.39 cfs)  
 3=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.02 cfs @ 12.14 hrs HW=11.04' (Free Discharge)  
 2=Exfiltration ( Controls 0.02 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

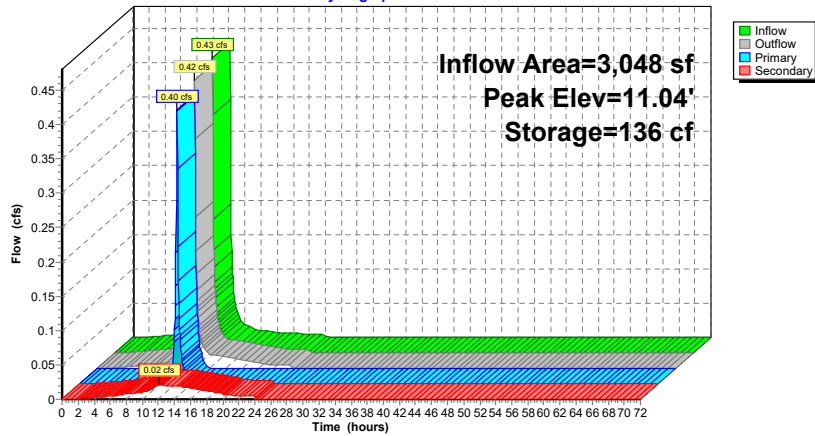
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 359

**Pond 4B-P: BB 4B - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 360

**Summary for Pond 4B-S: BB 4A-Stone**

Inflow = 0.02 cfs @ 12.14 hrs, Volume= 787 cf  
 Outflow = 0.02 cfs @ 12.17 hrs, Volume= 787 cf, Atten= 1%, Lag= 1.5 min  
 Primary = 0.02 cfs @ 12.17 hrs, Volume= 787 cf  
 Routed to Reach BMP4\_O : BMP-4 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.10' @ 12.17 hrs Surf.Area= 145 sf Storage= 4 cf

Plug-Flow detention time= 4.4 min calculated for 787 cf (100% of inflow)  
 Center-of-Mass det. time= 4.4 min ( 844.4 - 840.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	87 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 290 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	145	0	0
8.00	145	290	290

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.17 hrs HW=6.10' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.02 cfs @ 1.05 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

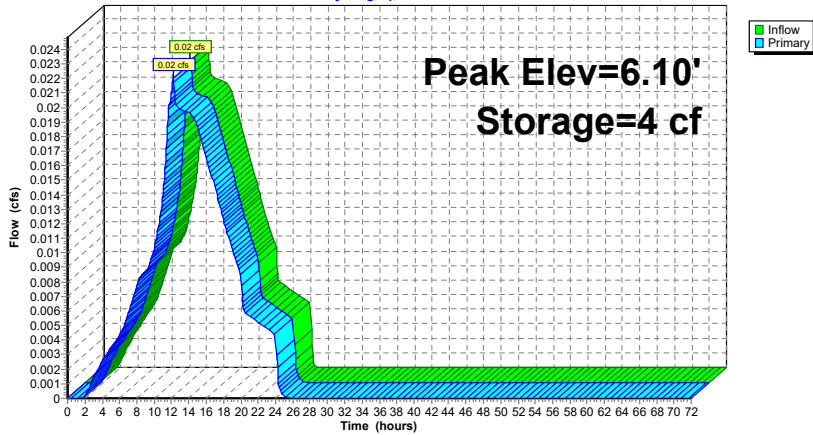
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 361

**Pond 4B-S: BB 4A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 362

**Summary for Pond 5A-P: BB 5A - POND**

Inflow Area = 3,072 sf, 73.44% Impervious, Inflow Depth = 5.11" for NOAA 25-yr event  
 Inflow = 0.42 cfs @ 12.13 hrs, Volume= 1,308 cf  
 Outflow = 0.12 cfs @ 12.37 hrs, Volume= 1,308 cf, Atten= 71%, Lag= 14.6 min  
 Primary = 0.07 cfs @ 12.37 hrs, Volume= 72 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW  
 Secondary = 0.05 cfs @ 12.37 hrs, Volume= 1,236 cf  
 Routed to Pond 5A-PS : BB 5A-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.52' @ 12.37 hrs Surf.Area= 718 sf Storage= 431 cf

Plug-Flow detention time= 68.7 min calculated for 1,307 cf (100% of inflow)  
 Center-of-Mass det. time= 68.6 min ( 847.6 - 778.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.80'	645 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)
	8.80	480	0
	9.80	810	645

Device	Routing	Invert	Outlet Devices
#1	Primary	7.60'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.60' / 7.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.80'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.60'
#3	Device 1	9.50'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.07 cfs @ 12.37 hrs HW=9.52' (Free Discharge)  
 ↑1=Culvert (Passes 0.07 cfs of 4.50 cfs potential flow)  
 ↑3=24inch-Dome Grate Capacity (Custom Controls 0.07 cfs)

**Secondary OutFlow** Max=0.05 cfs @ 12.37 hrs HW=9.52' (Free Discharge)  
 ↑2=Exfiltration ( Controls 0.05 cfs)



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

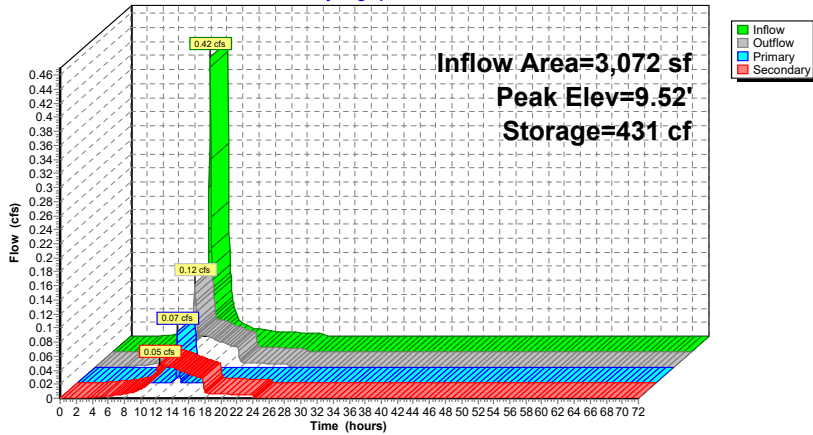
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 363

**Pond 5A-P: BB 5A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 364

**Summary for Pond 5A-PS: BB 5A-Stone**

Inflow = 0.05 cfs @ 12.37 hrs, Volume= 1,236 cf  
 Outflow = 0.05 cfs @ 12.52 hrs, Volume= 1,236 cf, Atten= 0%, Lag= 8.8 min  
 Primary = 0.05 cfs @ 12.52 hrs, Volume= 1,236 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.14' @ 12.52 hrs Surf.Area= 480 sf Storage= 21 cf

Plug-Flow detention time= 10.7 min calculated for 1,235 cf (100% of inflow)  
 Center-of-Mass det. time= 10.7 min ( 864.1 - 853.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	288 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 960 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	480	0	0
8.00	480	960	960

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.05 cfs @ 12.52 hrs HW=6.14' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.05 cfs @ 1.30 fps)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

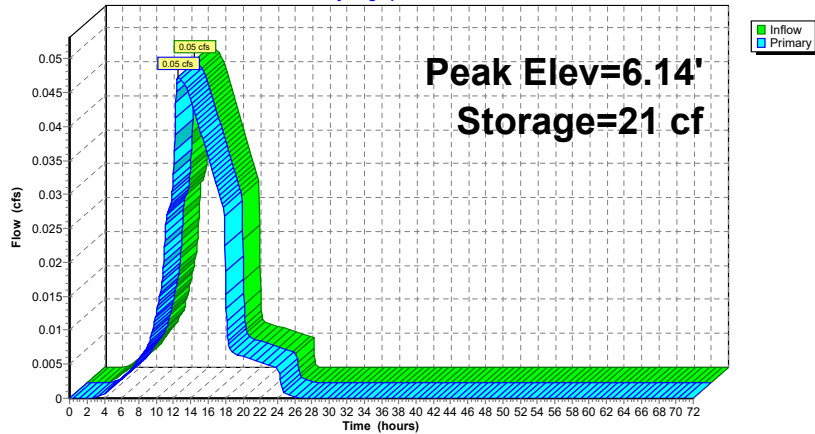
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 365

**Pond 5A-PS: BB 5A-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 366

**Summary for Pond 5B-P: BB 5B - POND**

Inflow Area = 34,755 sf, 71.39% Impervious, Inflow Depth = 5.00" for NOAA 25-yr event  
 Inflow = 4.69 cfs @ 12.13 hrs, Volume= 14,471 cf  
 Outflow = 4.61 cfs @ 12.14 hrs, Volume= 14,471 cf, Atten=2%, Lag= 0.9 min  
 Primary = 2.20 cfs @ 12.14 hrs, Volume= 1,514 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW  
 Secondary = 0.08 cfs @ 12.14 hrs, Volume= 3,958 cf  
 Routed to Pond 5B-PS : BB 5B-Stone  
 Tertiary = 2.33 cfs @ 12.14 hrs, Volume= 8,999 cf  
 Routed to Reach 6R : ISOLATOR ROW 2

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.07' @ 12.14 hrs Surf.Area= 1,309 sf Storage= 716 cf

Plug-Flow detention time= 24.0 min calculated for 14,461 cf (100% of inflow)  
 Center-of-Mass det. time= 24.1 min ( 807.0 - 783.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.20'	889 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.20	327	0	0
9.20	1,450	889	889

Device	Routing	Invert	Outlet Devices
#1	Primary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 ' / ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.20'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.00'
#3	Device 1	8.90'	<b>24inchDome Grate Capacity X 2.00</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Tertiary	8.70'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=2.13 cfs @ 12.14 hrs HW=9.07' (Free Discharge)

1=Culvert (Passes 2.13 cfs of 4.74 cfs potential flow)

3=24inchDome Grate Capacity (Custom Controls 2.13 cfs)

**Secondary OutFlow** Max=0.08 cfs @ 12.14 hrs HW=9.07' (Free Discharge)

2=Exfiltration ( Controls 0.08 cfs)

**Tertiary OutFlow** Max=2.31 cfs @ 12.14 hrs HW=9.07' (Free Discharge)

4=15inch-Dome Grate Capacity (Custom Controls 2.31 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

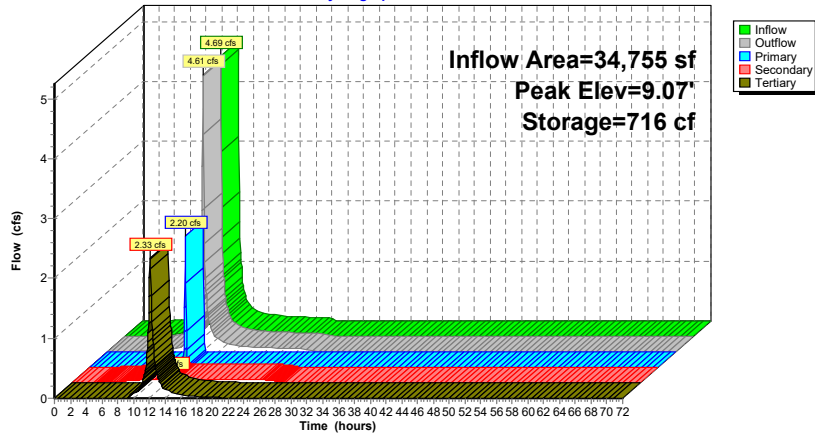
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 367

**Pond 5B-P: BB 5B - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 368

**Summary for Pond 5B-PS: BB 5B-Stone**

[44] Hint: Outlet device #1 is below defined storage

Inflow = 0.08 cfs @ 12.14 hrs, Volume= 3,958 cf  
 Outflow = 0.08 cfs @ 12.14 hrs, Volume= 3,958 cf, Atten= 0%, Lag= 0.1 min  
 Primary = 0.08 cfs @ 12.14 hrs, Volume= 3,958 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.00' @ 12.14 hrs Surf.Area= 690 sf Storage= 1 cf

Plug-Flow detention time= 0.1 min calculated for 3,955 cf (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 945.4 - 945.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	414 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 1,380 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	690	0	0
8.00	690	1,380	1,380

Device	Routing	Invert	Outlet Devices
#1	Primary	4.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.57 cfs @ 12.14 hrs HW=6.00' (Free Discharge)  
 #1=Orifice/Grate (Orifice Controls 0.57 cfs @ 6.52 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

