Transportation Impact Assessment

Proposed Child and Family Services Counseling and Treatment Center 947 & 965 Church Street New Bedford, Massachusetts

Prepared for:



Bourne, Massachusetts

January 2021

Prepared by:





Dear Reviewer:

This letter shall certify that this *Transportation Impact Assessment* has been prepared under my direct supervision and responsible charge. I am a Registered Professional Engineer (P.E.) in the Commonwealth of Massachusetts (Massachusetts P.E. No. 38871, Civil) and hold Certification as a Professional Traffic Operations Engineer (PTOE) from the Transportation Professional Certification Board, Inc. (TPCB), an independent affiliate of the Institute of Transportation Engineers (ITE) (PTOE Certificate No. 993). I am also a Fellow of the Institute of Transportation Engineers (FITE).

Sincerely,

VANASSE & ASSOCIATES, INC.

effrey S. Dirk

effrey S. Dirk, P.E., PTOE, FITE

Managing Partner

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EXECUTIVE SUMMARY

Vanasse & Associates, Inc. (VAI) has conducted a Transportation Impact Assessment (TIA) in order to determine the potential impacts on the transportation infrastructure associated with the proposed renovation of the Regal House Classic Furniture store building located at 965 Church Street in New Bedford, Massachusetts, and the construction of supporting parking on a portion of the abutting property at 947 Church Street to accommodate a counseling and treatment center for Child and Family Services (hereafter referred to as the Project). This assessment was prepared in consultation with the City of New Bedford and the Massachusetts Department of Transportation (MassDOT), and was performed in accordance with MassDOT's *Transportation Impact Assessment (TIA) Guidelines* and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports.

Based on this assessment, we have concluded the following with respect to the Project:

- 1. Using trip-generation statistics published by the Institute of Transportation Engineers (ITE), the Project is expected to generate approximately 638 vehicle trips on an average weekday (two-way 24-hour volume), with 50 vehicle trips expected during the weekday morning peak-hour and 84 vehicle trips expected during the weekday evening peak-hour;
- 2. The Project will not result in a significant impact (increase) on motorist delays or vehicle queuing over Existing or anticipated future conditions without the Project (No-Build conditions), with the majority of the movements at the study intersections shown to operate at a level-of-service (LOS) of C or better under all analysis conditions, where an LOS of "D" or better is defined as "acceptable" traffic operations;
- 3. Independent of the Project, the Chaffee Street approaches to Church Street are currently or are predicted to operate at or over capacity (i.e., LOS "E" or LOS "F") during one or both peak hours, with Project-related impacts on these movements generally defined by an increase in vehicle queuing of up to four (4) vehicles;
- 4. All movements exiting the Project site are expected to operate at LOS C or better during the peak hours with minimal vehicle queuing predicted (up to one (1) vehicle);

¹Trip Generation, 10th Edition; Institute of Transportation Engineers; Washington, DC; 2017.

- 5. No apparent safety deficiencies were noted with respect to the motor vehicle crash history at the study area intersections; and
- 6. Lines of sight to and from the Project site driveway intersections were found to meet or exceed the recommended minimum distances for safe operation based on the appropriate approach speed.

In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with implementation of the recommendations that follow.

RECOMMENDATIONS

A detailed transportation improvement program has been developed that is designed to provide safe and efficient access to the Project site and address any deficiencies identified at off-site locations evaluated in conjunction with this study. The following improvements have been recommended as a part of this evaluation and, where applicable, will be completed in conjunction with the Project subject to receipt of all necessary rights, permits, and approvals.

Project Access

Access to the Project site will be provided by way of three (3) driveways that will intersect the west side of Church Street approximately 180 feet south of Chaffee Street, the south side of Chaffee Street approximately 60 feet west of Church Street and the east side of Tarkiln Hill Place approximately 180 feet south of Chaffee Street, respectively. In addition, a curbside drop-off/pick-up area will be provided along the Project site frontage on Chaffee Street outside of the traveled way and a paved access to an on-site dumpster will be provided that will intersect the south side of Chaffee Street approximately 30 feet east of Tarkiln Hill Place. The following recommendations are offered with respect to the design and operation of the Project site access and internal circulation, many of which are reflected on the Site Plans:

- ➤ The Project site driveways and circulating drives within the Project site should be a minimum of 24-feet in width and designed to accommodate the turning and maneuvering requirements of the largest anticipated responding emergency vehicle.
- ➤ Vehicles exiting the Project site should be placed under STOP-sign control with a marked STOP-line provided
- ➤ All signs and pavement markings to be installed within the Project site should conform to the applicable standards of the *Manual on Uniform Traffic Control Devices* (MUTCD).²
- A sidewalk has been provided within the Project site that extends between the building to Church Street, Chaffee Street and Tarkiln Hill Place.
- Americans with Disabilities Act (ADA) compliant wheelchair ramps should be provided at all pedestrian crossings that are constructed or modified as a part of the Project.

²Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, D.C.; 2009.

- > Signs and landscaping to be installed as a part of the Project within the intersection sight triangle areas of the Project site driveway intersections should be designed and maintained so as not to restrict lines of sight.
- Snow windrows within sight triangle areas of the Project site driveway intersections should be promptly removed where such accumulations would impede sight lines.
- > Bicycle parking should be provided at an appropriate location within the Project site.

Off-Site

In an effort to address constraints or safety concerns identified as a part of this assessment, the following off-site roadway, intersection and traffic control improvements will be advanced as a part of the Project subject to receipt of all necessary rights, permits and approvals where necessary:

- ➤ Chaffee Street A double-yellow centerline should be installed along Chaffee Street between Church Street and Tarkiln Hill Place.
- ➤ Church Street at Chafee Street A STOP-sign should be installed on the Chaffee Street eastbound approach and marked STOP-lines should be installed on both Chaffee Street approaches. In addition, a crosswalk with ADA compliant wheelchair ramps should be provided across the Chaffee Street west leg of the intersection.
- ➤ Tarkiln Hill Place at Chaffee Street A STOP-sign and marked STOP-line should be installed on the Chaffee Street approach. In addition, a crosswalk with ADA compliant wheelchair ramps should be provided across the Tarkiln Hill Place north leg of the intersection.

Transportation Demand Management

Public transportation services are provided within the study area by the Southeastern Regional Transit Authority (STRA) by way of the Route 4, *Ashley Boulevard*, and the North End Shuttle. The Route 4 bus provides service along Ashley Boulevard, Chaffee Street and Church Street, with a stop located at the Church Street/Chaffee Street intersection, opposite the Project site. The North End Shuttle provides service along Phillips Road, Church Street and Acushnet Avenue, with the closest regular stop located approximately 0.4-miles north of the Project site at Amanda Avenue (Dottin Apartments). In addition to regular stops, SRTA buses operate in a passenger demand mode ("flag stop") and will stop anywhere along the regular service route where it is safe to pick-up or discharge a passenger when requested.

In addition to fixed-route bus services, the SRTA provides Dial-a-Ride paratransit services to eligible persons that cannot use fixed-route transit all or some of the time due to a physical, cognitive or mental disability in compliance with the ADA, and provides services for seniors through the New Bedford Council on Aging (COA).

In an effort to encourage the use of alternative modes of transportation to single-occupant vehicles (SOVs), the following Transportation Demand Management (TDM) measures will be implemented as a part of the Project:

➤ A Transportation Coordinator will be designated for the Project to coordinate the elements of the TDM program;

- Information regarding public transportation services, maps, schedules and fare information will be posted in a central location and/or otherwise made available to employees and clients:
- A "welcome packet" will be provided to employees detailing available public transportation services, bicycle and walking alternatives, and commuter options available;
- > The Transportation Coordinator will facilitate a ride matching program for employees, including posting a sign-up sheet in a central location or via a link on the corporate web site and proving information in the new employee "welcome packet". In addition, preferential parking will be reserved or provided for carpools and vanpools.
- On-site amenities will be incorporated into the Project to discourage off-site trips, including providing a break-room equipped with a microwave and refrigerator; offering direct deposit of paychecks; allowing telecommuting or flexible work schedules; and other such measures to reduce overall traffic volumes and travel during peak traffic volume periods;
- > Pedestrian accommodations have been incorporated into the Project; and
- > Secure bicycle parking will be provided within the Project site.

With implementation of the aforementioned recommendations, safe and efficient access will be provided to the Project site and the Project can be accommodated within the confines of the existing and improved transportation system.

INTRODUCTION

Vanasse & Associates, Inc. (VAI) has conducted a Transportation Impact Assessment (TIA) in order to determine the potential impacts on the transportation infrastructure associated with the proposed renovation of an existing building located at 965 Church Street in New Bedford, Massachusetts, and the construction of supporting parking on a portion of the abutting property at 947 Church Street to accommodate a counseling and treatment center for Child and Family Services (hereafter referred to as the Project). This study evaluates the following specific areas as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; and identifies and analyzes existing traffic conditions and future traffic conditions, both with and without the Project, along Church Street, Chaffee Street and Tarkiln Hill Place, and at the intersections of Church Street at Chaffee Street and Tarkiln Hill Place at Chaffee Street.

PROJECT DESCRIPTION

The Project will entail the renovation of the Regal House Classic Furniture store building located at 965 Church Street in New Bedford, Massachusetts, and the construction of supporting parking on a portion of the abutting property at 947 Church Street to accommodate a counseling and treatment center for Child and Family Services. The Project site (947 and 965 Church Street) encompasses a total of approximately 2.73± acres of land that is bounded by Chaffee Street to the north, a commercial property (F.W. Webb Company) to the south, Church Street to the east and Tarkiln Hill Place to the west. A private right-of-way (Dutton Street) bisects the Project site between Tarkiln Hill Place and Church Street. Figure 1 depicts the Project site location in relation to the existing roadway network. The Project site is currently occupied by the Regal House Classic Furniture store building and a single-family home (947 Church Street) with supporting parking areas, driveways and appurtenances. Both structures will be retained as a part of the Project.

Access to the Project site will be provided by way of three (3) driveways that will intersect the west side of Church Street approximately 180 feet south of Chaffee Street, the south side of Chaffee Street approximately 60 feet west of Church Street and the east side of Tarkiln Hill Place approximately 180 feet south of Chaffee Street, respectively. In addition, a curbside drop-off/pick-up area will be provided along the Project site frontage on Chaffee Street outside of the traveled way and a paved access to an on-site dumpster will be provided that will intersect the south side of Chaffee Street approximately 30 feet east of Tarkiln Hill Place.





Site Location Map

Off-street parking will be provided for 131 vehicles, including five (5) handicapped accessible spaces, which exceeds the parking requirements of Section 3100, *Parking and Loading*, of Chapter 9, *Comprehensive Zoning*, of the City of New Bedford Code of Ordinances, for the appropriate use.³

STUDY METHODOLOGY

This study was prepared in consultation with the City of New Bedford and the Massachusetts Department of Transportation (MassDOT); was performed in accordance with MassDOT's *Transportation Impact Assessment (TIA) Guidelines* and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports; and was conducted in three distinct stages.

The first stage involved an assessment of existing conditions in the study area and included an inventory of roadway geometrics; pedestrian and bicycle facilities; on-street parking; public transportation services; observations of traffic flow; and collection of pedestrian, bicycle and vehicle counts.

In the second stage of the study, future traffic conditions were projected and analyzed. Specific travel demand forecasts for the Project were assessed along with future traffic demands due to expected traffic growth independent of the Project. A seven-year time horizon from the date of publication of this assessment was selected for analyses consistent with MassDOT's *Transportation Impact Assessment (TIA) Guidelines*. The traffic analysis conducted in stage two identifies existing or projected future roadway capacity, traffic safety, and site access issues.