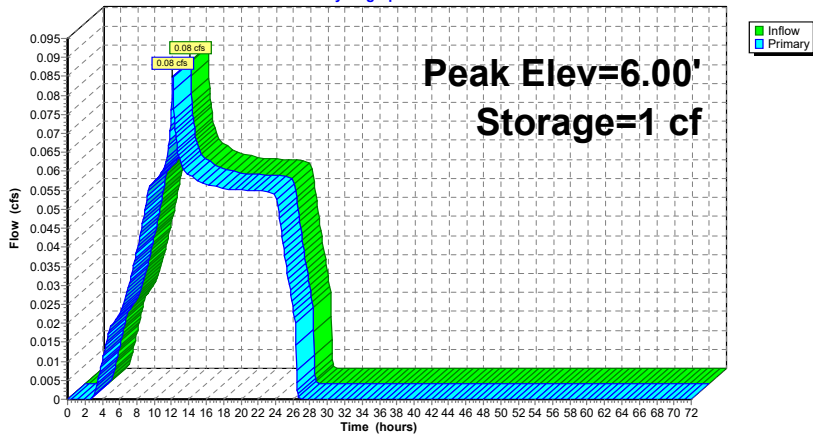
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 369

**Pond 5B-PS: BB 5B-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 370

**Summary for Pond 6A-P: BB 6A - POND**

Inflow Area = 15,148 sf, 46.97% Impervious, Inflow Depth = 4.34" for NOAA 25-yr event  
 Inflow = 1.85 cfs @ 12.13 hrs, Volume= 5,479 cf  
 Outflow = 1.83 cfs @ 12.14 hrs, Volume= 5,479 cf, Atten= 1%, Lag= 1.0 min  
 Primary = 1.78 cfs @ 12.14 hrs, Volume= 3,305 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW  
 Secondary = 0.05 cfs @ 12.14 hrs, Volume= 2,174 cf  
 Routed to Pond 6A-PS : BB 6A - STONE

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.03' @ 12.14 hrs Surf.Area= 711 sf Storage= 443 cf

Plug-Flow detention time= 38.9 min calculated for 5,475 cf (100% of inflow)  
 Center-of-Mass det. time= 39.0 min ( 842.1 - 803.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.20'	491 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.20	350	0	0
11.10	740	491	491

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.00' / 8.90' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.20'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 7.00'
#3	Device 1	10.80'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	11.00'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=1.75 cfs @ 12.14 hrs HW=11.03' (Free Discharge)  
 1=Culvert (Passes 1.66 cfs of 4.68 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 1.66 cfs)  
 4=Sharp-Crested Rectangular Weir (Weir Controls 0.09 cfs @ 0.58 fps)

**Secondary OutFlow** Max=0.05 cfs @ 12.14 hrs HW=11.03' (Free Discharge)  
 2=Exfiltration ( Controls 0.05 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

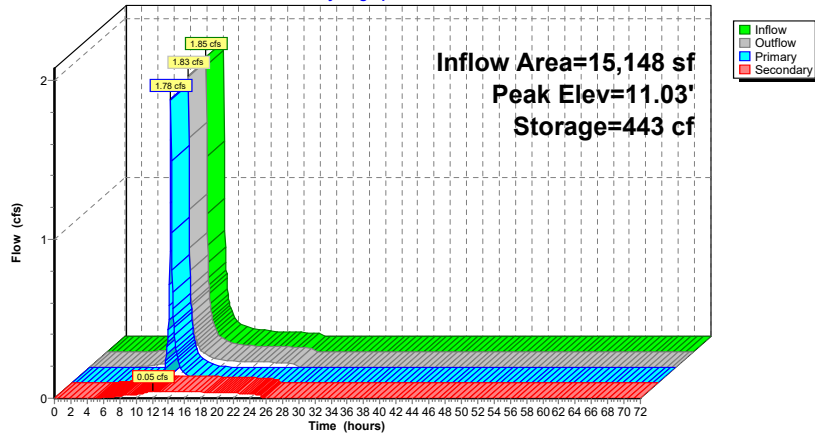
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 371

**Pond 6A-P: BB 6A - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 372

**Summary for Pond 6A-PS: BB 6A - STONE**

Inflow = 0.05 cfs @ 12.14 hrs, Volume= 2,174 cf  
 Outflow = 0.05 cfs @ 12.18 hrs, Volume= 2,174 cf, Atten= 1%, Lag= 2.1 min  
 Primary = 0.05 cfs @ 12.18 hrs, Volume= 2,174 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.14' @ 12.18 hrs Surf.Area= 290 sf Storage= 13 cf

Plug-Flow detention time= 5.3 min calculated for 2,172 cf (100% of inflow)  
 Center-of-Mass det. time= 5.4 min ( 991.5 - 986.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	174 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 580 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	290	0	0
8.00	290	580	580

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.05 cfs @ 12.18 hrs HW=6.14' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.05 cfs @ 1.29 fps)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

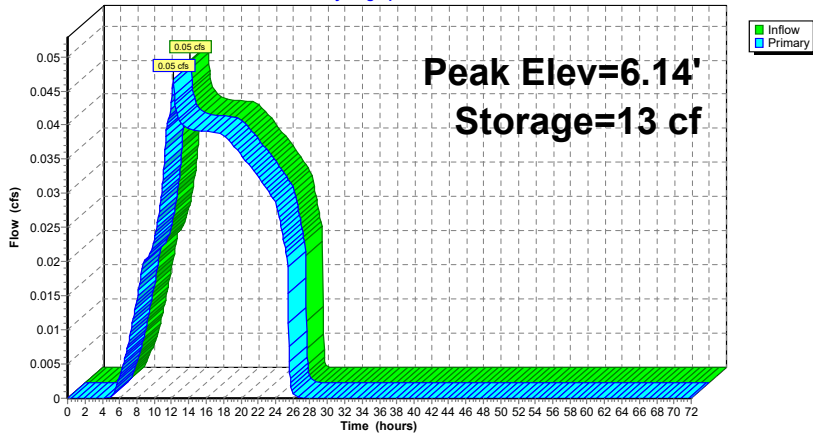
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 373

**Pond 6A-PS: BB 6A - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 374

**Summary for Pond 6B-P: BB 6B**

Inflow Area = 6,495 sf, 77.45% Impervious, Inflow Depth = 5.22" for NOAA 25-yr event  
 Inflow = 0.90 cfs @ 12.13 hrs, Volume= 2,826 cf  
 Outflow = 0.86 cfs @ 12.15 hrs, Volume= 2,826 cf, Atten= 4%, Lag= 1.2 min  
 Discarded = 0.02 cfs @ 12.15 hrs, Volume= 1,170 cf  
 Primary = 0.84 cfs @ 12.15 hrs, Volume= 1,657 cf  
 Routed to Reach BMP6\_O : BMP-6 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 12.10' @ 12.15 hrs Surf.Area= 690 sf Storage= 393 cf

Plug-Flow detention time= 99.9 min calculated for 2,824 cf (100% of inflow)  
 Center-of-Mass det. time= 100.2 min ( 874.8 - 774.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	11.20'	394 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
11.20	185	0	0
12.10	690	394	394

Device	Routing	Invert	Outlet Devices
#1	Primary	10.10'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.10' / 10.00' S= 0.0100 ' / Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Discarded	11.20'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 8.10'
#3	Device 1	11.95'	<b>24inch-Dome Grate Capacity</b>

**Discarded OutFlow** Max=0.02 cfs @ 12.15 hrs HW=12.10' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.02 cfs)

**Primary OutFlow** Max=0.84 cfs @ 12.15 hrs HW=12.10' (Free Discharge)  
 ↳1=Culvert (Passes 0.84 cfs of 4.63 cfs potential flow)  
 ↳3=24inch-Dome Grate Capacity (Custom Controls 0.84 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

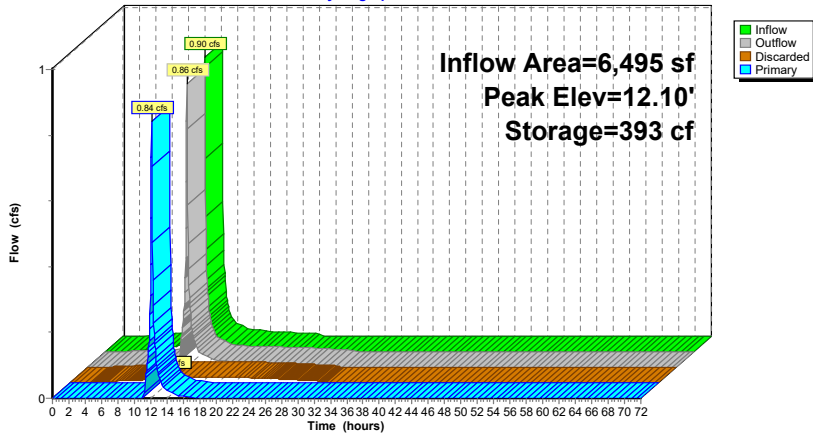
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 375

**Pond 6B-P: BB 6B**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 376

**Summary for Pond 7A-P: BB 7A PONDING**

Inflow Area = 3,165 sf, 87.74% Impervious, Inflow Depth = 5.45" for NOAA 25-yr event  
 Inflow = 0.45 cfs @ 12.13 hrs, Volume= 1,438 cf  
 Outflow = 0.44 cfs @ 12.14 hrs, Volume= 1,438 cf, Atten= 2%, Lag= 0.9 min  
 Primary = 0.42 cfs @ 12.14 hrs, Volume= 638 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW  
 Secondary = 0.02 cfs @ 12.14 hrs, Volume= 800 cf  
 Routed to Pond 7A-S : BB 7A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.99' @ 12.14 hrs Surf.Area= 327 sf Storage= 153 cf

Plug-Flow detention time= 38.3 min calculated for 1,437 cf (100% of inflow)  
 Center-of-Mass det. time= 38.3 min ( 803.2 - 764.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	9.30'	227 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.30	115	0	0
10.20	390	227	227

Device	Routing	Invert	Outlet Devices
#1	Primary	8.10'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.10' / 8.00' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.30'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.10'
#3	Device 1	9.90'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	10.10'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.41 cfs @ 12.14 hrs HW=9.99' (Free Discharge)  
 1=Culvert (Passes 0.41 cfs of 4.46 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 0.41 cfs)  
 4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.02 cfs @ 12.14 hrs HW=9.99' (Free Discharge)  
 2=Exfiltration ( Controls 0.02 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

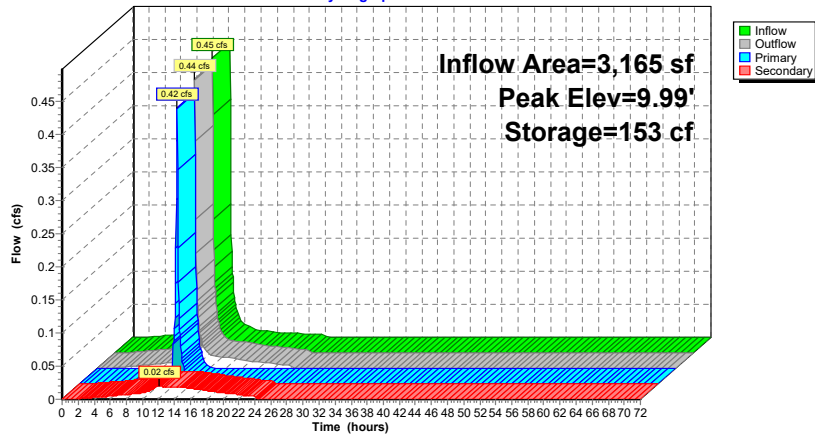
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 377

**Pond 7A-P: BB 7A PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 378

**Summary for Pond 7A-S: BB 7A - STONE**

Inflow = 0.02 cfs @ 12.14 hrs, Volume= 800 cf  
 Outflow = 0.02 cfs @ 12.17 hrs, Volume= 800 cf, Atten= 1%, Lag= 1.5 min  
 Primary = 0.02 cfs @ 12.17 hrs, Volume= 800 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5.19' @ 12.17 hrs Surf.Area= 150 sf Storage= 4 cf

Plug-Flow detention time= 4.6 min calculated for 800 cf (100% of inflow)  
 Center-of-Mass det. time= 4.5 min ( 863.0 - 858.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	5.10'	90 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 300 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
5.10	150	0	0
7.10	150	300	300

Device	Routing	Invert	Outlet Devices
#1	Primary	5.10'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.17 hrs HW=5.19' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.02 cfs @ 1.04 fps)



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

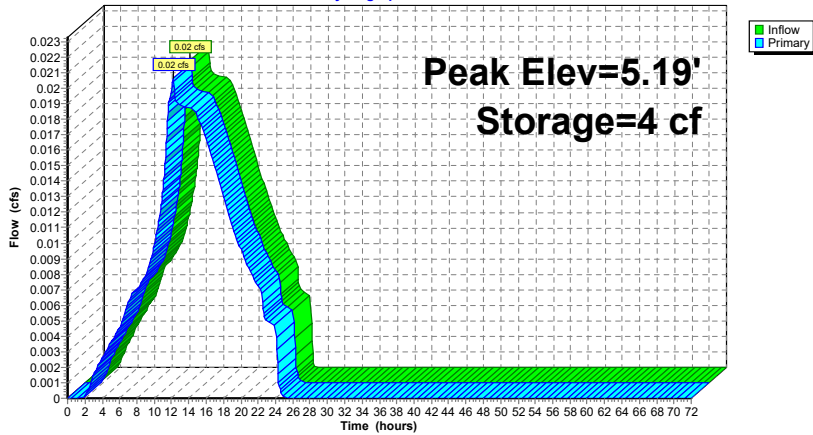
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 379

**Pond 7A-S: BB 7A - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 380

**Summary for Pond 7B-P: BB 7B PONDING**

Inflow Area = 4,942 sf, 88.73% Impervious, Inflow Depth = 5.45" for NOAA 25-yr event  
 Inflow = 0.70 cfs @ 12.13 hrs, Volume= 2,245 cf  
 Outflow = 0.68 cfs @ 12.14 hrs, Volume= 2,245 cf, Atten= 2%, Lag= 1.1 min  
 Primary = 0.65 cfs @ 12.14 hrs, Volume= 940 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW  
 Secondary = 0.03 cfs @ 12.14 hrs, Volume= 1,305 cf  
 Routed to Pond 7B-S : BB 7B - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 10.72' @ 12.14 hrs Surf.Area= 530 sf Storage= 282 cf

Plug-Flow detention time= 40.1 min calculated for 2,243 cf (100% of inflow)  
 Center-of-Mass det. time= 40.0 min ( 804.9 - 764.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	324 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)
	10.00	250	0
	10.80	560	324

Device	Routing	Invert	Outlet Devices
#1	Primary	8.90'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.90' / 8.80' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	10.00'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.90'
#3	Device 1	10.60'	<b>24inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=0.63 cfs @ 12.14 hrs HW=10.72' (Free Discharge)  
 ↑1=Culvert (Passes 0.63 cfs of 4.35 cfs potential flow)  
 ↑3=24inch-Dome Grate Capacity (Custom Controls 0.63 cfs)

**Secondary OutFlow** Max=0.03 cfs @ 12.14 hrs HW=10.72' (Free Discharge)  
 ↑2=Exfiltration ( Controls 0.03 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

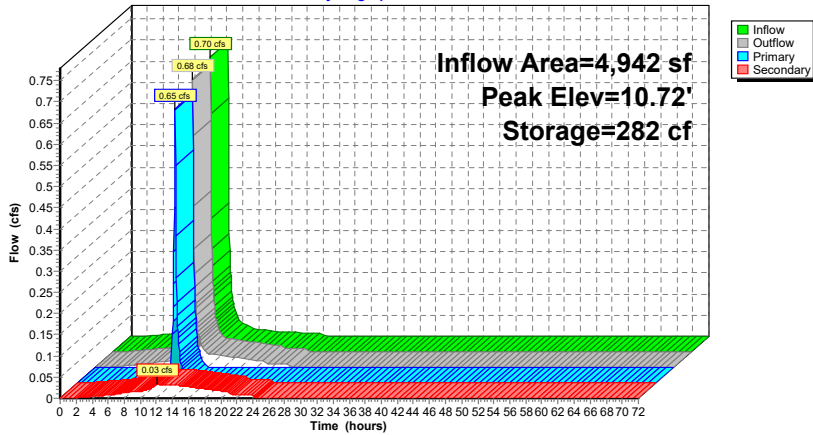
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 381

**Pond 7B-P: BB 7B PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 382

**Summary for Pond 7B-S: BB 7B - STONE**

Inflow = 0.03 cfs @ 12.14 hrs, Volume= 1,305 cf  
 Outflow = 0.03 cfs @ 12.17 hrs, Volume= 1,305 cf, Atten= 0%, Lag= 1.2 min  
 Primary = 0.03 cfs @ 12.17 hrs, Volume= 1,305 cf  
 Routed to Reach BMP7\_O : BMP-7 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5.22' @ 12.17 hrs Surf.Area= 150 sf Storage= 5 cf

Plug-Flow detention time= 3.6 min calculated for 1,304 cf (100% of inflow)  
 Center-of-Mass det. time= 3.6 min ( 858.6 - 855.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	5.10'	90 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 300 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
5.10	150	0	0
7.10	150	300	300

Device	Routing	Invert	Outlet Devices
#1	Primary	5.10'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.03 cfs @ 12.17 hrs HW=5.22' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.19 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

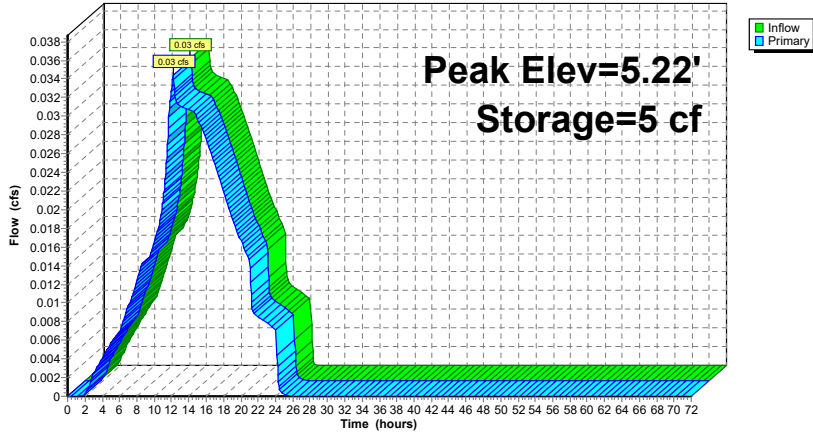
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 383

**Pond 7B-S: BB 7B - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 384

**Summary for Pond 8a-P: BB 8A PONDING**

Inflow Area = 3,978 sf, 79.99% Impervious, Inflow Depth = 5.22" for NOAA 25-yr event  
 Inflow = 0.55 cfs @ 12.13 hrs, Volume= 1,731 cf  
 Outflow = 0.52 cfs @ 12.15 hrs, Volume= 1,731 cf, Atten= 5%, Lag= 1.4 min  
 Primary = 0.48 cfs @ 12.15 hrs, Volume= 547 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE  
 Secondary = 0.04 cfs @ 12.15 hrs, Volume= 1,184 cf  
 Routed to Pond 8a-s : BB 8A - STONE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.10' @ 12.15 hrs Surf.Area= 619 sf Storage= 295 cf

Plug-Flow detention time= 37.1 min calculated for 1,730 cf (100% of inflow)  
 Center-of-Mass det. time= 37.1 min ( 811.7 - 774.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.50'	575 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.50	360	0	0
9.50	790	575	575

Device	Routing	Invert	Outlet Devices
#1	Primary	7.40'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.40' / 7.30' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.50'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.40'
#3	Device 1	9.00'	<b>24inch-Dome Grate Capacity</b>
#4	Primary	9.40'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Primary OutFlow** Max=0.48 cfs @ 12.15 hrs HW=9.10' (Free Discharge)  
 1=Culvert (Passes 0.48 cfs of 4.15 cfs potential flow)  
 3=24inch-Dome Grate Capacity (Custom Controls 0.48 cfs)  
 4=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.15 hrs HW=9.10' (Free Discharge)  
 2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

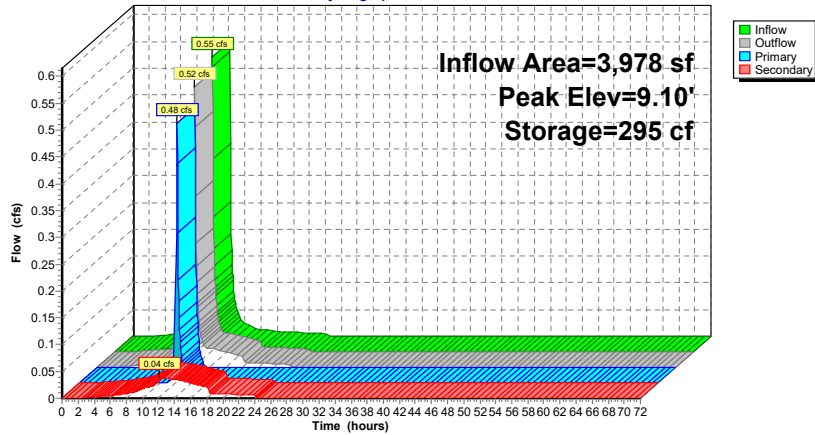
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 385

**Pond 8a-P: BB 8A PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 386

**Summary for Pond 8a-s: BB 8A - STONE**

Inflow = 0.04 cfs @ 12.15 hrs, Volume= 1,184 cf  
 Outflow = 0.04 cfs @ 12.20 hrs, Volume= 1,184 cf, Atten= 1%, Lag= 2.7 min  
 Primary = 0.04 cfs @ 12.20 hrs, Volume= 1,184 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 4.53' @ 12.20 hrs Surf.Area= 300 sf Storage= 12 cf

Plug-Flow detention time= 7.4 min calculated for 1,184 cf (100% of inflow)  
 Center-of-Mass det. time= 7.2 min ( 853.5 - 846.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	4.40'	180 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 600 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
4.40	300	0	0
6.40	300	600	600

Device	Routing	Invert	Outlet Devices
#1	Primary	4.40'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.20 hrs HW=4.53' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.23 fps)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

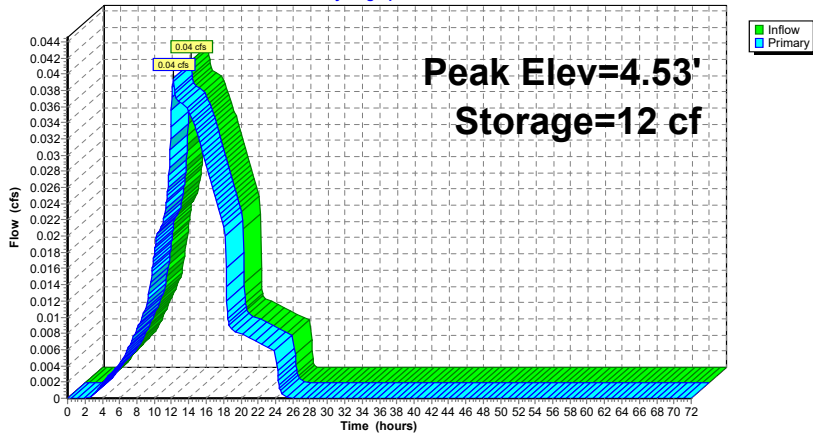
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 387

**Pond 8a-s: BB 8A - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 388

**Summary for Pond 8B-P: BB 8B-PONDING**

Inflow Area = 5,598 sf, 87.78% Impervious, Inflow Depth = 5.45" for NOAA 25-yr event  
 Inflow = 0.79 cfs @ 12.13 hrs, Volume= 2,543 cf  
 Outflow = 0.78 cfs @ 12.14 hrs, Volume= 2,543 cf, Atten= 2%, Lag= 0.9 min  
 Primary = 0.74 cfs @ 12.14 hrs, Volume= 1,102 cf  
 Routed to Reach H ST : HUDSON STREET DRAINAGE  
 Secondary = 0.04 cfs @ 12.14 hrs, Volume= 1,441 cf  
 Routed to Pond 8B-S : BB 8B-Stone

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.73' @ 12.14 hrs Surf.Area= 638 sf Storage= 262 cf

Plug-Flow detention time= 34.7 min calculated for 2,541 cf (100% of inflow)  
 Center-of-Mass det. time= 34.7 min ( 799.6 - 764.9 )

Volume #	Invert	Avail.Storage	Storage Description
#1	9.10'	306 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
	Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet) Cum.Store (cubic-feet)
	9.10	190	0 0
	9.80	685	306 306

Device	Routing	Invert	Outlet Devices
#1	Primary	7.90'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.90' / 7.80' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	9.10'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.90'
#3	Device 1	9.65'	<b>24inch-Dome Grate Capacity X 2.00</b>

**Primary OutFlow** Max=0.72 cfs @ 12.14 hrs HW=9.73' (Free Discharge)  
 ↑1=Culvert (Passes 0.72 cfs of 4.36 cfs potential flow)  
 ↑3=24inch-Dome Grate Capacity (Custom Controls 0.72 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 12.14 hrs HW=9.73' (Free Discharge)  
 ↑2=Exfiltration ( Controls 0.04 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