The third stage of the study presents and evaluates measures to address traffic and safety issues, if any, identified in stage two of the study.

³Offices: General, Professional, Business, Banks, Medical Clinics and Laboratories, Radio and Television Stations; Office of Non-Profit Educational, Cultural, or Charitable Organizations are required to provide one (1) space per each 200 sq ft of gross floor area, but not less than two (2) spaces for each business unit intended to occupy the premises; after 10,000 sq ft of gross floor area, one space for every 1,000 sq ft of gross floor area is required.

EXISTING CONDITIONS

A comprehensive field inventory of existing conditions within the study area was conducted in December 2020. The field investigation consisted of an inventory of existing roadway geometrics; pedestrian and bicycle facilities; public transportation services; traffic volumes; and operating characteristics; as well as posted speed limits and land use information within the study area. The study area that was assessed for the Project consisted of Church Street, Chaffee Street and Tarkiln Hill Place, and the following specific intersections: Church Street at Chaffee Street and Tarkiln Hill Place at Chaffee Street.

The following describes the study area roadways and intersections.

Roadways

Church Street

- Two-lane urban collector roadway under City jurisdiction
- Traverses study area in a general north-south alignment between Route 140/Exit 5 and Coffin Avenue
- ➤ Provides two 15± foot wide travel lanes that are separated by a double-yellow centerline with 7-foot wide bicycle lanes provided along both sides of the roadway
- The posted speed limit is 30 miles per hour (mph)
- ➤ In general, sidewalks are not provided along Church Street. A short segment of sidewalk has been constructed along the Church Street frontage (west side) of 1239 Chaffee Street
- > Illumination is provided by way of street lights mounted on wood poles
- ➤ Land use within the study area consists of the Project site, residential and commercial properties, and areas of open wooded space

Chaffee Street

- > Two-lane local access roadway under City jurisdiction
- Traverses study area in a general east-west alignment between Tori Lyon Drive and Tarkiln Hill Place
- Provides an approximate 26 to 38-foot wide traveled way (paved area) within the study area with no marked centerline or shoulders

- ➤ A posted speed limit is not provided and, therefore, the statutory or "prima facie" speed limit is 30 mph⁴
- > Sidewalks are not provided within the study area
- > Illumination is provided by way of street lights mounted on wood poles
- ➤ Land use within the study area consists of the Project site, residential and commercial properties, and areas of open wooded space

Tarkiln Hill Place

- > Two-lane local access roadway under City jurisdiction
- > Traverses study area in a general north-south between Mate Drive and the driveway to 9 Tarkiln Hill Place (approximately 275 feet south of Chaffee Street)
- ➤ Provides an approximate 24 to 36-foot wide traveled way (paved area) within the study area with no marked centerline or shoulders
- > A posted speed limit is not provided and, therefore, the statutory or "prima facie" speed limit is 30 mph
- ➤ Sidewalks are provided along both sides north of Chaffee Street
- > Illumination is provided by way of street lights mounted on wood poles
- ➤ Land use within the study area consists of the Project site, residential and commercial properties, and areas of open wooded space

Intersections

Table 1 and Figure 2 summarize existing lane use, traffic control, and pedestrian and bicycle accommodations at the study area intersections as observed in December 2020.

Table 1 STUDY AREA INTERSECTION DESCRIPTION

Intersection	Traffic Control Type ^a	No. of Travel Lanes Provided	Shoulder Provided? (Yes/No/Width)	Pedestrian Accommodations? (Yes/No/Description)	Bicycle Accommodations? (Yes/No/Description)
Church St./ Chaffee St.	S	1 general purpose travel lane on all approaches	Yes, 7-foot bike lanes on Church St.	Yes, sidewalk segment along west side of Church St. north of intersection; bus stop located on the northeast corner	Yes, 7-foot bike lanes on Church St. and shared traveled-way on Chaffee St. ^b
Tarkiln Hill Pl./ Chaffee St.	S	1 general purpose travel lane on all approaches	No	Yes, both sides of Tarkiln Hill Pl. north of intersection	Yes, shared traveled-way

^aTS = Traffic Signal Control; S = STOP-sign control.

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⁴The statutory or "prima facie" speed is defined in M.G.L Chapter 90, Section 17, as the speed which would be deemed reasonable and proper to operate a motor vehicle.

Legend:

- (I) Unsignalized Intersection
- B Bus Stop
- Sidewalk
- xx'- Lane Use and Travel Lane Width
- Bicycle Lane

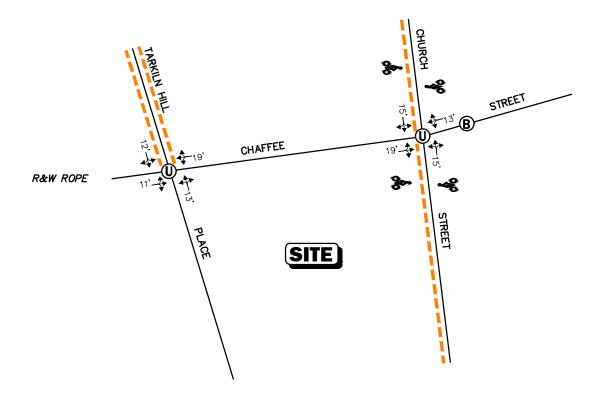




Figure 2

Existing Intersection Lane Use, Travel Lane Width and Pedestrian Facilities

TRAFFIC VOLUMES

In order to determine existing traffic-volume demands and flow patterns within the study area, automatic traffic recorder (ATR) counts, manual turning movement counts (TMCs) and vehicle classification counts were completed in December 2020. The ATR counts were conducted on December 1st and 2nd, 2020 (Tuesday through Wednesday, inclusive) on Church Street and Tarkiln Hill Place in the vicinity of the Project site in order to record weekday traffic conditions over an extended period, with weekday morning (7:00 to 9:00 AM) and evening (2:00 to 6:00 PM) peak period manual TMCs performed at the study intersections on December 1, 2020 (Tuesday). These time periods were selected for analysis purposes as they are representative of the peak-traffic-volume hours for both the Project and the adjacent roadway network.

Traffic-Volume Adjustments

In order to evaluate the potential for seasonal fluctuation of traffic volumes within the study area, traffic volume data from MassDOT Continuous Count Station No. 38 located on Interstate 195 (I-195) in Fairhaven were reviewed.⁵ Based on a review of this data it was determined that traffic volumes for the month of December are approximately 10.2 percent <u>below</u> average-month conditions. As such, the December traffic volumes were adjusted upward by 10.2 percent in order to be representative of average-month conditions.

In order to account for the impact on traffic volumes and trip patterns resulting from the "safer-at-home" order and the phased "Reopening Massachusetts" plan that was issued by the Governor on May 18, 2020, in response to the COVID-19 pandemic, the December 2020 traffic volumes that were collected as a part of this assessment were adjusted upward by an additional 13.4 percent based on a comparison of traffic volume data obtained from MassDOT Continuous Count Station No. 38.

The 2020 Existing traffic volumes are summarized in Table 2, with the weekday morning and evening peak-hour traffic volumes graphically depicted on Figure 3. Note that the peak-hour traffic volumes presented in Table 2 were obtained from the TMCs and are reflected on the aforementioned figure.

Table 2 2020 EXISTING TRAFFIC VOLUMES

Location/Peak Hour	AWT ^a	VPH ^b	K Factor ^c	Directional Distribution ^d
Church Street, south of Chaffee Street:	9,580			
Weekday Morning (7:00 – 8:00 AM)		571	6.0	64.1% SB
Weekday Evening (3:00 – 4:00 PM)		987	10.3	52.0% NB
Tarkiln Hill Place, south of Chaffee Street:	140			
Weekday Morning (7:00 – 8:00 AM)		18	12.9	94.4% SB
Weekday Evening (3:00 – 4:00 PM)		19	13.6	94.7% NB

^aAverage weekday traffic in vehicles per day.

^cPercent of daily traffic occurring during the peak hour.

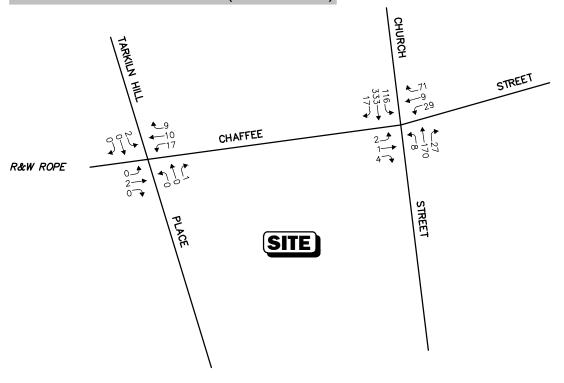
NB = northbound; SB = southbound.

^bVehicles per hour.

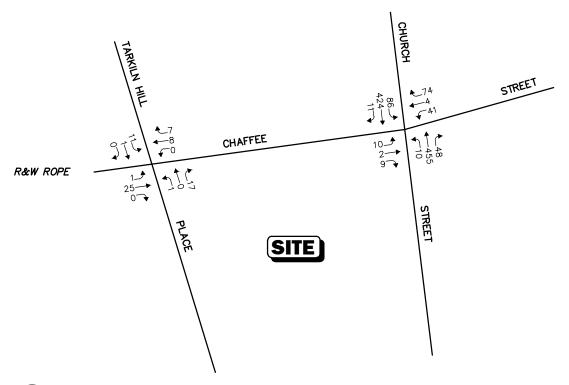
^dPercent traveling in peak direction.

⁵MassDOT Traffic Volumes for the Commonwealth of Massachusetts; 2020.

WEEKDAY MORNING PEAK HOUR (7:00 - 8:00 AM)



WEEKDAY EVENING PEAK HOUR (3:00 - 4:00 PM)





Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale Figure 3



2020 Existing Peak Hour Traffic Volumes As can be seen in Table 2, Church Street in the vicinity of the Project site was found to accommodate approximately 9,580 vehicles on an average weekday (two-way, 24-hour volume), with approximately 571 vehicles per hour (vph) during the weekday morning peak-hour and 987 vph during the weekday evening peak-hour.

Tarkiln Hill Place in the vicinity of the Project site was found to accommodate approximately 140 vehicles on an average weekday, with approximately 18 vph during the weekday morning peak-hour and 19 vph during the weekday evening peak-hour.

PEDESTRIAN AND BICYCLE FACILITIES

A comprehensive field inventory of pedestrian and bicycle facilities within the study area was undertaken in December 2020. The field inventory consisted of a review of the location of sidewalks and pedestrian crossing locations along the study roadways and at the study area intersections. As detailed on Figure 2, sidewalks are provided along both sides Tarkiln Hill Place north of Chaffee Street and the Church Street frontage (west side) of 1239 Chaffee Street.

Marked bicycle lanes are provided along both sides of Church Street within the study area, with both Chaffee Street and Tarkiln Hill Place providing sufficient width to accommodate bicycle travel in a shared traveled-way configuration (i.e., bicyclists and motor vehicles sharing the traveled-way).⁶

PUBLIC TRANSPORTATION

Public transportation services are provided within the study area by the Southeastern Regional Transit Authority (STRA) by way of the Route 4, *Ashley Boulevard*, and the North End Shuttle. The Route 4 bus provides service along Ashley Boulevard, Chaffee Street and Church Street, with a stop located at the Church Street/Chaffee Street intersection, opposite the Project site. The North End Shuttle provides service along Phillips Road, Church Street and Acushnet Avenue, with the closest regular stop located approximately 0.4-miles north of the Project site at Amanda Avenue (Dottin Apartments). In addition to regular stops, SRTA buses operate in a passenger demand mode ("flag stop") and will stop anywhere along the regular service route where it is safe to pick-up or discharge a passenger when requested.

In addition to fixed-route bus services, the SRTA provides Dial-a-Ride paratransit services to eligible persons that cannot use fixed-route transit all or some of the time due to a physical, cognitive or mental disability in compliance with the ADA, and provides services for seniors through the New Bedford Council on Aging (COA).

The public transportation schedules and fare information are provided in the Appendix.

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⁶A minimum combined travel lane and paved shoulder width of 14-feet is required to support bicycle travel in a shared traveled-way condition.

SPOT SPEED MEASUREMENTS

Vehicle travel speed measurements were performed on Church Street and Tarkiln Hill Place in the vicinity of the Project site in conjunction with the ATR counts. Table 3 summarizes the vehicle travel speed measurements.

Table 3
VEHICLE TRAVEL SPEED MEASUREMENTS

	Church	n Street	Tarkiln Hill Place			
	Northbound	Southbound	Northbound	Southbound		
Mean Travel Speed (mph)	32	29	17	18		
85 th Percentile Speed (mph)	38	34	22	23		
Posted or Statutory Speed Limit (mph)	30	30	30 ^a	30 ^a		

^aStatutory speed limit. mph = miles per hour.

As can be seen in Table 3, the mean vehicle travel speed along Church Street in the vicinity of the Project site was found to be 32 mph northbound and 29 mph southbound. The measured 85th percentile vehicle travel speed, or the speed at which 85 percent of the observed vehicles traveled at or below, was found to be 38 mph northbound and 34 mph southbound, which is 4 to 8 mph above the posted speed limit (35 mph). The 85th percentile speed is used as the basis of engineering design and in the evaluation of sight distances, and is often used in establishing posted speed limits.

The mean vehicle travel speed along Tarkiln Hill Place in the vicinity of the Project site was found to be 17 mph northbound and 18 mph southbound, with the measured 85th percentile vehicle travel speed found to be 22 mph northbound and 23 mph southbound, which is 7 to 8 mph below the statutory speed limit (30 mph) and is indicative of the relatively short segment of roadway along which the measurements were performed.

MOTOR VEHICLE CRASH DATA

Motor vehicle crash information for the study area intersections was provided by the MassDOT Highway Division Safety Management/Traffic Operations Unit for the most recent five-year period available (2013 through 2017, inclusive) in order to examine motor vehicle crash trends occurring within the study area. The data is summarized by intersection, type, severity, roadway and weather conditions, and day of occurrence, and presented in Table 4.

Table 4 MOTOR VEHICLE CRASH DATA SUMMARY^a

	Church Street/ Chaffee Street	Chaffee Street/ Tarkiln Hill Place
Traffic Control Type:b	U	U
Year:		
2013	2	0
2014	1	0
2015	1	0
2016	2	0
<u>2017</u> Total	$\frac{1}{7}$	$\frac{0}{0}$
Average	1.40	0.00
Rate ^c	0.29	0.00
MassDOT Crash Rate:d	0.57/0.57	0.57/0.57
Significant?e	No	No
Type:		
Angle	4	0
Rear-End	2	0
Head-On	0	0
Sideswipe	0	0
Fixed Object	1	0
Pedestrian/Bicycle Unknown/Other	0	0
Total	<u>0</u> 7	$\frac{\sigma}{0}$
Conditions:		
Clear	4	0
Cloudy	0	0
Rain	3	0
Snow/Ice	<u>0</u>	<u>0</u>
Total	7	0
Lighting:	_	
Daylight	5	0
Dawn/Dusk	1	0
Dark (Road Lit)	1	0
Dark (Road Unlit) Total	$\frac{0}{7}$	$\frac{0}{0}$
Day of Week:		
Monday through Friday	5	0
Saturday	1	0
Sunday	1	0
Total	- 7	$\frac{\overline{o}}{0}$
Severity:		
Property Damage Only	1	0
Personal Injury	6	0
Fatality	0	0
Total	7	0

aSource: MassDOT Safety Management/Traffic Operations Unit records, 2013 through 2017.
bTraffic Control Type: U = unsignalized; TS = traffic signal.
cCrash rate per million vehicles entering the intersection.
dStatewide/District crash rate.

As can be seen in Table 4, the Church Street/Chaffee Street intersection was found to have experienced a total of seven (7) reported motor vehicle crashes over the five-year review period, or an average of 1.4 crashes per year, the majority of which occurred on a weekday; under clear weather conditions; during daylight; and were reported as angle type collisions that resulted in personal injury. The intersection was found to have a motor vehicle crash rate that was <u>below</u> the MassDOT Statewide and District 5 average crash rates for an unsignalized intersection. No (0) motor vehicle crashes were reported to have occurred at the Tarkiln Hill Place/Chaffee Street intersection over the five-year review period.

A review of the MassDOT statewide High Crash Location List indicated that there are no locations within the study area that are included on MassDOT's Highway Safety Improvement Program (HSIP) listing as a high crash location. In addition, no fatal motor vehicle crashes were reported to have occurred at the study area intersections over the five-year review period.

The detailed MassDOT Crash Rate Worksheets are provided in the Appendix.

Traffic volumes in the study area were projected to the year 2028, which reflects a seven-year planning horizon from the date of publication of this assessment, consistent with MassDOT's *Transportation Impact Assessment (TIA) Guidelines*. Independent of the Project, traffic volumes on the roadway network in the year 2028 under No-Build conditions include all existing traffic and new traffic resulting from background traffic growth. Anticipated Project-generated traffic volumes superimposed upon the 2028 No-Build traffic volumes reflect 2028 Build traffic volume conditions with the Project.

FUTURE TRAFFIC GROWTH

Future traffic growth is a function of the expected land development in the immediate area and the surrounding region. Several methods can be used to estimate this growth. A procedure frequently employed estimates an annual percentage increase in traffic growth and applies that percentage to all traffic volumes under study. The drawback to such a procedure is that some turning volumes may actually grow at either a higher or a lower rate at particular intersections.

An alternative procedure identifies the location and type of planned development, estimates the traffic to be generated, and assigns it to the area roadway network. This procedure produces a more realistic estimate of growth for local traffic; however, potential population growth and development external to the study area would not be accounted for in the resulting traffic projections.

To provide a conservative analysis framework, both procedures were used, the salient components of which are described below.

Specific Development by Others

The City of New Bedford Planning Department was consulted in order to determine if there were any projects planned within the study area that would have an impact on future traffic volumes at the study intersections. Based on this consultation, the following projects were identified for inclusion in this assessment:

➤ U-Haul Moving & Storage, 429 Church Street, New Bedford, Massachusetts. This project entails the construction of a 84,785 sf U-Haul Moving & Storage facility to be located at 429 Church Street.

> Alma Del Mar II Charter School, 739 Church Street, New Bedford, Massachusetts. This project entails the construction of a charter school to be located at 739 Church Street that will consist of two (2) buildings totaling 57,127 sf of space that will accommodate 600 students.

Traffic volumes associated with the aforementioned specific development projects by others were obtained from the respective traffic study or using trip-generation information available from the Institute of Transportation Engineers (ITE)⁷ for the appropriate land uses, and were assigned onto the study area roadway network based on existing traffic patterns where no other information was available. No other developments were identified at this time that are expected to result in an increase in traffic within the study area beyond the general background traffic growth rate.

General Background Traffic Growth

Traffic-volume data obtained for MassDOT Continuous Count Station No. 38 (I-195 in Fairhaven) was reviewed in order to determine general traffic growth trends in the area. Based on a review of this data, a 1.0 percent per year compounded annual background traffic growth rate was used in order to account for future traffic growth and presently unforeseen development within the study

Roadway Improvement Projects

The City of New Bedford and MassDOT were contacted in order to determine if there were any planned future roadway improvement projects expected to be complete by 2028 within the study Based on these discussions, no roadway improvement projects aside from routine maintenance activities were identified to be planned within the study area at this time.

No-Build Traffic Volumes

The 2028 No-Build condition peak-hour traffic-volumes were developed by applying the 1.0 percent per year compounded annual background traffic growth rate to the 2020 Existing peak-hour traffic volumes and then adding the traffic volumes associated with the identified specific development projects by others. The resulting 2028 No-Build weekday morning and evening peak-hour traffic volumes are shown on Figure 4.

PROJECT-GENERATED TRAFFIC

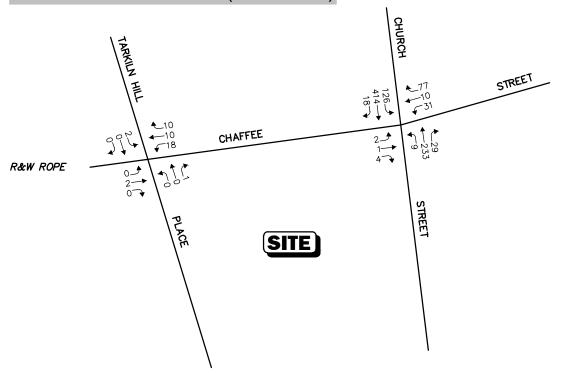
Design year (2028 Build) traffic volumes for the study area roadways were determined by estimating Project-generated traffic volumes and assigning those volumes on the study roadways. The following sections describe the methodology used to develop the anticipated traffic characteristics of the Project.

The Project will entail the renovation of the Regal House Classic Furniture store building, which encompasses 69,283± sf, to accommodate a counseling and treatment center for Child and Family Services. A maximum of 85 employees are expected at the center. In order to develop the traffic characteristics of the Project, trip-generation statistics published by the ITE⁸ for a similar land use as that proposed were used. ITE Land Use Code (LUC) 720, Medical-Dental Office Building, was

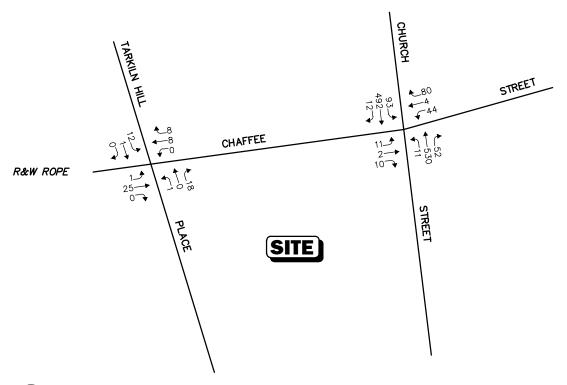
⁷Ibid 1.

⁸Ibid 1.

WEEKDAY MORNING PEAK HOUR (7:00 - 8:00 AM)



WEEKDAY EVENING PEAK HOUR (3:00 - 4:00 PM)





Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale

Figure 4



2028 No-Build Peak Hour Traffic Volumes used to develop the traffic characteristics of the Project, the results of which are summarized in Table 5, with detailed trip calculations provided in the Appendix.

Table 5
TRIP GENERATION SUMMARY

	Vehicle Trips ^a								
Time Period	Entering	Exiting	Total						
Average Weekday:	319	319	638						
Weekday Morning Peak-Hour:	39	11	50						
Weekday Evening Peak-Hour:	29	55	84						

^aBased on ITE LUC 720, Medical-Dental Office; 85 employees.

Project-Generated Traffic Volume Summary

As can be seen in Table 5, the Project is expected to generate approximately 638 vehicle trips on an average weekday (two-way, 24-hour volume, or 319 vehicles entering and 319 exiting), with 50 vehicle trips (39 vehicles entering and 11 exiting) expected during the weekday morning peak-hour and 84 vehicle trips (29 vehicles entering and 55 exiting) expected during the weekday evening peak-hour.