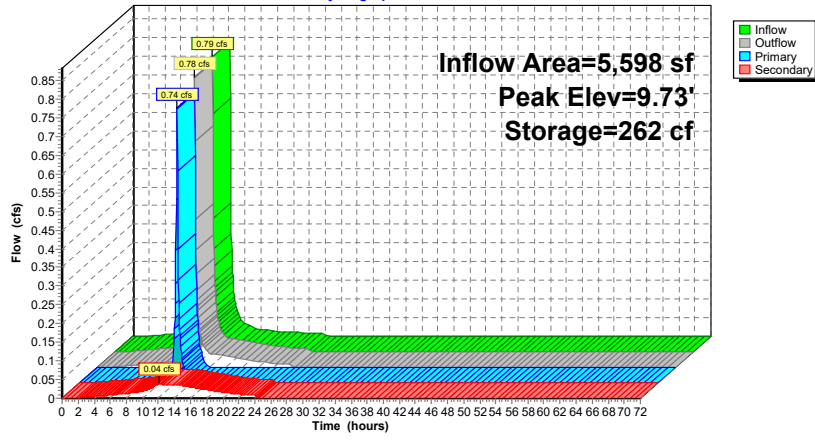
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 389

**Pond 8B-P: BB 8B-PONDING**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 390

**Summary for Pond 8B-S: BB 8B-Stone**

Inflow = 0.04 cfs @ 12.14 hrs, Volume= 1,441 cf  
 Outflow = 0.04 cfs @ 12.18 hrs, Volume= 1,441 cf, Atten= 1%, Lag= 2.2 min  
 Primary = 0.04 cfs @ 12.18 hrs, Volume= 1,441 cf  
 Routed to Reach H ST : HUDSON STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 4.53' @ 12.18 hrs Surf.Area= 300 sf Storage= 12 cf

Plug-Flow detention time= 7.0 min calculated for 1,441 cf (100% of inflow)  
 Center-of-Mass det. time= 6.8 min ( 857.2 - 850.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	4.40'	180 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 600 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
4.40	300	0	0
6.40	300	600	600

Device	Routing	Invert	Outlet Devices
#1	Primary	4.40'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.04 cfs @ 12.18 hrs HW=4.53' (Free Discharge)  
 ↳1=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.23 fps)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

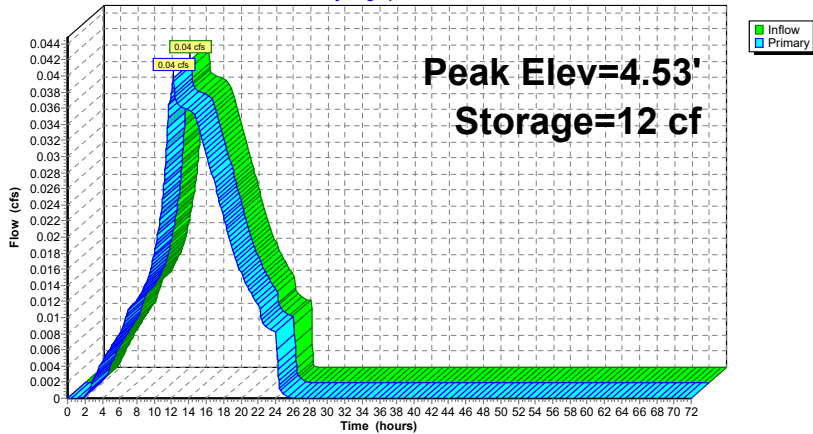
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 391

**Pond 8B-S: BB 8B-Stone**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 392

**Summary for Pond 9-P: BB9 - POND**

Inflow Area = 29,651 sf, 74.77% Impervious, Inflow Depth = 5.11" for NOAA 25-yr event  
 Inflow = 4.05 cfs @ 12.13 hrs, Volume= 12,623 cf  
 Outflow = 3.97 cfs @ 12.14 hrs, Volume= 12,623 cf, Atten=2%, Lag= 1.0 min  
 Primary = 1.24 cfs @ 12.15 hrs, Volume= 542 cf  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW  
 Secondary = 0.05 cfs @ 12.14 hrs, Volume= 2,332 cf  
 Routed to Pond 9-PS : BB9 - STONE  
 Tertiary = 2.68 cfs @ 12.14 hrs, Volume= 9,749 cf  
 Routed to Reach 1R : ISOLATOR ROW C

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 8.99' @ 12.14 hrs Surf.Area= 773 sf Storage= 476 cf

Plug-Flow detention time= 16.7 min calculated for 12,615 cf (100% of inflow)  
 Center-of-Mass det. time= 16.8 min ( 795.7 - 778.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	8.00'	485 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
8.00	190	0	0
9.00	780	485	485

Device	Routing	Invert	Outlet Devices
#1	Primary	7.00'	<b>12.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 7.00' / 6.90' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Secondary	8.00'	<b>2.410 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.00'
#3	Device 1	8.80'	<b>24inchDome Grate Capacity</b> Head (feet) 0.00 0.05 0.10 0.15 0.20 0.25 0.30 0.35 0.40 0.45 0.50 0.55 0.60 0.65 0.70 0.75 0.80 0.85 0.90 0.95 1.00 1.05 1.10 Disch. (cfs) 0.000 0.180 0.460 0.850 1.360 1.830 2.420 3.100 3.600 3.800 4.000 4.200 4.380 4.600 4.750 4.900 5.100 5.200 5.350 5.450 5.650 5.800 5.950
#4	Tertiary	8.50'	<b>15inch-Dome Grate Capacity</b>

**Primary OutFlow** Max=1.19 cfs @ 12.15 hrs HW=8.98' (Free Discharge)  
 1=Culvert (Passes 1.19 cfs of 4.61 cfs potential flow)  
 3=24inchDome Grate Capacity (Custom Controls 1.19 cfs)

**Secondary OutFlow** Max=0.05 cfs @ 12.14 hrs HW=8.98' (Free Discharge)  
 2=Exfiltration ( Controls 0.05 cfs)

**Tertiary OutFlow** Max=2.66 cfs @ 12.14 hrs HW=8.98' (Free Discharge)  
 4=15inch-Dome Grate Capacity (Custom Controls 2.66 cfs)

**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

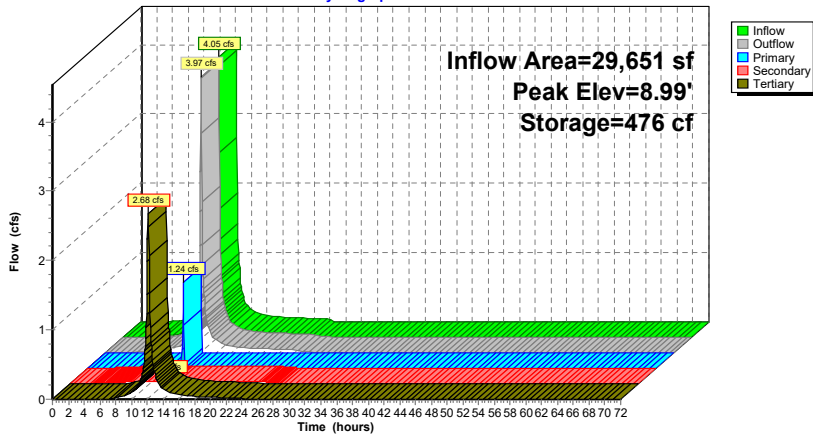
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 393

**Pond 9-P: BB9 - POND**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 394

**Summary for Pond 9-PS: BB9 - STONE**

Inflow = 0.05 cfs @ 12.14 hrs, Volume= 2,332 cf  
 Outflow = 0.05 cfs @ 12.17 hrs, Volume= 2,332 cf, Atten= 2%, Lag= 1.4 min  
 Primary = 0.05 cfs @ 12.17 hrs, Volume= 2,332 cf  
 Routed to Reach BMP9\_O : BMP-9 OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 6.15' @ 12.17 hrs Surf.Area= 190 sf Storage= 9 cf

Plug-Flow detention time= 3.8 min calculated for 2,332 cf (100% of inflow)  
 Center-of-Mass det. time= 3.7 min ( 919.9 - 916.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	114 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc) 380 cf Overall x 30.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6.00	190	0	0
8.00	190	380	380

Device	Routing	Invert	Outlet Devices
#1	Primary	6.00'	<b>4.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.05 cfs @ 12.17 hrs HW=6.15' (Free Discharge)  
 ↳ **1=Orifice/Grate** (Orifice Controls 0.05 cfs @ 1.32 fps)



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

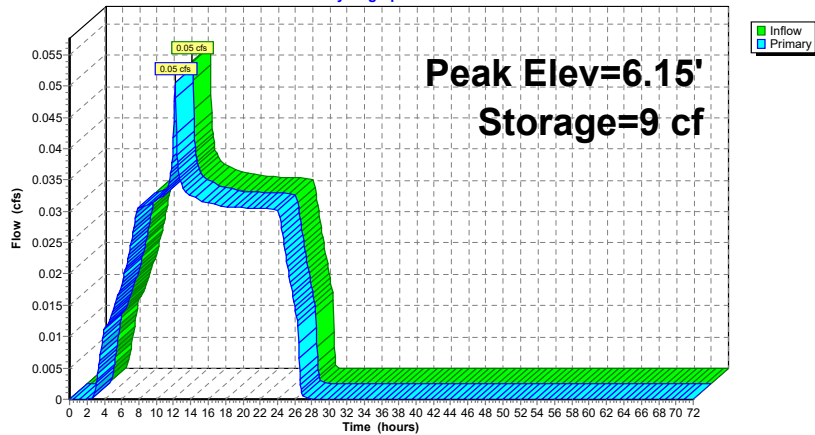
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 395

**Pond 9-PS: BB9 - STONE**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 396

**Summary for Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST**

[57] Hint: Peaked at 9.92' (Flood elevation advised)

Inflow Area = 19,582 sf, 58.17% Impervious, Inflow Depth = 4.66" for NOAA 25-yr event  
 Inflow = 2.53 cfs @ 12.13 hrs, Volume= 7,612 cf  
 Outflow = 2.53 cfs @ 12.13 hrs, Volume= 7,612 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 0.78 cfs @ 12.13 hrs, Volume= 5,625 cf  
 Routed to Pond INF-1 : INFILTRATION SYSTEM #1  
 Secondary = 1.75 cfs @ 12.13 hrs, Volume= 1,987 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.92' @ 12.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	6.0" Vert. WATER QUALITY STORM DIVERSION C= 0.600 Limited to weir flow at low heads
#2	Secondary	9.20'	12.0" Vert. LARGE STORM OVEFLOW C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.76 cfs @ 12.13 hrs HW=9.90' (Free Discharge)

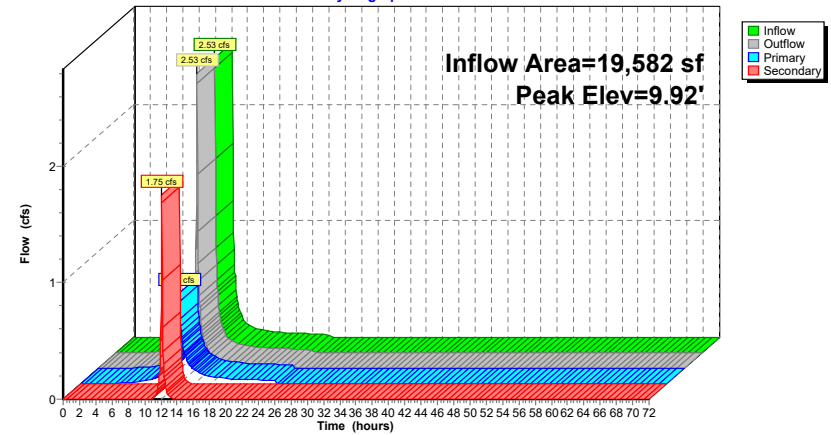
↳1=WATER QUALITY STORM DIVERSION(Orifice Controls 0.76 cfs @ 3.88 fps)

Secondary OutFlow Max=1.66 cfs @ 12.13 hrs HW=9.90' (Free Discharge)

↳2=LARGE STORM OVEFLOW (Orifice Controls 1.66 cfs @ 2.84 fps)

**Pond DMH1: DIVERSION MANHOLE - SEYMOUR ST**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 397

**Summary for Pond DMH2: DIVERSION MANHOLE - HUDSON STREET**

[57] Hint: Peaked at 25.58' (Flood elevation advised)

Inflow Area = 143,309 sf, 49.69% Impervious, Inflow Depth = 4.45" for NOAA 25-yr event  
 Inflow = 17.88 cfs @ 12.13 hrs, Volume= 53,113 cf  
 Outflow = 17.88 cfs @ 12.13 hrs, Volume= 53,113 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 3.65 cfs @ 12.13 hrs, Volume= 28,364 cf  
 Routed to Pond INF-2 : INFILTRATION SYSTEM #2  
 Secondary = 14.23 cfs @ 12.13 hrs, Volume= 24,749 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 25.58' @ 12.13 hrs