TRIP DISTRIBUTION AND ASSIGNMENT

The directional distribution of generated trips to and from the Project site was determined based on a review of existing traffic patterns within the study area. The general trip distribution for the Project is graphically depicted on Figure 5. The additional traffic expected to be generated by the Project was assigned on the study area roadway network as shown on Figure 6 for the weekday morning and evening peak hours.

FUTURE TRAFFIC VOLUMES - BUILD CONDITION

The 2028 Build condition traffic volumes consist of the 2028 No-Build traffic volumes with the additional traffic expected to be generated by the Project added to them. The 2028 Build weekday morning and evening peak-hour traffic-volumes are graphically depicted on Figure 7.

A summary of peak-hour projected traffic-volume changes outside of the study area that is the subject of this assessment is shown in Table 6. These changes are a result of the construction of the Project.

Legend:

XX Entering Trips
(XX) Exiting Trips

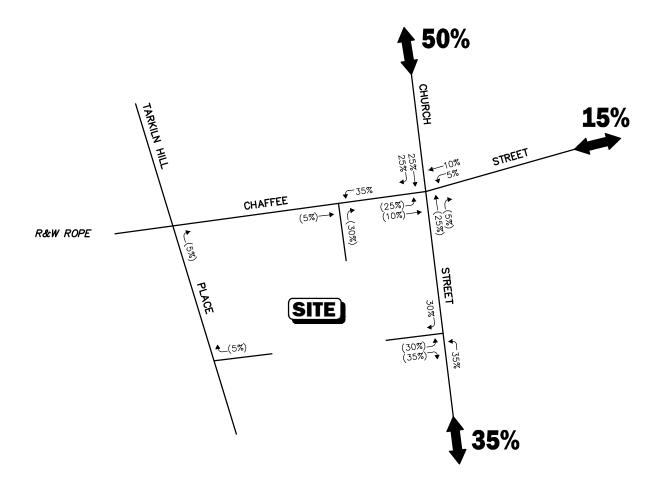
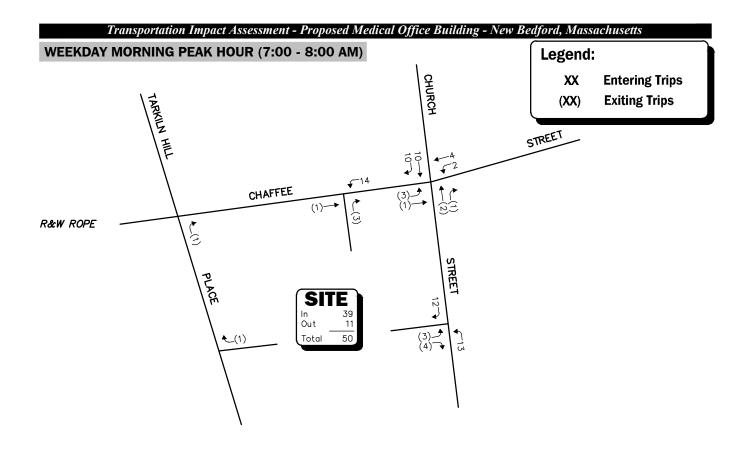


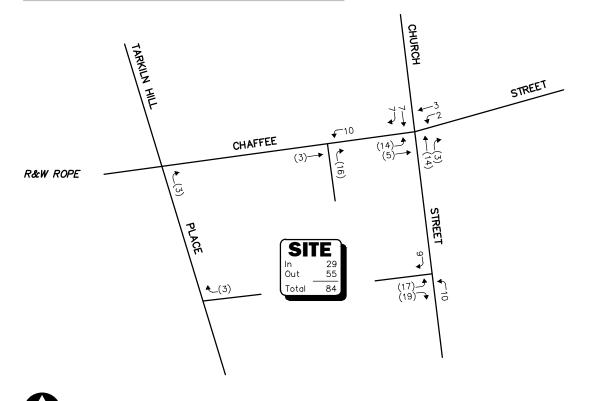


Figure 5

Trip Distribution Map



WEEKDAY EVENING PEAK HOUR (3:00 - 4:00 PM)



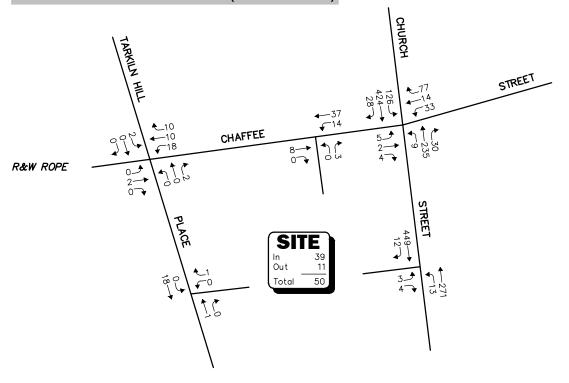


Project-Generated Peak Hour Traffic Volumes

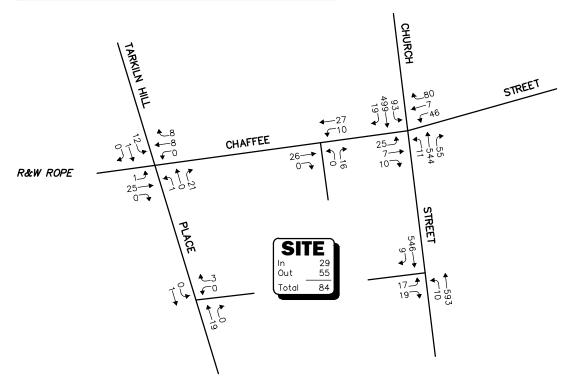
Figure 6

Not To Scale

WEEKDAY MORNING PEAK HOUR (7:00 - 8:00 AM)



WEEKDAY EVENING PEAK HOUR (3:00 - 4:00 PM)





Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale

Figure 7



2028 Build Peak Hour Traffic Volumes

Table 6
PEAK-HOUR TRAFFIC-VOLUME INCREASES

Location/Peak Hour	2020 Existing	2028 No-Build	2028 Build	Traffic Volume Increase Over No-Build	Percent Increase Over No-Build
Church Street, north of Chaffee Street:					
Weekday Morning	709	870	895	25	2.9
Weekday Evening	1,060	1,218	1,260	42	3.4
Church Street, south of Chaffee Street:					
Weekday Morning	571	720	737	17	2.4
Weekday Evening	978	1,139	1,168	29	2.5
Chaffee Street, east of Church Street:					
Weekday Morning	523	274	282	8	2.9
Weekday Evening	255	275	288	13	4.7

As shown in Table 6, Project-related traffic-volume increases outside of the study area relative to 2028 No-Build conditions are anticipated to range from 2.4 to 4.7 percent during the peak periods, with vehicle increases shown to range from 8 to 42 vehicles. When distributed over the peak-hour, the predicted traffic volume increases would not result in a significant impact (increase) on motorist delays or vehicle queuing outside of the immediate study area that is the subject of this assessment.

TRAFFIC OPERATIONS ANALYSIS

Measuring existing and future traffic volumes quantifies traffic flow within the study area. To assess quality of flow, roadway capacity and vehicle queue analyses were conducted under Existing, No-Build and Build traffic volume conditions. Capacity analyses provide an indication of how well the roadway facilities serve the traffic demands placed upon them, with vehicle queue analyses providing a secondary measure of the operational characteristics of an intersection or section of roadway under study.

METHODOLOGY

Levels of Service

A primary result of capacity analyses is the assignment of level of service to traffic facilities under various traffic-flow conditions. The concept of level of service is defined as a qualitative measure describing operational conditions within a traffic stream and their perception by motorists and/or passengers. A level-of-service definition provides an index to quality of traffic flow in terms of such factors as speed, travel time, freedom to maneuver, traffic interruptions, comfort, convenience, and safety.

Six levels of service are defined for each type of facility. They are given letter designations from A to F, with level-of-service (LOS) A representing the best operating conditions and LOS F representing congested or constrained operating conditions.

Since the level of service of a traffic facility is a function of the traffic flows placed upon it, such a facility may operate at a wide range of levels of service, depending on the time of day, day of week, or period of year.

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⁹The capacity analysis methodology is based on the concepts and procedures presented in the *Highway Capacity Manual*; Transportation Research Board; Washington, DC; 2010.

Unsignalized Intersections

The six levels of service for unsignalized intersections may be described as follows:

- LOS A represents a condition with little or no control delay to minor street traffic.
- LOS B represents a condition with short control delays to minor street traffic.
- LOS C represents a condition with average control delays to minor street traffic.
- LOS D represents a condition with long control delays to minor street traffic.
- LOS E represents operating conditions at or near capacity level, with very long control delays to minor street traffic.
- LOS F represents a condition where minor street demand volume exceeds capacity of an approach lane, with extreme control delays resulting.

The levels of service of unsignalized intersections are determined by application of a procedure described in the 2010 *Highway Capacity Manual*. Level of service is measured in terms of average control delay. Mathematically, control delay is a function of the capacity and degree of saturation of the lane group and/or approach under study and is a quantification of motorist delay associated with traffic control devices such as traffic signals and STOP signs. Control delay includes the effects of initial deceleration delay approaching a STOP sign, stopped delay, queue move-up time, and final acceleration delay from a stopped condition. Definitions for level of service at unsignalized intersections are also given in the 2010 *Highway Capacity Manual*. Table 7 summarizes the relationship between level of service and average control delay for two-way stop controlled and all-way stop controlled intersections.

Table 7
LEVEL-OF-SERVICE CRITERIA FOR UNSIGNALIZED INTERSECTIONS^a

Level-Of-Service by V	Level-Of-Service by Volume-to-Capacity Ratio					
v/c ≤ 1.0	v/c > 1.0	(Seconds Per Vehicle)				
A	F	≤10.0				
В	F	10.1 to 15.0				
C	F	15.1 to 25.0				
D	F	25.1 to 35.0				
E	F	35.1 to 50.0				
F	F	>50.0				

^aSource: *Highway Capacity Manual*; Transportation Research Board; Washington, DC; 2010; page 19-2.

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¹⁰Highway Capacity Manual; Transportation Research Board; Washington, DC; 2010.

Vehicle Queue Analysis

Vehicle queue analyses are a direct measurement of an intersection's ability to process vehicles under various traffic control and volume scenarios and lane use arrangements. The vehicle queue analysis was performed using the Synchro® intersection capacity analysis software which is based upon the methodology and procedures presented in the 2010 *Highway Capacity Manual*. The Synchro® vehicle queue analysis methodology is a simulation based model which reports the number of vehicles that experience a delay of six seconds or more at an intersection. For signalized intersections, Synchro® reports both the average (50th percentile) the 95th percentile vehicle queue. For unsignalized intersections, Synchro® reports the 95th percentile vehicle queue. Vehicle queue lengths are a function of the capacity of the movement under study and the volume of traffic being processed by the intersection during the analysis period. The 95th percentile vehicle queue is the vehicle queue length that will be exceeded only 5 percent of the time, or approximately three minutes out of sixty minutes during the peak one hour of the day (during the remaining fifty-seven minutes, the vehicle queue length will be less than the 95th percentile queue length).

ANALYSIS RESULTS

Level-of-service and vehicle queue analyses were conducted for 2020 Existing, 2028 No-Build and 2028 Build conditions for the intersections within the study area. The results of the intersection capacity and vehicle queue analyses are summarized in Table 8, with the detailed analysis results presented in the Appendix.