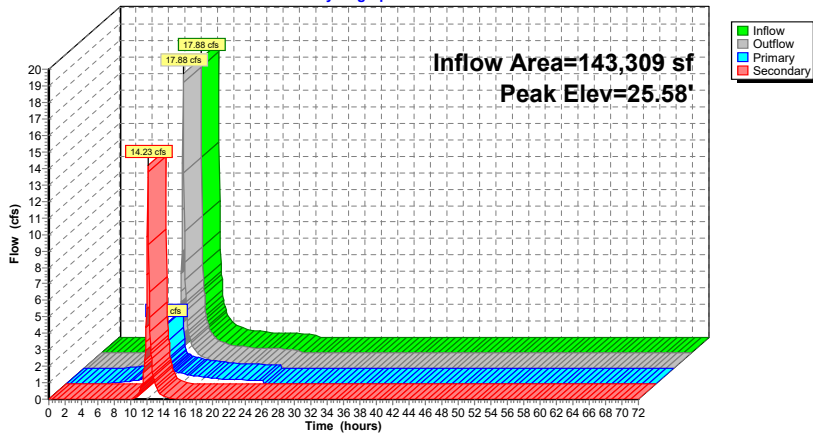
Device	Routing	Invert	Outlet Devices
#1	Primary	10.60'	<b>6.0" Vert. WATER QUALITY STORM DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	11.10'	<b>12.0" Vert. LARGE STORM OVERFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=3.51 cfs @ 12.13 hrs HW=24.62' (Free Discharge)  
 ↳1=WATER QUALITY STORM DIVERSION(Orifice Controls 3.51 cfs @ 17.87 fps)

**Secondary OutFlow** Max=13.65 cfs @ 12.13 hrs HW=24.62' (Free Discharge)  
 ↳2=LARGE STORM OVERFLOW(Orifice Controls 13.65 cfs @ 17.38 fps)

**Pond DMH2: DIVERSION MANHOLE - HUDSON STREET**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 398

**Summary for Pond DMH3: DIVERSION MANHOLE - PORTLAND ST**

[57] Hint: Peaked at 11.85' (Flood elevation advised)

Inflow Area = 19,743 sf, 50.83% Impervious, Inflow Depth = 4.45" for NOAA 25-yr event  
 Inflow = 2.46 cfs @ 12.13 hrs, Volume= 7,317 cf  
 Outflow = 2.46 cfs @ 12.13 hrs, Volume= 7,317 cf, Atten= 0%, Lag= 0.0 min  
 Primary = 1.21 cfs @ 12.13 hrs, Volume= 6,020 cf  
 Routed to Pond INF3 : INFILTRATION SYSTEM #1  
 Secondary = 1.25 cfs @ 12.13 hrs, Volume= 1,297 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 11.85' @ 12.13 hrs

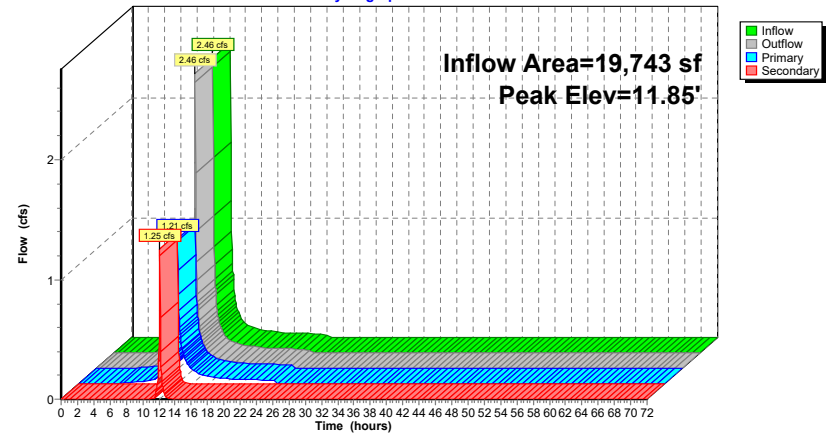
Device	Routing	Invert	Outlet Devices
#1	Primary	11.00'	<b>8.0" Vert. WATER QUALITY DIVERSION</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	11.20'	<b>10.0" Vert. LARGE STORM OVERFLOW</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=1.18 cfs @ 12.13 hrs HW=11.83' (Free Discharge)  
 ↳1=WATER QUALITY DIVERSION(Orifice Controls 1.18 cfs @ 3.38 fps)

**Secondary OutFlow** Max=1.18 cfs @ 12.13 hrs HW=11.83' (Free Discharge)  
 ↳2=LARGE STORM OVERFLOW(Orifice Controls 1.18 cfs @ 2.69 fps)

**Pond DMH3: DIVERSION MANHOLE - PORTLAND ST**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 399

**Summary for Pond INF-1: INFILTRATION SYSTEM #1**

[81] Warning: Exceeded Pond DMH1 by 0.24' @ 15.20 hrs

Inflow Area = 19,582 sf, 58.17% Impervious, Inflow Depth = 3.45" for NOAA 25-yr event  
 Inflow = 0.78 cfs @ 12.13 hrs, Volume= 5,625 cf  
 Outflow = 0.45 cfs @ 12.37 hrs, Volume= 5,625 cf, Atten= 42%, Lag= 14.5 min  
 Discarded = 0.08 cfs @ 12.37 hrs, Volume= 4,624 cf  
 Primary = 0.37 cfs @ 12.37 hrs, Volume= 1,001 cf  
 Routed to Reach DP-1 : French Rodney Blvd 14" Outfall

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.48' @ 12.37 hrs Surf.Area= 1,772 sf Storage= 1,689 cf

Plug-Flow detention time= 201.8 min calculated for 5,621 cf (100% of inflow)  
 Center-of-Mass det. time= 201.8 min ( 1,018.7 - 816.9 )

Volume	Invert	Avail.Storage	Storage Description
#1A	7.80'	1,091 cf	<b>21.50'W x 81.52'L x 2.33'H Field A</b> 4,090 cf Overall - 973 cf Embedded = 3,117 cf x 35.0% Voids
#2A	8.30'	973 cf	<b>ADS_StormTech SC-310 +Cap</b> x 66 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 66 Chambers in 6 Rows
#3	7.80'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		2,201 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	7.80'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.80'
#2	Primary	8.10'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.10' / 8.00' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.40'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.08 cfs @ 12.37 hrs HW=9.48' (Free Discharge)  
 1=Exfiltration ( Controls 0.08 cfs)

**Primary OutFlow** Max=0.36 cfs @ 12.37 hrs HW=9.48' (Free Discharge)  
 2=Culvert (Passes 0.36 cfs of 2.58 cfs potential flow)  
 3=Sharp-Crested Rectangular Weir (Weir Controls 0.36 cfs @ 0.92 cfs)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 400

**Pond INF-1: INFILTRATION SYSTEM #1 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-310 +Cap (ADS StormTech®SC-310 with cap length)**

Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf

Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap

34.0" Wide + 6.0" Spacing = 40.0" C-C Row Spacing

11 Chambers/Row x 7.12' Long +0.60' Cap Length x 2 = 79.52' Row Length +12.0" End Stone x 2 = 81.52' Base Length

6 Rows x 34.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 21.50' Base Width

6.0" Stone Base + 16.0" Chamber Height + 6.0" Stone Cover = 2.33' Field Height

66 Chambers x 14.7 cf = 973.0 cf Chamber Storage

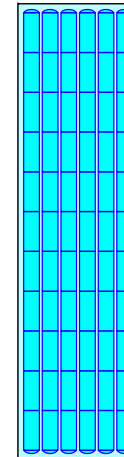
4,089.6 cf Field - 973.0 cf Chambers = 3,116.6 cf Stone x 35.0% Voids = 1,090.8 cf Stone Storage

Chamber Storage + Stone Storage = 2,063.8 cf = 0.047 af

Overall Storage Efficiency = 50.5%

Overall System Size = 81.52' x 21.50' x 2.33'

66 Chambers  
 151.5 cy Field  
 115.4 cy Stone



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

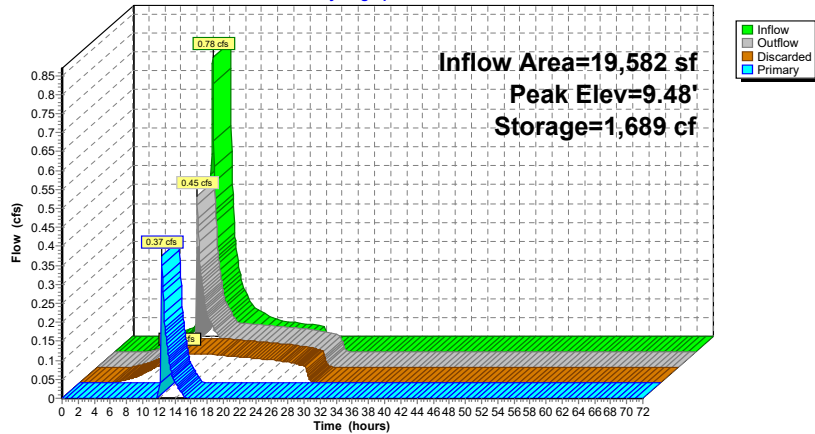
NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 401

**Pond INF-1: INFILTRATION SYSTEM #1**

Hydrograph



**14850\_Proposed-Drainage-Areas**

Prepared by {enter your company name here}

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Printed 4/6/2022

Page 402

**Summary for Pond INF-2: INFILTRATION SYSTEM #2**

Inflow Area = 143,309 sf, 49.69% Impervious, Inflow Depth = 2.38" for NOAA 25-yr event  
 Inflow = 3.65 cfs @ 12.13 hrs, Volume= 28,364 cf  
 Outflow = 3.30 cfs @ 12.17 hrs, Volume= 28,364 cf, Atten= 10%, Lag= 2.4 min  
 Discarded = 0.12 cfs @ 12.16 hrs, Volume= 9,579 cf  
 Primary = 3.18 cfs @ 12.17 hrs, Volume= 18,785 cf  
 Routed to Reach B : PARKING LOT B OVERFLOW

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.87' @ 12.16 hrs Surf.Area= 2,268 sf Storage= 3,518 cf

Plug-Flow detention time= 125.5 min calculated for 28,344 cf (100% of inflow)  
 Center-of-Mass det. time= 125.9 min ( 984.2 - 858.3 )

Volume	Invert	Avail.Storage	Storage Description
#1A	7.50'	1,790 cf	<b>25.25'W x 89.06'L x 3.50'H Field A</b> 7,870 cf Overall - 2,756 cf Embedded = 5,114 cf x 35.0% Voids
#2A	8.00'	2,756 cf	<b>ADS_StormTech SC-740 +Cap</b> x 60 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 60 Chambers in 5 Rows
#3	7.50'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		4,684 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	7.50'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 5.50'
#2	Primary	8.00'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.00' / 7.90' S= 0.0100'/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.50'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.12 cfs @ 12.16 hrs HW=9.86' (Free Discharge)  
 1=Exfiltration ( Controls 0.12 cfs)

**Primary OutFlow** Max=3.15 cfs @ 12.17 hrs HW=9.85' (Free Discharge)  
 2=Culvert (Inlet Controls 3.15 cfs @ 5.77 fps)  
 3=Sharp-Crested Rectangular Weir(Passes 3.15 cfs of 3.39 cfs potential flow)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 403

**Pond INF-2: INFILTRATION SYSTEM #2 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-740+Cap (ADS StormTech®SC-740 with cap length)**

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf

Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

12 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 87.06' Row Length +12.0" End Stone x 2 = 89.06' Base Length

5 Rows x 51.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.25' Base Width

6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

60 Chambers x 45.9 cf = 2,756.4 cf Chamber Storage

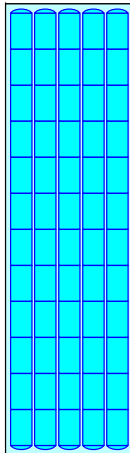
7,870.4 cf Field - 2,756.4 cf Chambers = 5,114.0 cf Stone x 35.0% Voids = 1,789.9 cf Stone Storage

Chamber Storage + Stone Storage = 4,546.3 cf = 0.104 af

Overall Storage Efficiency = 57.8%

Overall System Size = 89.06' x 25.25' x 3.50'

60 Chambers  
291.5 cy Field  
189.4 cy Stone



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

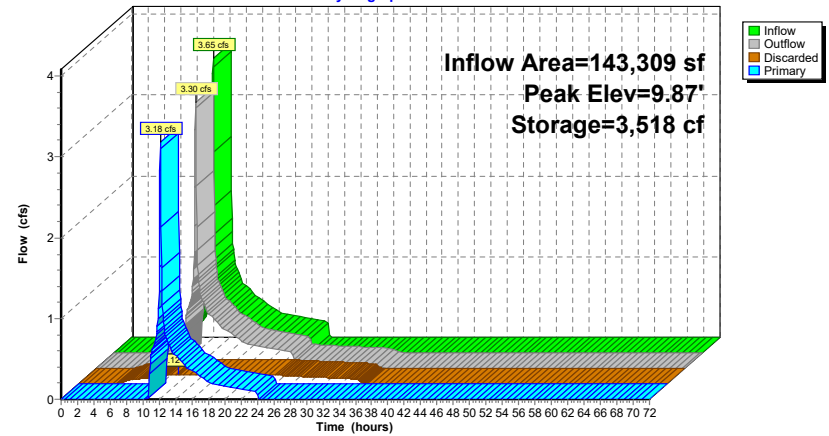
Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 404

**Pond INF-2: INFILTRATION SYSTEM #2**

Hydrograph



**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 405

**Summary for Pond INF3: INFILTRATION SYSTEM #1**

Inflow Area = 19,743 sf, 50.83% Impervious, Inflow Depth = 3.66" for NOAA 25-yr event  
 Inflow = 1.21 cfs @ 12.13 hrs, Volume= 6,020 cf  
 Outflow = 1.20 cfs @ 12.14 hrs, Volume= 6,020 cf, Atten= 1%, Lag= 0.9 min  
 Discarded = 0.05 cfs @ 12.14 hrs, Volume= 3,178 cf  
 Primary = 1.15 cfs @ 12.14 hrs, Volume= 2,842 cf  
 Routed to Reach P ST : PORTLAND STREET DRAINAGE

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 9.67' @ 12.14 hrs Surf.Area= 1,113 sf Storage= 1,000 cf

Plug-Flow detention time= 132.5 min calculated for 6,016 cf (100% of inflow)  
 Center-of-Mass det. time= 132.7 min ( 948.3 - 815.6 )

Volume	Invert	Avail.Storage	Storage Description
#1A	8.10'	686 cf	<b>18.17'W x 60.16'L x 2.33'H Field A</b> 2,550 cf Overall - 590 cf Embedded = 1,960 cf x 35.0% Voids
#2A	8.60'	590 cf	<b>ADS_StormTech SC-310 +Cap</b> x 40 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 40 Chambers in 5 Rows
#3	8.10'	137 cf	<b>5.00'D x 7.00'H Vertical Cone/Cylinder</b>
		1,413 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	8.10'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.10'
#2	Primary	8.40'	<b>10.0" Round Culvert</b> L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 8.40' / 8.30' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.55 sf
#3	Device 2	9.50'	<b>5.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)

**Discarded OutFlow** Max=0.05 cfs @ 12.14 hrs HW=9.67' (Free Discharge)  
 ↳1=Exfiltration ( Controls 0.05 cfs)

**Primary OutFlow** Max=1.13 cfs @ 12.14 hrs HW=9.67' (Free Discharge)  
 ↳2=Culvert (Passes 1.13 cfs of 2.36 cfs potential flow)  
 ↳3=Sharp-Crested Rectangular Weir(Weir Controls 1.13 cfs @ 1.34 fps)

**14850\_Proposed-Drainage-Areas**

NOAA 24-hr C NOAA 25-yr Rainfall=6.04"

Prepared by {enter your company name here}

Printed 4/6/2022

HydroCAD® 10.10-7a s/n 00546 © 2021 HydroCAD Software Solutions LLC

Page 406

**Pond INF3: INFILTRATION SYSTEM #1 - Chamber Wizard Field A**

**Chamber Model = ADS\_StormTechSC-310 +Cap (ADS StormTech®SC-310 with cap length)**

Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf

Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap

34.0" Wide + 6.0" Spacing = 40.0" C-C Row Spacing

8 Chambers/Row x 7.12' Long +0.60' Cap Length x 2 = 58.16' Row Length +12.0" End Stone x 2 = 60.16' Base Length

5 Rows x 34.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 18.17' Base Width

6.0" Stone Base + 16.0" Chamber Height + 6.0" Stone Cover = 2.33' Field Height

40 Chambers x 14.7 cf = 589.7 cf Chamber Storage

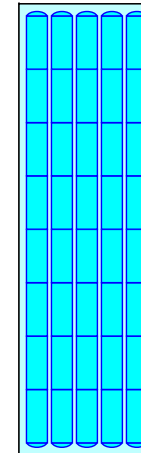
2,550.1 cf Field - 589.7 cf Chambers = 1,960.4 cf Stone x 35.0% Voids = 686.2 cf Stone Storage

Chamber Storage + Stone Storage = 1,275.8 cf = 0.029 af

Overall Storage Efficiency = 50.0%

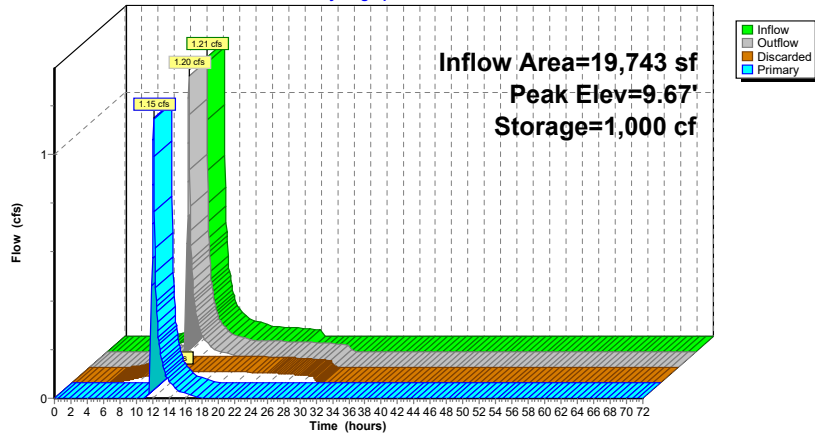
Overall System Size = 60.16' x 18.17' x 2.33'

40 Chambers  
 94.4 cy Field  
 72.6 cy Stone



**Pond INF3: INFILTRATION SYSTEM #1**

Hydrograph



**APPENDIX E**

---

**Long-Term Pollution Prevention and Stormwater Operation and Maintenance Plan**



## **LONG-TERM POLLUTION PREVENTION PLAN AND STORMWATER OPERATION AND MAINTENANCE PLAN**

East Beach, New Bedford, MA

### **TABLE OF CONTENTS**

<b>1.0 INTRODUCTION .....</b>	<b>1</b>
<b>2.0 LONG-TERM POLLUTION PREVENTION PLAN .....</b>	<b>2</b>
2.1 Storage of Hazardous Materials .....	2
2.2 Storage of Waste Products .....	2
2.3 Spill Prevention and Response .....	2
2.4 Minimize Soil Erosion .....	2
2.5 Maintenance of Lawns, Gardens, and other Landscaped Areas .....	2
2.6 Management of Deicing Chemicals and Snow .....	3
2.7 Coordination with other Permits and Requirements .....	3
<b>3.0 STORMWATER MANAGEMENT SYSTEM OPERATION AND MAINTENANCE PLAN</b>	<b>3</b>
3.1 Introduction .....	3
3.2 Stormwater Operation and Maintenance Requirements .....	4
Deep Sump and Hooded Catch Basins .....	4
Area Drains .....	4
Isolator Row .....	5
Subsurface Infiltration Structures .....	5
Bioretention Areas .....	5
3.3 Repair of the Stormwater Management System .....	5
3.4 Reporting .....	6
<b>STORMWATER MANAGEMENT SYSTEM INSPECTION FORM.....</b>	<b>1</b>
 <b>ATTACHMENT A – ISOLATOR ROW MAINTENANCE MANUAL</b>	

## **1.0 INTRODUCTION**

---

The purpose of this document is to specify the pollution prevention measures and stormwater management system operation and maintenance for the Smith Beach site. The Responsible Party indicated below shall implement the management practices outlined in this document and proactively conduct operations at the project site in an environmentally responsible manner. Compliance with this Manual does not in any way dismiss the responsible party, owner, property manager, or occupants from compliance with other applicable federal, state or local laws.

Responsible Party: City of New Bedford

This Document has been prepared in compliance with Standards 4 and 9 of the 2008 Massachusetts Department of Environmental Protection (MassDEP) Stormwater Management Standards, which state:

### Standard 4:

The Long Term Pollution Prevention Plan shall include the proper procedures for the following:

- Good housekeeping
- Storing materials and waste products inside or under cover
- Vehicle washing
- Routine inspections of stormwater best management practices
- Spill prevention and response
- Maintenance of lawns, gardens, and other landscaped areas
- Storage and use of slow-release phosphorous free fertilizer
- Prohibition of herbicides, fungicides and pesticides
- Pet waste management
- Operation and management of septic systems
- Proper management of deicing chemicals and snow

### Standard 9:

The Long-Term Operation and Maintenance Plan shall at a minimum include:

- Stormwater management system(s) owner(s)
- The party or parties responsible for operation and maintenance, including how future property owners shall be notified of the presence of the stormwater management system and the requirement for operation and maintenance
- The routine and non-routine maintenance tasks to be undertaken after construction is complete and a schedule for implementing those tasks
- A plan that is drawn to scale and shows the location of all stormwater BMPs in each treatment train along with the discharge point
- A description and delineation of public safety features
- An estimated operations and maintenance budget

## **2.0 LONG-TERM POLLUTION PREVENTION PLAN**

---

The Responsible Party shall implement the following good housekeeping procedures at the project site to reduce the possibility of accidental releases and to reduce safety hazards.

### **2.1 Storage of Hazardous Materials**

To prevent leaks and spills, keep hazardous materials and waste products under cover or inside. Use drip pans or spill containment systems to prevent chemicals from entering the drainage system. Inspect storage areas for materials and waste products at least once per year to determine amount and type of the material on site, and if the material requires disposal.

Securely store liquid petroleum products and other liquid chemicals in federally- and state-approved containers. Restrict access to maintenance personnel and administrators.

### **2.2 Storage of Waste Products**

Collect and store all waste materials in securely lidded dumpster(s) or other secure containers as applicable to the material. Keep dumpster lids closed and the areas around them clean. Do not fill the dumpsters with liquid waste or hose them out. Sweep areas around the dumpster regularly and put the debris in the garbage, instead of sweeping or hosing it into the parking lot. Legally dispose of collected waste on a regular basis.

Segregate liquid wastes from solid waste and recycle through hazardous waste disposal companies, whenever possible. Contact a hazardous waste hauler for proper disposal to a hazardous waste collection center.

### **2.3 Spill Prevention and Response**

Implement spill response procedures for releases of significant materials such as fuels, oils, or chemical materials onto the ground or other area that could reasonably be expected to discharge to surface or groundwater.

- For minor spills, keep fifty (50) gallon spill control kits and Speedy Dry at all shop and work areas.
- Immediately contact applicable Federal, State, and local agencies for reportable quantities as required by law.
- Immediately perform applicable containment and cleanup procedures following a spill release.
- Promptly remove and dispose of all material collected during the response in accordance with Federal, State and local requirements. A licensed emergency response contractor may be required to assist in cleanup of releases depending on the amount of the release, and the ability of the Contractor to perform the required response.
- Reportable quantities of chemicals, fuels, or oils are established under the Clean Water Act and enforced through Massachusetts Department of Environmental Protection (DEP).

### **2.4 Minimize Soil Erosion**

Soil erosion facilitates mechanical transport of nutrients, pathogens, and organic matter to surface water bodies. Repair all areas where erosion is occurring throughout the project site. Stabilize bare soil with riprap, seed, mulch, or vegetation.

### **2.5 Maintenance of Lawns, Gardens, and other Landscaped Areas**

Pesticides, herbicide and fungicide shall not be used in the landscaped areas associated with the

project site and shall not be stored on-site. Slow-release, phosphorus free fertilizer is allowed. Dumping of lawn wastes, brush or leaves or other materials or debris is not permitted in any Resource Area. Grass clippings pruned branches and any other landscaped waste should be disposed of or composted in an appropriate location. No irrigation shall be used in the landscaped areas for this project.

## **2.6 Management of Deicing Chemicals and Snow**

The qualified contractor selected for snow plowing and deicing shall be made fully aware of the requirements of this section.

No road salt (sodium chloride) shall be stored on-site. The use of magnesium chloride de-icing product with a 0.5 to 1.0 percent sodium chloride mix for snow and ice treatment is permitted. The product shall be stored in a locked room inside the building and shall be used at exterior stairs and walkways. The snow plow contractor shall adhere to these magnesium chloride use and storage requirements.