The following is a summary of the level-of-service and vehicle queue analyses for the intersections within the study area. For context, we note that an LOS of "D" or better is generally defined as "acceptable" operating conditions. Project-related impacts at the study area intersections were identified as follows:

Church Street at Chaffee Street – During the weekday morning peak-hour, operating conditions for the Chaffee Street east and westbound approaches to Church Street were shown to degrade as a result of the addition of Project-related traffic, with the eastbound approach degrading from LOS D to LOS E as a result of an increase in average motorist delay of approximately 11.5 seconds and the westbound approach degrading from LOS E to LOS F as a result of an increase in average motorist delay of approximately 15.2 seconds. Operating conditions for both Chaffee Street approaches remained at LOS F during the weekday evening peak-hour (no change over Existing or No-Build conditions). Vehicle queues at the intersection are predicted to increase by up to four (4) vehicles as a result of the Project.

Tarklin Hill Place at Chaffee Street – All movements at this intersection were shown to operate at LOS A under all analysis conditions with negligible vehicle queuing.

Church Street at the Project Site Driveway – All movements exiting the Project site are predicted to operate at LOS B during the weekday morning peak-hour and at LOS C during the weekday evening peak-hour with residual vehicle queues of up to one (1) vehicle which can be contained within the Project site without impeding the movement of vehicles, pedestrians or bicyclists along Church Street. All movements along Church Street approaching the Project site driveway are expected to operate at LOS A with negligible vehicle queuing.

Chaffee Street and Tarklin Hill Place at the Project Site Driveways – All movements are predicted to operate at LOS A during the peak-hours with negligible vehicle queuing.

Table 8 UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

		2028 No-Build				2028 Build						
				Queued				Queue				Queue
Unsignalized Intersection/Peak-hour/Movement	Demanda	Delay ^b	LOSc	95 th	Demand	Delay	LOS	95 th	Demand	Delay	LOS	95 th
Church Street at Chaffee Street												
Weekday Morning:												
Chaffee Street EB LT/TH/RT	7	19.2	C	0	7	28.0	D	0	11	39.5	Е	1
Chaffee Street WB LT/TH/RT	109	23.2	C	3	118	43.3	Е	4	124	>50.0	F	6
Church Street NB LT/TH/RT	205	0.3	A	0	271	0.3	A	0	274	0.3	A	0
Church Street SB LT/TH/RT	466	2.0	Α	1	558	2.0	Α	1	578	2.0	A	1
Weekday Evening:												
Chaffee Street EB LT/TH/RT	21	>50.0	F	2	23	>50.0	F	3	42	>50.0	F	7
Chaffee Street WB LT/TH/RT	119	>50.0	F	7	128	>50.0	F	12	133	>50.0	F	14
Church Street NB LT/TH/RT	513	0.2	Α	0	593	0.2	Α	0	607	0.2	A	0
Church Street SB LT/TH/RT	521	1.6	A	1	597	1.6	A	1	611	1.6	A	1
Tarkiln Hill Place at Chaffee Street												
Weekday Morning:												
Private Driveway EB LT/TH/RT	2	0.0	A	0	2	0.0	Α	0	2	0.0	A	0
Chaffee Street WB LT/TH/RT	36	0.0	A	0	38	0.0	A	0	38	0.0	A	0
Tarkiln Hill Place NB LT/TH/RT	1	0.0	Α	0	1	0.0	Α	0	2	0.0	Α	0
Tarkiln Hill Place SB LT/TH/RT	2	7.2	A	0	2	7.2	Α	0	2	7.2	A	0
Weekday Evening:												
Private Driveway EB LT/TH/RT	26	9.7	A	0	26	9.8	Α	0	26	9.8	A	0
Chaffee Street EB LT/TH/RT	15	9.1	A	0	16	9.1	A	0	16	9.1	A	0
Tarkiln Hill Place NB LT/TH/RT	18	0.4	A	0	19	0.4	Α	0	22	0.4	A	0
Tarkiln Hill Place SB LT/TH/RT	12	6.7	A	0	13	6.7	A	0	13	6.7	A	0

See notes at end of table.

Table 8 (Continued) UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

		2020 Ex	isting		2028 No-Build				2028 Build			
Unsignalized Intersection/Peak-hour/Movement	Demand ^a	Delay ^b	LOSc	Queue ^d 95 th	Demand	Delay	LOS	Queue 95 th	Demand	Delay	LOS	Queue 95 th
Church Street at the Project Site Driveway												
Weekday Morning:												
Project Site Driveway EB LT/RT									7	13.2	В	0
Church Street NB LT/TH									284	0.4	A	0
Church Street SB TH/RT									461	0.0	A	0
Weekday Evening:												
Project Site Driveway EB LT/RT									36	20.2	C	1
Church Street NB LT/TH									603	0.1	A	0
Church Street SB TH/RT									555	0.0	A	0
Chaffee Street at the Project Site Driveway												
Weekday Morning:												
Chaffee Street EB TH/RT									8	0.0	A	0
Chaffee Street WB LT/TH									51	0.2	A	0
Project Site Driveway NB LT/RT									3	8.4	Α	0
Weekday Evening:												
Chaffee Street EB TH/RT									26	0.0	A	0
Chaffee Street WB LT/TH									37	2.0	A	0
Project Site Driveway NB LT/RT									16	8.5	A	0
Tarkiln Hill Place at the Project Site Driveway												
Weekday Morning:												
Project Site Driveway WB LT/RT									1	8.3	A	0
Tarkiln Hill Place NB TH/RT									1	0.0	A	0
Tarkiln Hill Place SB LT/TH									18	0.0	A	0
Weekday Evening:												
Project Site Driveway WB LT/RT									3	8.4	A	0
Tarkiln Hill Place NB TH/RT									19	0.0	A	0
Tarkiln Hill Place SB LT/TH									1	0.0	A	0

^aDemand in vehicles per hour.

^bAverage control delay per vehicle (in seconds). ^cLevel-of-Service.

^dQueue length in vehicle.

NB = northbound; SB = southbound; EB = eastbound; WB = westbound; LT = left-turning movements; TH = through movements; RT = right-turning movements

SIGHT DISTANCE EVALUATION

Sight distance measurements were performed at the Project site driveway intersections with Church Street, Chaffee Street and Tarklin Hill Place in accordance with American Association of State Highway and Transportation Officials (AASHTO)¹¹ requirements. Both stopping sight distance (SSD) and intersection sight distance (ISD) measurements were performed. In brief, SSD is the distance required by a vehicle traveling at the design speed of a roadway, on wet pavement, to stop prior to striking an object in its travel path. ISD or corner sight distance (CSD) is the sight distance required by a driver entering or crossing an intersecting roadway to perceive an on-coming vehicle and safely complete a turning or crossing maneuver with on-coming traffic. In accordance with AASHTO standards, if the measured ISD is at least equal to the required SSD value for the appropriate design speed, the intersection can operate in a safe manner. Table 9 presents the measured SSD and ISD at the subject intersections.

¹¹A Policy on Geometric Design of Highway and Streets, 7th Edition; American Association of State Highway and Transportation Officials (AASHTO); Washington D.C.; 2018.

Table 9 SIGHT DISTANCE MEASUREMENTS^a

Intersection/Sight Distance Measurement	Feet		
	Required Minimum (SSD)	Desirable (ISD) ^b	Measured
Church Street at the Project Site Driveway			
Stopping Sight Distance:			
Church Street approaching from the north	305		500+
Church Street approaching from the south	305		500+
Intersection Sight Distance:			
Looking to the north from the Site Driveway	305	385	500+
Looking to the south from the Site Driveway	305	445	500+
Chaffee Street at the Project Site Driveway Stopping Sight Distance:			
Chaffee Street approaching from the east	80		80°
Chaffee Street approaching from the west	200		371 ^d
Intersection Sight Distance:			
Looking to the east from Site Driveway	80	170	80°
Looking to the west from Site Driveway	200	290	371 ^d
Tarkiln Hill Place at the Site Driveway			
Stopping Sight Distance:			
Tarkiln Hill Place approaching from the north	155		500+
Tarkiln Hill Place approaching from the south	155		212e
Intersection Sight Distance:			
Looking to the north from the Site Driveway	155	280	500+
Looking to the south from the Site Driveway	155	240	212e

^aRecommended minimum values obtained from *A Policy on Geometric Design of Highways and Streets*, 7th Edition; American Association of State Highway and Transportation Officials (AASHTO); 2018; and based on a 40 mph approach speed along Church Street, a 25 mph approach speed along Tarkiln Hill Place and a 30 mph approach speed along Chaffee Street with the exception of approaching the Chaffee Street driveway from the east where a 15 mph approach speed was used to reflect the reduced speed of vehicles turning from Church Street onto Chaffee Street given the proximity of the driveway to Church Street.

As can be seen in Table 9, the available lines of sight at the Project site driveway intersections meet or exceed the recommended minimum sight distances to function in a safe manner (SSD) based on the appropriate approach speed along the intersecting roadway.

^bValues shown are the intersection sight distance for a vehicle turning right or left exiting a roadway under STOP control such that motorists approaching the intersection on the major street should not need to adjust their travel speed to less than 70 percent of their initial approach speed.

^cClear line of sight is provide to/from Church Street.

^dClear line of sight is provide to/from Tarkiln Hill Place.

^eClear line of sight is provided to/from the end of Tarkiln Hill Place.

CONCLUSIONS

VAI has conducted a TIA in order to determine the potential impacts on the transportation infrastructure associated with the proposed renovation of an existing building located at 965 Church Street in New Bedford, Massachusetts, and the construction of supporting parking on a portion of the abutting property at 947 Church Street to accommodate a counseling and treatment center for Child and Family Services. The following specific areas have been evaluated as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; under existing and future conditions, both with and without the Project. Based on this assessment, we have concluded the following with respect to the Project:

- 1. Using trip-generation statistics published by the ITE, ¹² the Project is expected to generate approximately 638 vehicle trips on an average weekday (two-way 24-hour volume), with 50 vehicle trips expected during the weekday morning peak-hour and 84 vehicle trips expected during the weekday evening peak-hour;
- 2. The Project will not result in a significant impact (increase) on motorist delays or vehicle queuing over Existing or anticipated future conditions without the Project (No-Build conditions), with the majority of the movements at the study intersections shown to operate at LOS of C or better under all analysis conditions, where an LOS of "D" or better is defined as "acceptable" traffic operations;
- 3. Independent of the Project, the Chaffee Street approaches to Church Street are currently or are predicted to operate at or over capacity (i.e., LOS "E" or LOS "F") during one or both peak hours, with Project-related impacts on these movements generally defined by an increase in vehicle queuing of up to four (4) vehicles;
- 4. All movements exiting the Project site are expected to operate at LOS C or better during the peak hours with minimal vehicle queuing predicted (up to one (1) vehicle);
- 5. No apparent safety deficiencies were noted with respect to the motor vehicle crash history at the study area intersections; and

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¹²Ibid 1.

6. Lines of sight to and from the Project site driveway intersections were found to meet or exceed the recommended minimum distances for safe operation based on the appropriate approach speed.

In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with implementation of the recommendations that follow.

RECOMMENDATIONS

A detailed transportation improvement program has been developed that is designed to provide safe and efficient access to the Project site and address any deficiencies identified at off-site locations evaluated in conjunction with this study. The following improvements have been recommended as a part of this evaluation and, where applicable, will be completed in conjunction with the Project subject to receipt of all necessary rights, permits, and approvals.

Project Access

Access to the Project site will be provided by way of three (3) driveways that will intersect the west side of Church Street approximately 180 feet south of Chaffee Street, the south side of Chaffee Street approximately 60 feet west of Church Street and the east side of Tarkiln Hill Place approximately 180 feet south of Chaffee Street, respectively. In addition, a curbside drop-off/pick-up area will be provided along the Project site frontage on Chaffee Street outside of the traveled way and a paved access to an on-site dumpster will be provided that will intersect the south side of Chaffee Street approximately 30 feet east of Tarkiln Hill Place. The following recommendations are offered with respect to the design and operation of the Project site access and internal circulation, many of which are reflected on the Site Plans:

- ➤ The Project site driveways and circulating drives within the Project site should be a minimum of 24-feet in width and designed to accommodate the turning and maneuvering requirements of the largest anticipated responding emergency vehicle.
- ➤ Vehicles exiting the Project site should be placed under STOP-sign control with a marked STOP-line provided
- ➤ All signs and pavement markings to be installed within the Project site should conform to the applicable standards of the *Manual on Uniform Traffic Control Devices* (MUTCD). ¹³
- A sidewalk has been provided within the Project site that extends between the building to Church Street, Chaffee Street and Tarkiln Hill Place.
- Americans with Disabilities Act (ADA) compliant wheelchair ramps should be provided at all pedestrian crossings that are constructed or modified as a part of the Project.
- > Signs and landscaping to be installed as a part of the Project within the intersection sight triangle areas of the Project site driveway intersections should be designed and maintained so as not to restrict lines of sight.

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¹³Ibid 2.

- Snow windrows within sight triangle areas of the Project site driveway intersections should be promptly removed where such accumulations would impede sight lines.
- ➤ Bicycle parking should be provided at an appropriate location within the Project site.

Off-Site

In an effort to address constraints or safety concerns identified as a part of this assessment, the following off-site roadway, intersection and traffic control improvements will be advanced as a part of the Project subject to receipt of all necessary rights, permits and approvals where necessary:

- ➤ Chaffee Street A double-yellow centerline should be installed along Chaffee Street between Church Street and Tarklin Hill Place.
- ➤ Church Street at Chafee Street A STOP-sign should be installed on the Chaffee Street eastbound approach and marked STOP-lines should be installed on both Chaffee Street approaches. In addition, a crosswalk with ADA compliant wheelchair ramps should be provided across the Chaffee Street west leg of the intersection.
- ➤ Tarkiln Hill Place at Chaffee Street A STOP-sign and marked STOP-line should be installed on the Chaffee Street approach. In addition, a crosswalk with ADA compliant wheelchair ramps should be provided across the Tarkiln Hill Place north leg of the intersection.

Transportation Demand Management

Public transportation services are provided within the study area by the STRA by way of the Route 4, *Ashley Boulevard*, and the North End Shuttle. The Route 4 bus provides service along Ashley Boulevard, Chaffee Street and Church Street, with a stop located at the Church Street/Chaffee Street intersection, opposite the Project site. The North End Shuttle provides service along Phillips Road, Church Street and Acushnet Avenue, with the closest regular stop located approximately 0.4-miles north of the Project site at Amanda Avenue (Dottin Apartments). In addition to regular stops, SRTA buses operate in a passenger demand mode ("flag stop") and will stop anywhere along the regular service route where it is safe to pick-up or discharge a passenger when requested.

In addition to fixed-route bus services, the SRTA provides Dial-a-Ride paratransit services to eligible persons that cannot use fixed-route transit all or some of the time due to a physical, cognitive or mental disability in compliance with the ADA, and provides services for seniors through the New Bedford Council on Aging (COA).

In an effort to encourage the use of alternative modes of transportation to single-occupant vehicles (SOVs), the following Transportation Demand Management (TDM) measures will be implemented as a part of the Project:

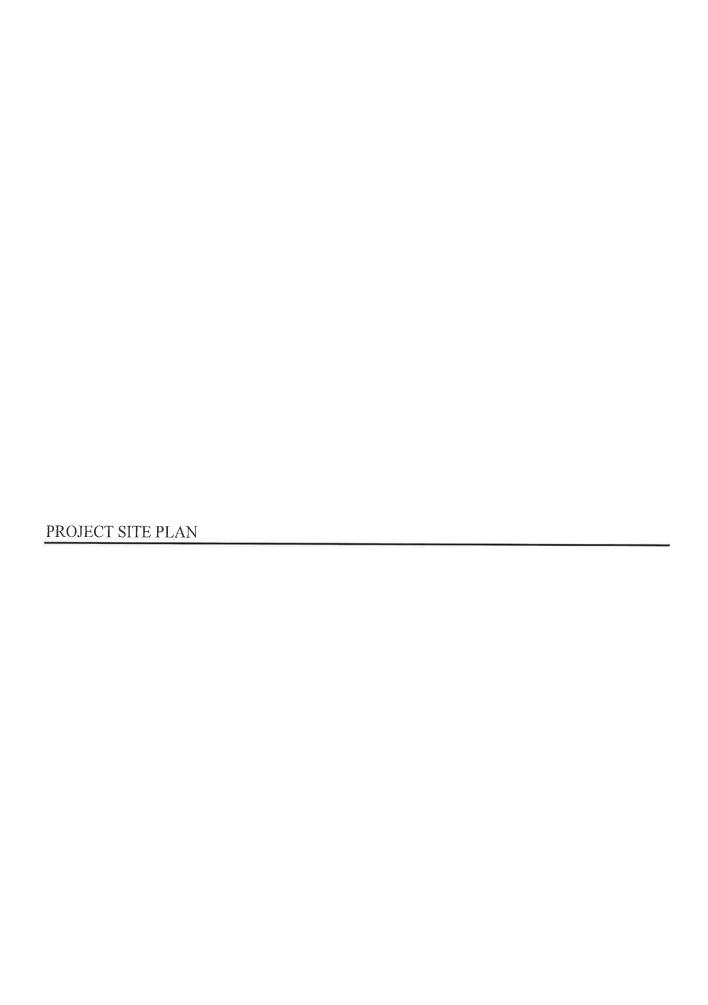
- ➤ A Transportation Coordinator will be designated for the Project to coordinate the elements of the TDM program;
- Information regarding public transportation services, maps, schedules and fare information will be posted in a central location and/or otherwise made available to employees and clients;

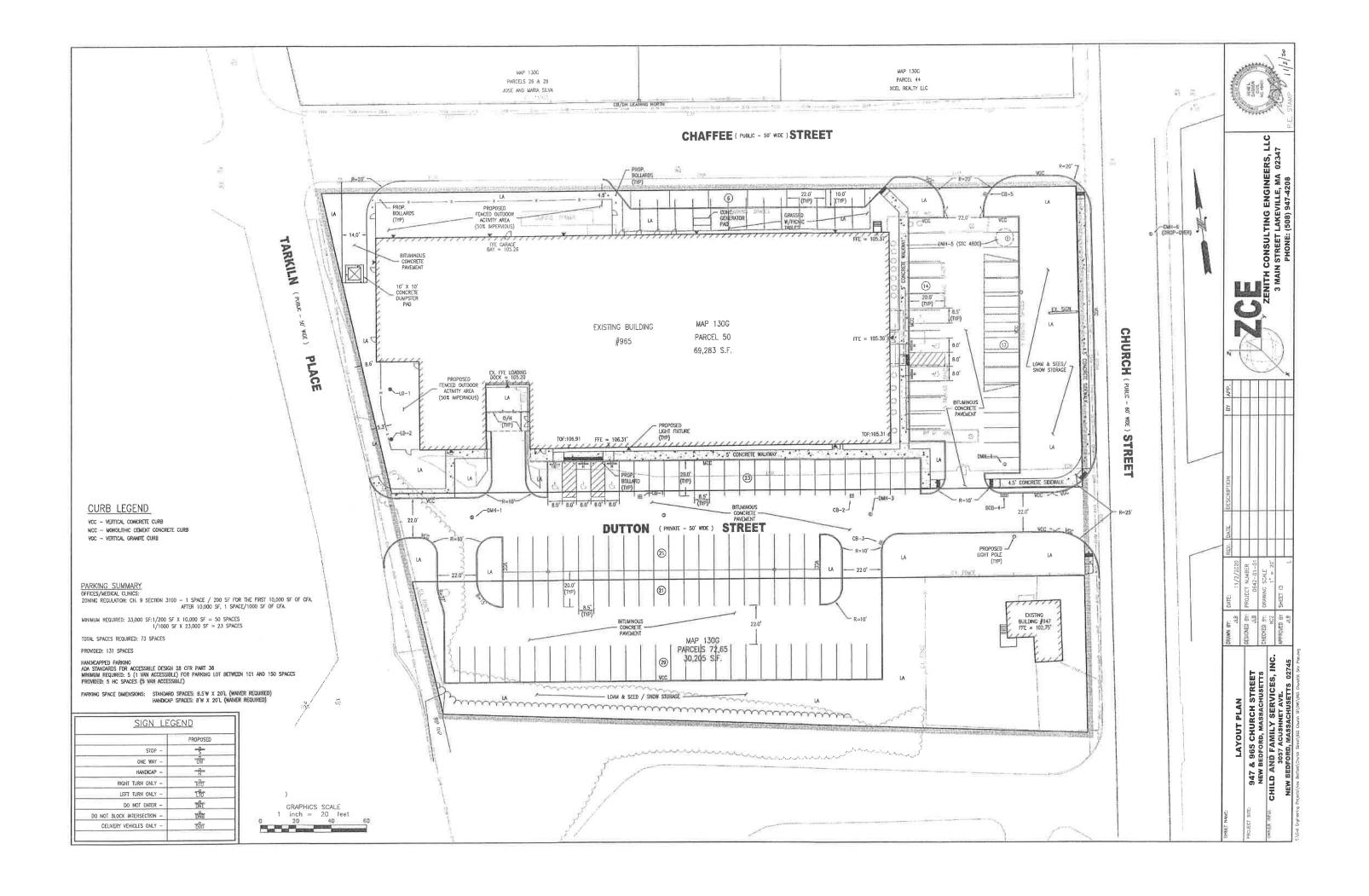
- ➤ A "welcome packet" will be provided to employees detailing available public transportation services, bicycle and walking alternatives, and commuter options available;
- > The Transportation Coordinator will facilitate a ride matching program for employees, including posting a sign-up sheet in a central location or via a link on the corporate web site and proving information in the new employee "welcome packet". In addition, preferential parking will be reserved or provided for carpools and vanpools.
- On-site amenities will be incorporated into the Project to discourage off-site trips, including providing a break-room equipped with a microwave and refrigerator; offering direct deposit of paychecks; allowing telecommuting or flexible work schedules; and other such measures to reduce overall traffic volumes and travel during peak traffic volume periods;
- > Pedestrian accommodations have been incorporated into the Project; and
- > Secure bicycle parking will be provided within the Project site.

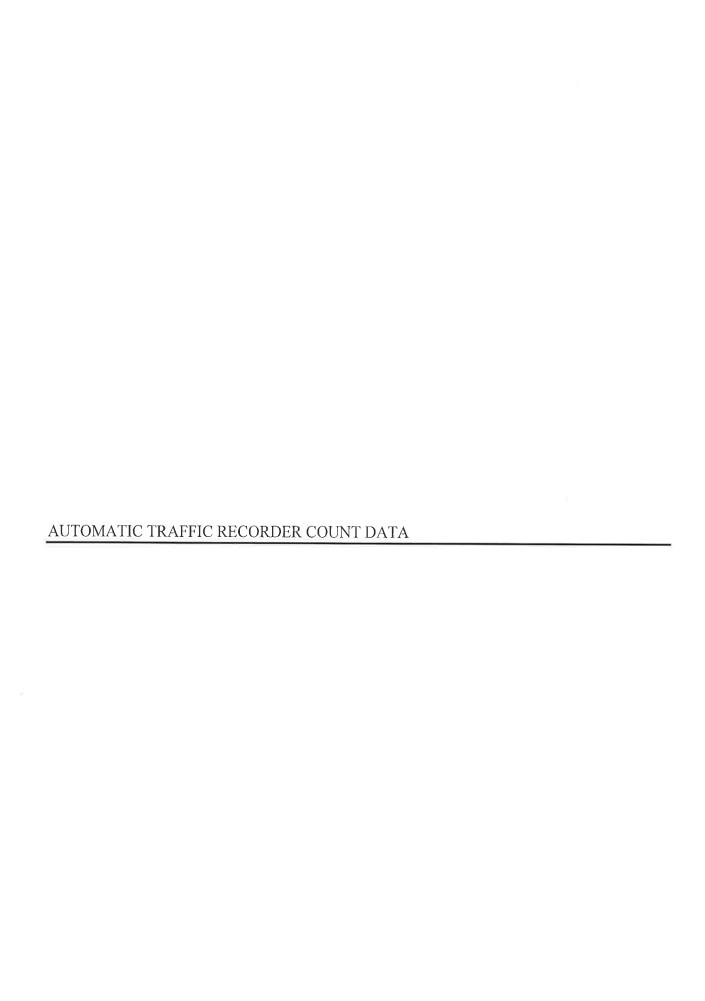
With implementation of the aforementioned recommendations, safe and efficient access will be provided to the Project site and the Project can be accommodated within the confines of the existing and improved transportation system.

APPENDIX

PROJECT SITE PLAN
AUTOMATIC TRAFFIC RECORDER COUNT DATA
MANUAL TURNING MOVEMENT COUNT DATA
SEASONAL ADJUSTMENT DATA
VEHICLE TRAVEL SPEED DATA
PUBLIC TRANSPORTATION INFORMATION
MASSDOT CRASH RATE WORKSHEETS AND HIGH CRASH LOCATION MAPPING
BACKGROUND DEVELOPMENT TRAFFIC-VOLUME NETWORKS
GENERAL BACKGROUND TRAFFIC GROWTH
TRIP-GENERATION CALCULATIONS
CAPACITY ANALYSIS WORKSHEETS







Location : Church Street Location : South of Chaffee Street City/State: New Bedford, MA

8807VL01

Start	12/1/2020		SB	Hour	Totals		√B	Hour	Totals	Combin	ed Totals
Time	Tue	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoo
12:00		11	73			1	68				
12:15		4	81			2 2	73				
12:30		1	77			2	69		1		
12:45		1	57	17	288	2	69	7	279	24	56
01:00		9	71		1	1	60				
01:15		3	50			3	78				
01:30		6	68		i	2	59		1		
01:45		4	57	22	246	2 2	73	8	270	30	51
02:00		1	61			1	60	Ū	2,0	00	0.
02:15		1	64		1	0	66				
02:30		1	83			1	97				
02:45		Ö	80	3	288	3	86	5	309	8	59
03:00		2	93	O	200	2	146	9	303	O	39
03:15		1	72			2	108				
03:30		o O	116		1	1	80		1		
03:45		3	97	6	378	1	81	6	415	40	70
04:00		1	102	0	3/6	9	88	О	415	12	79
04:00		1	90			9					
04:30		1	104			9	90				
04:45			93	44	000	14	80		202		
05:00		8		11	389	25	70	57	328	68	71
05:00		6	95			19	91		- 1		
05:15		9	69			13	70				
05:30		9	52			37	69				
05:45		17	53	41	269	34	55	103	285	144	55
06:00		17	53		l.	19	46				
06:15		28	44			27	62				
06:30		28	49			41	53				
06:45		45	44	118	190	56	40	143	201	261	39
07:00		103	40			38	40				
07:15		62	34			56	38				
07:30		63	30			42	42				
07:45		67	27	295	131	32	36	168	156	463	28
08:00		55	22			60	21				
08:15		53	19			48	27				
08:30		48	17		1	45	21				
08:45		59	21	215	79	42	18	195	87	410	160
09:00		48	15		1	52	21				
09:15		55	16		1	35	14		1		
09:30		52	16		1	60	14				
09:45		45	16	200	63	44	17	191	66	391	129
10:00		55	12			46	14		00	001	,,,
10:15		61	10			46	15				
10:30		59	9		i i	68	21		i i		
10:45		48	11	223	42	48	13	208	63	431	10
11:00		57	26	220	74	59	8	200	03	431	10
11:15		60	8			57	4				
11:30		67	7		1	57 53					
11:45		66	4	250	45	53	9	005	200	105	
Total				250	45	66	2404	235	22	485	67
		1401	2408			1326	2481			2727	4889
Percent		36.8%	63.2%			34.8%	65.2%			35.8%	64.2%

Location: Church Street Location: South of Chaffee Street City/State: New Bedford, MA

8807VL01

Start	12/2/2020	5	SB	Hour	Totals		VB	Hour	Totals	Combin	ed Totals
Time	Wed	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00		14	72			5	67				
12:15		3	79			0	76				
12:30		5	66			2	60		I.		
12:45		1	64	23	281	3	67	10	270	33	551
01:00		4	64			0	68				
01:15		1	78			0	65				
01:30		4	68			3	58				
01:45		2	68	11	278	2	75	5	266	16	544
02:00		7	46		1	2	78		i i		
02:15		1	60			3	69		1		
02:30		3	69			0	105		1		
02:45		2	92	13	267	2	88	7	340	20	607
03:00		3	115		1	1	135				
03:15		1	91		- 1	2	94				
03:30		2	111			4	84				
03:45		1	83	7	400	4	83	11	396	18	796
04:00		0	107			4	105				
04:15		3	85			11	81				
04:30		3	96			14	98				
04:45		4	90	10	378	24	75	53	359	63	737
05:00		8	76			19	88				
05:15		5	73			11	69				
05:30		7	66			32	69				
05:45		20	50	40	265	25	40	87	266	127	531
06:00		23	41			25	47		1		
06:15		25	50			23	51				
06:30		36	46			51	34				
06:45		51	38	135	175	43	36	142	168	277	343
07:00		97	44			41	41				
07:15		82	33			48	49		1		
07:30		53	31			36	35				
07:45		70	34	302	142	55	52	180	177	482	319
08:00		58	23			53	31		I		
08:15		56	26			54	26		- 1		
08:30		51	24	000		51	20				
08:45		41	15	206	88	43	19	201	96	407	184
09:00		44	21			49	8				
09:15		54	11			35	17				
09:30		60	12	000	50	60	16				
09:45		65	14	223	58	35	17	179	58	402	116
10:00		62	13			43	19				
10:15		70	8		1)	56	16				
10:30		47	5	0.45	00	50	22				
10:45 11:00		66		245	29	60	15	209	72	454	101
		59	35			66	9				
11:15		64	5			54	6				
11:30 11:45		70 81	3 2	074	4.5	60	10	1990541541	00		
				274	45	61	7	241	32	515	77
Total		1489	2406			1325	2500			2814	4906
Percent Grand		38.2%	61.8%			34.6%	65.4%			36.5%	63.5%
Total		2890	4814			2651	4981			5541	9795
			62 69/								
Percent		37.5%	62.5%			34.7%	65.3%			36.1%	63.9%

ADT

ADT 7,668

AADT 7,668

8807VL01

Accurate Counts 978-664-2565

Location : Church Street Location : South of Chaffee Street City/State: New Bedford, MA

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vved	SB	23	1	5	5 1	- (10	40	135	302	206	223	245	274	281	278	267	400	378	265	175	142	88	58	29	45	3895	7720	07:00	302	15:00	400	7720	
!	NB NB		00	٧.	<u> </u>	- [2/	103	143	168	195	191	208	235	279	270	309	415	328	285	201	156	87	99	63	22	3807		11:00	235	15:00	415	9	
en l ne	SB	17	22	c) ((7	Ξ:	41	118	295	215	200	223	250	288	246	288	378	389	269	190	131	79	63	42	45	3809	7616	00:20	295	16:00	389	7616	
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- Clari	Ime	12:00 AM	01:00	02:00	03:00	00.80	04.00	00:00	00:90	00:20	08:00	00:60	10:00	11:00	12:00 PM	01:00	02:00	03.00	04.00	02:00	00.90	00.70	08.00	00.60	10:00	11:00	Lane	Day	AM Peak	Vol.	PM Peak	Vol	Comb. Total	

Location: Tarkiln Hill Place Location: South of Chaffee Street City/State: New Bedford, MA

8807VL02

Start	12/1/2020		1B	Hour	Totals		SB	Hour	Totals	Combin	ed Totals
Time	Tue	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoo
12:00		0	1			0	1				
12:15		0	1		1	0	3				
12:30		0	2		- 1	0	2		- 1		
12:45		0	3	0	7	0	0	0	6	0	1
01:00		0	1		1	0	1				
01:15		0	0			0	2				
01:30		0	1			0	1		- 1		
01:45		0	1	0	3	0	2	0	6	0	
02:00		0	2			0	3		-		
02:15		0	1			0	0				
02:30		0	1			0	1		1		
02:45		0	3	0	7	0	2	0	6	0	1
03:00		0	0		- 1	0	1				
03:15		0	0			0	0				
03:30		0	1			0	2				
03:45		0	1	0	2	0	0	0	3	0	
04:00		0	0		H	0	0				
04:15		0	2			0	1				
04:30		0	2		- 1	0	1				
04:45		0	1	0	5	0	0	0	2	0	
05:00		0	7		- 1	0	0				
05:15		0	6			0	0				
05:30		0	0			0	0		1		
05:45		0	0	0	13	0	0	0	0	0	1
06:00		0	0		1	0	0		0.00		
06:15		0	0			0	0				
06:30		0	0		1	0	0		- 1		
06:45		0	0	0	0	1	0	1	0	1	
07:00		0	0			0	0				
07:15		0	0		1	1	0				
07:30		0	0			8	0				
07:45		1	1	1	1	3	1	12	1	13	
08:00		0	0			4	0				
08:15		1	0			1	0				
08:30		1	0		- 1	1	1		1		
08:45		0	1	2	1	0	0	6	1	8	
09:00		0	0		1	0	0		1		
09:15		0	0			1	0				
09:30		1	0			2	0				
09:45		0	0	1	0	0	0	3	0	4	(
10:00		0	0			2	0				
10:15		0	0			1	0				
10:30		1	0			0	0				
10:45		1	0	2	0	1	0	4	0	6	- (
11:00		0	0			2	0				
11:15		3	0			1	0				
11:30		2	0			1	0				
11:45		2	0	7	0	0	0	4	0	11	
Total		13	39			30	25			43	64
Percent		25.0%	75.0%			54.5%	45.5%			40.2%	59.8%

Location: Tarkiln Hill Place Location: South of Chaffee Street City/State: New Bedford, MA

8807VL02

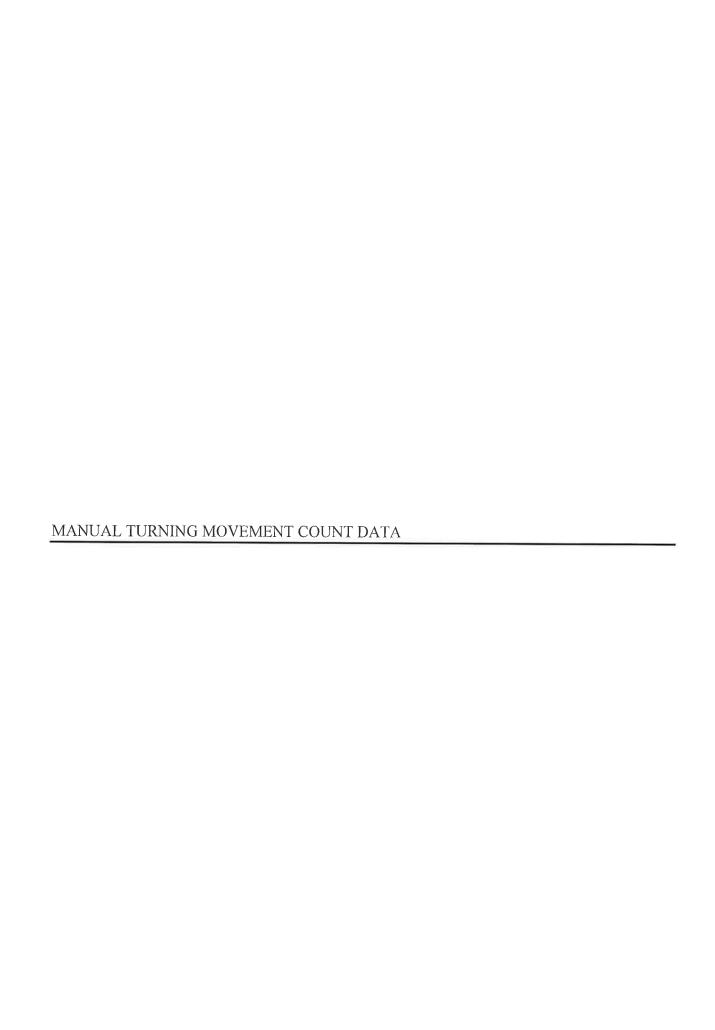
Start	12/2/2020		NB	Hour	Totals		SB	Hour	Totals	Combin	ed Totals
Time	Wed	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoo
12:00		0	0			0	2				7 11100
12:15		0	0			0	0				
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12:45		0	1	0	3	0	0	0	3	0	
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02:15		Ö	0		1	0	4				
02:30		0	2		1	0	0				
02:45		0	1	0	5	0	1	0	7	0	4
03:00		0	ó	U	3	0		0	7	0	1.
03:15		0	2				0				
03:30		0	0		- 1	0	1				
03:45		0		0		0	1				
04:00			1	U	3	0	0	0	2	0	,
04:00		0	1			0	1		- 1		
04:15		0	0			0	0				
04:45		0	4			0	1				
		0	3	0	8	0	0	0	2	0	10
05:00		0	3			0	1				
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05:30		0	1			0	0		1		
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06:00		0	0			0	0				
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07:00		1	0			2	0		1		
07:15		2	0		- 1	2	0				
07:30		0	0		1	3	0		1		
07:45		1	0	4	0	4	0	9	0	13	(
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Total		18	33		- 01	31	22		0.1	13	0
ercent		35.3%	64.7%			58.5%	41.5%			49	55
Grand										47.1%	52.9%
Total		31	72			61	47			92	119
ercent		30.1%	69.9%			56.5%	43.5%				
		55,170	00,070			30,376	43.3%			43.6%	56.4%

8807VL02

Accurate Counts 978-664-2565

Location: Tarkiln Hill Place Location: South of Chaffee Street City/State: New Bedford, MA

NB	SB	- 8	SB	NB SB	AN TH	NB Sat	Sun	Week Average
0			0			*	NB SB	NB
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17:00		16:00	13:00				57 A	17:00
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	AADT 106	(92)						
	A CANADA IN THE SECOND							



N/S Street: Church Street E/W Street: Chaffee Street City/State: New Bedford, MA Weather: Cloudy

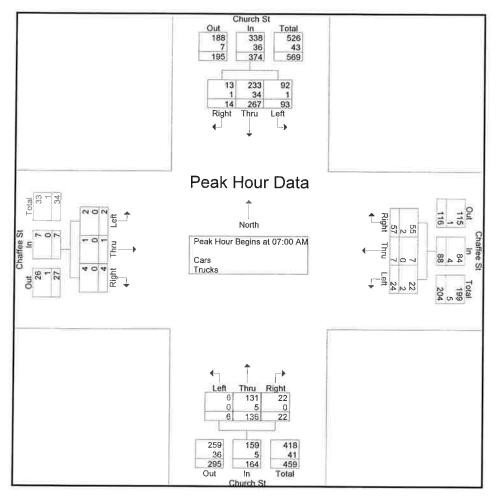
						- Trucks	inted- Cars	Groups Pr					
		naffee St om West	4-1		nurch St om South			affee St om East			hurch St om North	Fre	
Int. Total	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
176	2	0	0	5	29	1	11	0	7	3	95	23	07:00 AM
162	2	1	1	9	45	2	14	1	5	2	54	26	07:15 AM
163	0	Ô	ò	2	36	2	25	3	6	6	59	24	07:30 AM
132	0	0	_ 1	6	26	1	7	3	6	3	59	20	07:45 AM
633	4	1	2	22	136	6	57	7	24	14	267	93	Total
144	1	0	0	6	50	3	13	1	6	3	49	12	08:00 AM
124	0	0	3	6	41	1	9	0	6	2	51	5	08:15 AM
128	1	1	2	8	35	1	10	4	9	6	37	14	08:30 AM
125	3	0	3	4	37	1	11	1	3	2	49	11	08:45 AM
521	5	1	8	24	163	6	43	6	24	13	186	42	Total
1154	9	2	10	46	299	12	100	13	48	27	453	135	Grand Total
	42.9	9.5	47.6	12.9	83.8	3.4	62.1	8.1	29.8	4.4	73.7	22	Apprch %
	0.8	0.2	0.9	4	25.9	1	8.7	1.1	4.2	2.3	39.3	11.7	Total %
1075	9	2	7	44	283	12	96	13	46	26	404	133	Cars
93.2	100	100	70	95.7	94.6	100	96	100	95.8	96.3	89.2	98.5	% Cars
79	0	0	3	2	16	0	4	0	2	1	49	2	Trucks
6.8	0	Õ	30	4.3	5.4	0	4	0	4.2	3.7	10.8	1.5	% Trucks

			rch St				fee St				rch St				ffee St		
			North			From	n East			From	South			Fron	n West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App Total	Int. Total
Peak Hour Analy	sis Fror	n 07:00	AM to C	08:45 AM -	Peak 1	of 1							- CONTRACTOR			A. C.	
Peak Hour for E	ntire Inte	rsection	Begins	at 07:00	AM												
07:00 AM	23	95	3	121	7	0	11	18	1	29	5	35	0	0	2	2	176
07:15 AM	26	54	2	82	5	1	14	20	2	45	9	56	1	1	2	4	162
07:30 AM	24	59	6	89	6	3	25	34	2	36	2	40	0	0	0	0	163
07:45 AM	20	59	3	82	6	3	7	16	1	26	6	33	1	0	0	. 1	132
Total Volume	93	267	14	374	24	7	57	88	6	136	22	164	2	1	4	7	633
% App. Total	24.9	71.4	3.7		27.3	8	64.8		3.7	82.9	13.4		28.6	14.3	57.1		
PHF	.894	.703	.583	.773	.857	.583	.570	.647	.750	.756	.611	.732	.500	.250	.500	.438	899
Cars	92	233	13	338	22	7	55	84	6	131	22	159	2	1	4	7	588
% Cars	98.9	87.3	92.9	90.4	91.7	100	96.5	95.5	100	96.3	100	97.0	100	100	100	100	92.9
Trucks	1	34	1	36	2	0	2	4	0	5	0	5	0	0	0	0	45
% Trucks □	1.1	12.7	7.1	9.6	8.3	0	3.5	4.5	0	3.7	0	3.0	0	0	0	0	7.1

N/S Street: Church Street E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

File Name @ 88070001 Site Code 88070001 Start Date 12/1/2020

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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

	07:00 AM				07:15 AM				08:00 AM				08:00 AM			
+0 mins.	23	95	3	121	5	1	14	20	3	50	6	59	0	0	1	1
+15 mins.	26	54	2	82	6	3	25	34	1	41	6	48	3	0	0	3
+30 mins.	24	59	6	89	6	3	7	16	1	35	8	44	2	1	1	4
+45 mins.	20	59	3	82	6	1	13	20	1	37	4	42	3	0	3	6
otal Volume	93	267	14	374	23	8	59	90	6	163	24	193	8	1	5	14
% App, Total	24.9	71.4	3.7		25.6	8.9	65.6		3.1	84.5	12.4		57.1	7.1	35.7	
PHF	894	.703	.583	.773	.958	-667	.590	.662	.500	.815	.750	_818	.667	-250	-417	.583
Cars	92	233	13	338	22	8	57	87	6	152	22	180	5	1	5	11
% Cars	98.9	87.3	92.9	90.4	95.7	100	96.6	96.7	100	93.3	91.7	93.3	62.5	100	100	78.6
Trucks	1	34	1	36	1	0	2	3	0	11	2	13	3	0	0	3
% Trucks	1.1	12.7	7.1	9,6	4.3	0	3.4	3.3	0	6.7	8.3	6.7	37.5	0	0	21-4

N/S Street : Church Street E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

						Cars	ps Printed-	Grou					
		naffee St om West			hurch St om South			naffee St om East			hurch St om North	Fr	
Int. Total	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
150	2	0	0	5	27	1	10	0	6	3	73	23	07:00 AM
158	2	1	1	9	43	2	14	1	5	2	52	26	07:15 AM
153	0	0	0	2	35	2	24	3	6	5	52	24	07:30 AM
127	0	0	1	6	26	1	7	3	5	3	56	19	07:45 AM
588	4	1	2	22	131	6	55	7	22	13	233	92	Total
136	1 /	0	0	4	48	3	12	1	6	3	47	11	08:00 AM
111	Ó	Ō	0	6	38	1	9	0	6	2	44	5	08:15 AM
123	1	1	2	8	33	1	9	4	9	6	35	14	08:30 AM
117	3	0	3	4	33	1	11	1	3	2	45	11	08:45 AM
487	5	1	5	22	152	6	41	6	24	13	171	41	Total
1075	9	2	7	44	283	12	96	13	46	26	404	133	Grand Total
1070	50	11,1	38.9	13	83.5	3.5	61.9	8.4	29.7	4.6	71.8	23.6	Apprch %
	0.8	0.2	0.7	4.1	26.3	1.1	8.9	1.2	4.3	2.4	37.6	12.4	Total %

			rch St North				ffee St				rch St South				ffee St West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru		App Total	Left			App. Total	Int. Total
Peak Hour Analy	sis Fror	n 07:00	AM to 0	8:45 AM -	Peak 1	of 1					3.03			7711 G	rugite	7 pp. Total	IIII. TOTAL
Peak Hour for En																	
07:00 AM	23	73	3	99	6	0	10	16	1	27	5	33	0	0	2	2	150
07:15 AM	26	52	2	80	5	1	14	20	2	43	9	54	1	1	2	4	158
07:30 AM	24	52	5	81	6	3	24	33	2	35	2	39	0	0	0	0	153
07:45 AM	19	56	3	78	5	3	7	15	1	26	6	33	1	0	0	1	127
Total Volume	92	233	13	338	22	7	55	84	6	131	22	159	2	1	4	7	588
% App. Total	27.2	68.9	3.8		26.2	8.3	65.5		3.8	82.4	13.8		28.6	14.3	57.1		000
PHF	.885	.798	.650	.854	.917	.583	.573	.636	.750	.762	.611	.736	.500	.250	.500	.438	.930

N/S Street : Church Street E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

File Name : 88070001 Site Code \$88070001 Start Date : 12/1/2020

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Groups Printed- Trucks Church St Chaffee St Church St Chaffee St From North From East From West From South Start Time Thru 2 2 Left Right Thru Left Thru Right Left