Snow shall not be stockpiled in the riverfront area or the 100-foot Buffer Zone, catch basins, or area drains. In severe conditions where snow cannot be stockpiled on site, the snow shall be removed from the site and properly disposed of in accordance with DEP Guideline BRP601-01.

Use of sand is not permitted within the porous pavement parking lot. If sand is applied, the snow plowed from impervious areas shall not be stored on porous asphalt.

Porous asphalt is proposed in the parking lot, as indicated on the plans. Porous asphalt performs well in cold climates and can reduce meltwater runoff during the snowmelt period; however there are specific winter management techniques that must be followed for porous asphalt systems.

The porous asphalt areas shall be maintained during snow events as provided below:

- Apply anti-icing treatments only when absolutely necessary (in extreme events). It is not anticipated that deicing chemicals will be required for typical winter events.
- Plow as needed after storm events. Avoid scarifying the porous asphalt surface. Special plow blades should be used whenever possible. Raised blade is not recommended.
- Apply the minimum amount of deicing agents during and after storms required to control compact snow and ice that are not removed by plowing.
- Do not apply sand in porous asphalt areas “No Sanding” signs shall be posted before the first snowfall and maintenance and snow removal contractors shall be made aware of this requirement.

Before winter begins, the property owner and the contractor shall review snow plowing, deicing, and stockpiling procedures. Areas designated for stockpiling should be cleaned of any debris. Street and parking lot sweeping should be followed in accordance with the Operation and Maintenance Plan.

## **2.7 Coordination with other Permits and Requirements**

Certain conditions of other approvals affecting the long term management of the property shall be considered part of this Long Term Pollution Prevention Plan. The Owner shall become familiar with those documents and comply with the guidelines set forth in those documents.

## **3.0 STORMWATER MANAGEMENT SYSTEM OPERATION AND MAINTENANCE PLAN**

### **3.1 Introduction**

This Operation and Maintenance Plan (O&M Plan) for Smith Beach site is required under Standard 9 of the 2008 MassDEP Stormwater Handbook to provide best management practices for implementing

maintenance activities for the stormwater management system in a manner that minimizes impacts to wetland resource areas.

The Owner shall implement this O&M Plan and proactively conduct operations at the site in an environmentally responsible manner. Compliance with this O&M Plan does not in any way dismiss the Owner from compliance with other applicable Federal, State or local laws.

Routine maintenance during construction and post-development phases of the project, as defined in the Operation and Maintenance Plan, shall be permitted without amendment to the Order of Conditions. A continuing condition in the Certificate of Compliance shall ensure that maintenance can be performed without triggering further filings under the Wetlands Protection Act.

All stormwater best management practices (BMPs) shall be operated and maintained in accordance with the design plans and the Operation and Maintenance Plan approved by the issuing authority. The Owner shall:

- a. Maintain an operation and maintenance log for the last three years, including inspections, repairs, replacement and disposal (for disposal the log shall indicate the type of material and the disposal location). This is a rolling log in which the responsible party records all operation and maintenance activities for the past three years.
- b. Make this log available to MassDEP and the Conservation Commission upon request; and
- c. Allow members and agents of the MassDEP and the Conservation Commission to enter and inspect the premises to evaluate and ensure that the Owner complies with the Operation and Maintenance requirements for each BMP.

### **3.2 Stormwater Operation and Maintenance Requirements**

Inspect and maintain the stormwater management system as directed below. Refer to the Stormwater Management System Location Map (Figure 1) for the location of each component of the system. Repairs to any component of the system shall be made as soon as possible to prevent any potential pollutants (including silt) from entering the resource areas.

#### Deep Sump and Hooded Catch Basins

Inspect or clean catch basins four times per year and at the end of foliage and snow-removal seasons. Other inspection and maintenance requirements include:

- Remove organic material, sediment and hydrocarbons four times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin.
- Always clean out catch basins after street sweeping. If any evidence of hydrocarbons is found during inspection, immediately remove the material using absorbent pads or other suitable measures and dispose of legally. Remove other accumulated debris as necessary.
- If handling runoff from land uses with higher potential pollutant loads or discharging runoff near or to a critical area, more frequent cleaning may be necessary.
- Transport and disposal of accumulated sediment off-site shall be in accordance with applicable local, state and federal guidelines and regulations.

#### Area Drains

Inspect area drains at least once per month and remove debris from the grate. Clean out accumulated sediments at least once per year and more frequently as necessary.

Isolator Row

Maintain water quality units according the recommendations set forth by the manufacturer. Refer to the Isolator Row O&M Manual, provided as Attachment A.

Subsurface Infiltration Structures

Inspect subsurface detention/infiltration structures twice per year. Inspect the inlets and observation ports to determine if there is accumulated sediment within the system. Remove all debris and accumulated sediment that may clog the system.

Bioretention Areas

Perform annual maintenance of all components of the bioretention area, including plants, soil, and mulch. Table 1, below, outlines recommended maintenance activities.

**Table 1. Bioretention area maintenance recommendations**

<b>Location</b>	<b>Description</b>	<b>Frequency</b>	<b>Time of Year</b>
Surface	Inspect and remove trash	Monthly	Year round
Soil	Inspect and repair erosion	Monthly	Year round
Organic Layer	Remulch void areas	Annually	Spring
	Remove previous mulch layer before applying new layer (optional)	Annually	Spring
Plants	Water vegetation at end of day for 14 consecutive days after planting	Immediately after planting	As needed
	Fertilize	Annually	Spring
	Mow grass	2 to 12 times per year	As needed
	Remove and replace all dead and diseased vegetation that cannot be treated	Annually	Spring
	Treat all diseased trees and shrubs	As needed	Variable

During and after storm events, record the length of time standing water remains in the bioretention areas. If the time is greater than 72 hours, thoroughly inspect the basins for signs of clogging and develop a corrective action plan. The corrective action plan, prepared by a qualified professional, will outline procedures to restore infiltrative function. The owner of the site shall take immediate action to implement these corrective measures. Inspect pretreatment devices and bioretention cells regularly for sediment build-up, structural damage, and standing water. Never store snow in bioretention areas.

**3.3 Repair of the Stormwater Management System**

The stormwater management system shall be maintained. The repair of any component of the system shall be made as soon as possible to prevent any potential pollutants including silt from entering the resource areas or the existing closed drainage system.

### **3.4 Reporting**

The City shall maintain a record of drainage system inspections and maintenance (per this Plan) and submit a yearly report to the New Bedford Conservation Commission.

**STORMWATER MANAGEMENT SYSTEM INSPECTION FORM**

---

<b>East Beach New Bedford, MA</b>		Inspected by: _____ Date: _____
Component	Status/Inspection	Action Taken
Deep Sump Catch Basins, Area Drains and Drain Manholes		
Subsurface Infiltration System		
Bioretention Basins		
Isolator Row		
General site conditions – evidence of erosion, etc.		

**SUBMIT COPIES OF STORMWATER MANAGEMENT SYSTEM INSPECTION FORM TO THE NEW BEDFORD CONSERVATION COMMISSION WITH THE YEARLY REPORT**



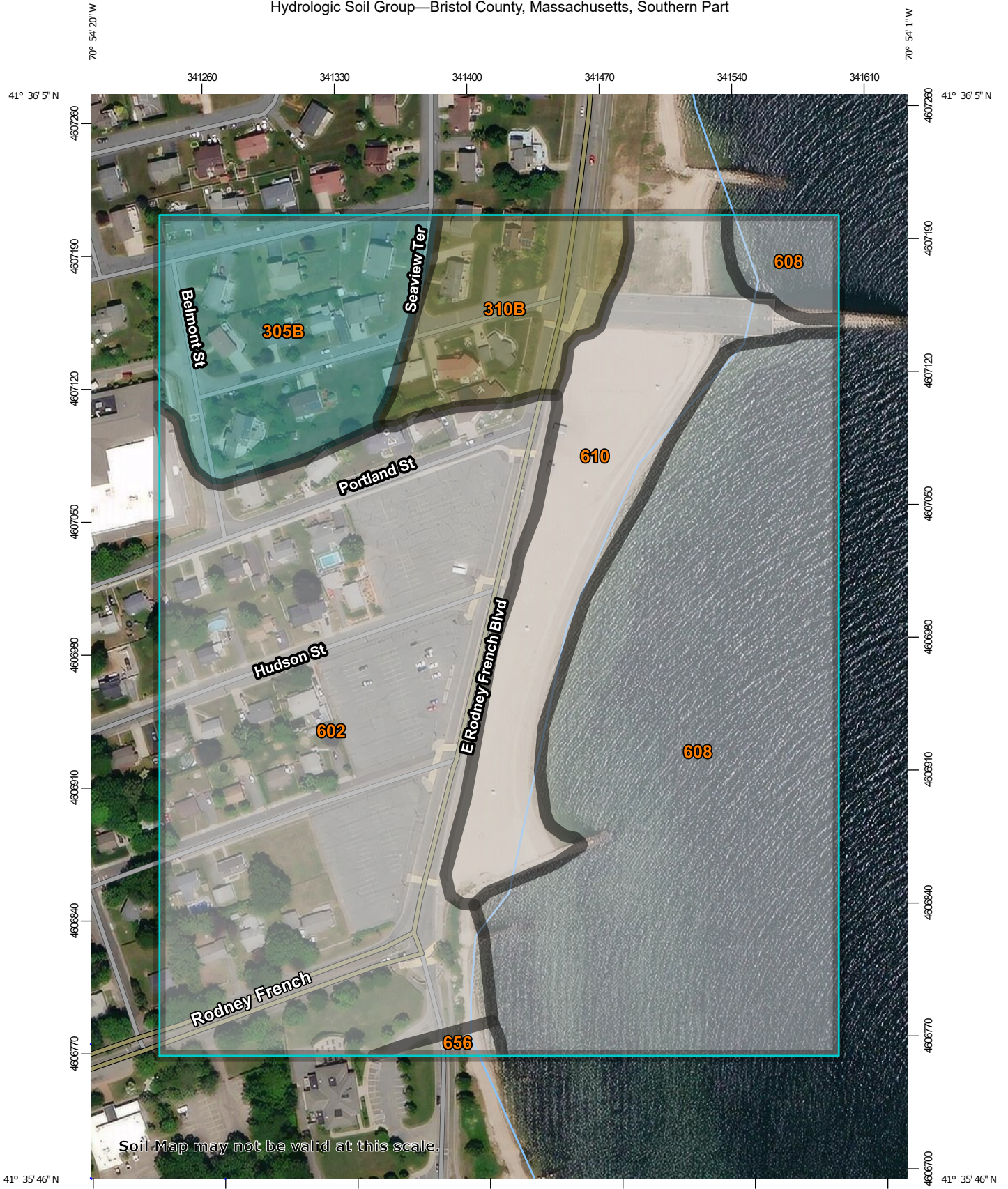
## **APPENDIX F**

---

### **Soil Investigations**

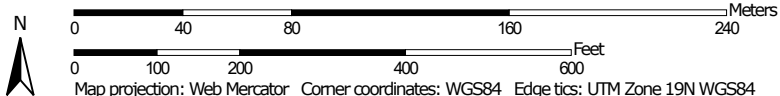
NRCS Soil Maps and Descriptions  
Soil Test Pit Logs

Hydrologic Soil Group—Bristol County, Massachusetts, Southern Part



Soil Map may not be valid at this scale.

Map Scale: 1:2,780 if printed on A portrait (8.5" x 11") sheet.




Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84



## MAP LEGEND

### Area of Interest (AOI)









 Area of Interest (AOI)

### Soils

#### Soil Rating Polygons





 A  
 A/D  
 B  
 B/D  
 C  
 C/D  
 D  
 Not rated or not available

#### Soil Rating Lines


 A  
 A/D  
 B  
 B/D  
 C  
 C/D  
 D  
 Not rated or not available

#### Soil Rating Points






 A  
 A/D  
 B  
 B/D

 C  
 C/D  
 D  
 Not rated or not available

### Water Features

 Streams and Canals

### Transportation

 Rails  
 Interstate Highways  
 US Routes  
 Major Roads  
 Local Roads

### Background

 Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Bristol County, Massachusetts, Southern Part  
 Survey Area Data: Version 15, Sep 2, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 31, 2009—Oct 15, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
305B	Paxton fine sandy loam, 3 to 8 percent slopes	C	4.1	10.5%
310B	Woodbridge fine sandy loam, 3 to 8 percent slopes	C/D	2.4	6.2%
602	Urban land		14.1	35.8%
608	Water, ocean		13.7	34.7%
610	Beaches, sand		4.9	12.4%
656	Udorthents - Urban land complex		0.2	0.4%
<b>Totals for Area of Interest</b>			<b>39.4</b>	<b>100.0%</b>



## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

### C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: 1 Hole #      2/24/2022 Date      9:15 Time      30/cloudy Weather      \_\_\_\_\_ Latitude      \_\_\_\_\_ Longitude:

1. Land Use Parking Lot (e.g., woodland, agricultural field, vacant lot, etc.)      - Vegetation      - Surface Stones (e.g., cobbles, stones, boulders, etc.)      2-5% Slope (%)

Description of Location: Southern parking lot

2. Soil Parent Material: - Landform - Position on Landscape (SU, SH, BS, FS, TS) -

3. Distances from:      Open Water Body 150' feet      Drainage Way - feet      Wetlands - feet  
Property Line - feet      Drinking Water Well - feet      Other - feet

4. Unsuitable Materials Present:  Yes  No      If Yes:  Disturbed Soil       Fill Material       Weathered/Fractured Rock       Bedrock

5. Groundwater Observed:  Yes  No      If yes: \_\_\_\_\_ Depth Weeping from Pit      \_\_\_\_\_ Depth Standing Water in Hole

#### Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-4	Pavement	-	-	-	-	-	-	-	-	-	-
4-19	Fill	Loamy Sand	10 YR 5/6	-	-	-	30%	-			
19-30	Fill	Sandy Loam	10 YR 4/1	-	-	-	15%	-			
30-36	Ab	Sandy Loam	10 YR 2/1	-	-	-	-				
36-96	C1	Sandy Loam	10 YR 5/1	-	-	-	10%				

Additional Notes:



**Commonwealth of Massachusetts**  
**City/Town of**

**Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal**

**C. On-Site Review** (minimum of two holes required at every proposed primary and reserve disposal area)

**Deep Observation Hole Number:** 2      2/24/2022      10:15      30/cloudy  
Hole #      Date      Time      Weather      Latitude      Longitude:

1. Land Use Parking Lot      -      -      -  
(e.g., woodland, agricultural field, vacant lot, etc.)      Vegetation      Surface Stones (e.g., cobbles, stones, boulders, etc.)      Slope (%)

Description of Location: Southern parking lot

2. Soil Parent Material: -      -      -  
Landform      Position on Landscape (SU, SH, BS, FS, TS)

3. Distances from:      Open Water Body 150' feet      Drainage Way - feet      Wetlands - feet  
    Property Line - feet      Drinking Water Well - feet      Other - feet

4. Unsuitable Materials Present:  Yes  No      If Yes:  Disturbed Soil     Fill Material     Weathered/Fractured Rock     Bedrock

5. Groundwater Observed:  Yes     No      If yes: \_\_\_\_\_ Depth Weeping from Pit      \_\_\_\_\_ Depth Standing Water in Hole

**Soil Log**

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-4	Pavement	-	-	-	-	-	-	-	-	-	-
4-25	Fill	Loamy Sand	10 YR 5/6	-	-	-	30%	-	Massive	Friable	
25-35	Ab	Sandy Loam	10 YR 2/1	-	-	-	-	-	Massive	Friable	
35-96	C1	Sandy Loam	10 YR 4/1	-	-	-	10%		Massive	Friable	

Additional Notes:



## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

### C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: 3 Hole #      2/24/2022 Date      1:15 Time      30/cloudy Weather      \_\_\_\_\_ Latitude      \_\_\_\_\_ Longitude:

1. Land Use Parking Lot (e.g., woodland, agricultural field, vacant lot, etc.)      - Vegetation      - Surface Stones (e.g., cobbles, stones, boulders, etc.)      2-5% Slope (%)

Description of Location: Middle parking lot

2. Soil Parent Material: - Landform - Position on Landscape (SU, SH, BS, FS, TS) -

3. Distances from:      Open Water Body 150' feet      Drainage Way - feet      Wetlands - feet  
Property Line - feet      Drinking Water Well - feet      Other - feet

4. Unsuitable Materials Present:  Yes  No      If Yes:  Disturbed Soil       Fill Material       Weathered/Fractured Rock       Bedrock

5. Groundwater Observed:  Yes  No      If yes: \_\_\_\_\_ Depth Weeping from Pit      \_\_\_\_\_ Depth Standing Water in Hole

#### Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-4	Pavement	-	-	-	-	-	-	-	-	-	-
4-24	Fill	Loamy Sand	10 YR 5/6	-	-	-	30%	0%	Massive	Friable	
24-33	Ab	Sandy Loam	10 YR 2/1	-	-	-		10%	Massive	Friable	
33-70	C1	Sandy Loam	10 YR 4/1	-	-	-	0%		Massive	Friable	
							0%				

Additional Notes:



Commonwealth of Massachusetts  
City/Town of

**Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal**

**C. On-Site Review** (minimum of two holes required at every proposed primary and reserve disposal area)

Deep Observation Hole Number: 4 Hole #      2/24/2022 Date      1:55 Time      30/cloudy Weather      \_\_\_\_\_ Latitude      \_\_\_\_\_ Longitude:

1. Land Use Parking Lot (e.g., woodland, agricultural field, vacant lot, etc.)      - Vegetation      - Surface Stones (e.g., cobbles, stones, boulders, etc.)      2-5% Slope (%)

Description of Location: Middle parking lot

2. Soil Parent Material: - Landform - Position on Landscape (SU, SH, BS, FS, TS) -

3. Distances from:      Open Water Body 150' feet      Drainage Way - feet      Wetlands - feet  
Property Line - feet      Drinking Water Well - feet      Other - feet

4. Unsuitable Materials Present:  Yes  No      If Yes:  Disturbed Soil       Fill Material       Weathered/Fractured Rock       Bedrock

5. Groundwater Observed:  Yes  No      If yes: \_\_\_\_\_ Depth Weeping from Pit      \_\_\_\_\_ Depth Standing Water in Hole

**Soil Log**

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-4	Pavement	-	-	-	-	-	-	-	-	-	-
4-19	Fill	Loamy Sand	10 YR 5/6	-	-	-	30%	0%	Massive	Friable	
19-30		Sandy Loam	10 YR 4/1	-	-	-		10%	Massive	Friable	
30-36	Ab	Sandy Loam	10 YR 2/1	-	-	-	10%		Massive	Friable	
	Bw						20%				

Additional Notes:





## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

### C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: 5      2/24/2022      11:00      30/cloudy  
Hole #      Date      Time      Weather      Latitude      Longitude:

1. Land Use Parking Lot      -      -      -  
(e.g., woodland, agricultural field, vacant lot, etc.)      Vegetation      Surface Stones (e.g., cobbles, stones, boulders, etc.)      Slope (%)  
 Description of Location: North parking lot

2. Soil Parent Material: -      -      -  
Landform      Position on Landscape (SU, SH, BS, FS, TS)

3. Distances from:      Open Water Body 150' feet      Drainage Way - feet      Wetlands - feet  
    Property Line - feet      Drinking Water Well - feet      Other - feet

4. Unsuitable Materials Present:  Yes  No      If Yes:  Disturbed Soil  Fill Material  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed:  Yes  No      If yes: - Depth Weeping from Pit      56" Depth Standing Water in Hole

#### Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-4	Pavement	-	-	-	-	-	-	-	-	-	-
4-12	Fill	Loamy Sand	10 YR 5/6	-	-	-	30%	-	Massive	Friable	
12-44		Sandy Loam	10 YR 3/3	-	-	-		15%	Massive	Friable	
44-64	C2		10 YR 6/4	-	-	-	0%		Massive	Friable	

Additional Notes:



## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

### C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: 6 Hole #      2/24/2022 Date      12:30 Time      30/cloudy Weather      - Latitude      - Longitude:

1. Land Use Parking Lot (e.g., woodland, agricultural field, vacant lot, etc.)      - Vegetation      - Surface Stones (e.g., cobbles, stones, boulders, etc.)      2-5% Slope (%)

Description of Location: North parking lot

2. Soil Parent Material: - Landform - Position on Landscape (SU, SH, BS, FS, TS) -

3. Distances from:      Open Water Body 150' feet      Drainage Way - feet      Wetlands - feet  
Property Line - feet      Drinking Water Well - feet      Other - feet

4. Unsuitable Materials Present:  Yes  No      If Yes:  Disturbed Soil       Fill Material       Weathered/Fractured Rock       Bedrock

5. Groundwater Observed:  Yes  No      If yes: 83" Depth Weeping from Pit      93" Depth Standing Water in Hole

#### Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-4	Pavement	-	-	-	-	-	-	-	-	-	-
4-17	Fill	Loamy Sand	10 YR 5/6	-	-	-	30%		Massive	Friable	-
17-25	Ab	Sandy Loam	10 YR 2/1	-	-	-	10%		Massive	Friable	-
25-36				-	-	-		0%	Massive	Friable	Large stones
	C1		10 YR 6/2				20%	10%			

Additional Notes:



## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

### C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: 7 Hole #      3/3/2022 Date      9:00 Time      38/sunny Weather      \_\_\_\_\_ Latitude      \_\_\_\_\_ Longitude:

1. Land Use Parking Lot (e.g., woodland, agricultural field, vacant lot, etc.)      - Vegetation      - Surface Stones (e.g., cobbles, stones, boulders, etc.)      2-5% Slope (%)

Description of Location: South parking lot

2. Soil Parent Material: - Landform - Position on Landscape (SU, SH, BS, FS, TS) -

3. Distances from: Open Water Body 200' feet      Drainage Way - feet      Wetlands - feet  
Property Line - feet      Drinking Water Well - feet      Other - feet

4. Unsuitable Materials Present:  Yes  No      If Yes:  Disturbed Soil       Fill Material       Weathered/Fractured Rock       Bedrock

5. Groundwater Observed:  Yes  No      If yes: \_\_\_\_\_ Depth Weeping from Pit      \_\_\_\_\_ Depth Standing Water in Hole

#### Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-4	Pavement	-	-	-	-	-	-	-	-	-	-
4-22	Fill	Sand	10 YR 5/4	-	-	-	25%		Massive	Friable	
22-35	Ab	Sandy Loam	10 YR 2/1	-	-	-			Massive	Friable	
35-65			10 YR 4/6	-	-	-	15%		Massive	Friable	
								10%			

Additional Notes:



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

## C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: 8      3/3/2022      10:30      38/sunny      \_\_\_\_\_  
Hole #      Date      Time      Weather      Latitude

1. Land Use Parking Lot      \_\_\_\_\_      \_\_\_\_\_      \_\_\_\_\_      \_\_\_\_\_  
(e.g., woodland, agricultural field, vacant lot, etc.)      Vegetation      Surface Stones (e.g., cobbles, stones, boulders, etc.)      Longitude: 2-5%

Description of Location: Middle parking lot

2. Soil Parent Material: -      \_\_\_\_\_      \_\_\_\_\_  
Landform      Position on Landscape (SU, SH, BS, FS, TS)

3. Distances from:      Open Water Body 200' feet      Drainage Way - feet      Wetlands - feet  
    Property Line - feet      Drinking Water Well - feet      Other - feet

4. Unsuitable Materials Present:  Yes  No      If Yes:  Disturbed Soil     Fill Material       Weathered/Fractured Rock     Bedrock

5. Groundwater Observed:  Yes     No      If yes: \_\_\_\_\_ Depth Weeping from Pit      \_\_\_\_\_ Depth Standing Water in Hole

### Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-4	Pavement	-	-	-	-	-	-	-	-	-	-
4-23	Fill	Sand	10 YR 5/4	-	-	-	25%		Massive		
23-33	Ab	Sandy Loam	10 YR 2/1	-	-	-			Massive		
33-55			10 YR 4/6	-	-	-	10%		Massive		
								10%			

Additional Notes:



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

## C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: 9 Hole #      3/3/2022 Date      12:30 Time      38/sunny Weather      - Latitude      - Longitude:

1. Land Use Parking Lot (e.g., woodland, agricultural field, vacant lot, etc.)      - Vegetation      - Surface Stones (e.g., cobbles, stones, boulders, etc.)      2-5% Slope (%)

Description of Location: North parking lot

2. Soil Parent Material: - Landform - Position on Landscape (SU, SH, BS, FS, TS) -

3. Distances from:      Open Water Body 200' feet      Drainage Way - feet      Wetlands - feet  
    Property Line - feet      Drinking Water Well - feet      Other - feet

4. Unsuitable Materials Present:  Yes  No      If Yes:  Disturbed Soil       Fill Material       Weathered/Fractured Rock       Bedrock

5. Groundwater Observed:  Yes  No      If yes: 78" Depth Weeping from Pit      \_\_\_\_\_ Depth Standing Water in Hole

### Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-4	Pavement	-	-	-	-	-	-	-	-	-	-
4-13	Fill	Sandy Loam	10 YR 4/1	-	-	-	20%		Massive	Friable	
13-27	Bw	Loamy Sand	10 YR 4/6	-	-	-			Massive	Friable	-
27-84			10 YR 5/3	-	-	-	20%	15%	Massive	Friable	

Additional Notes: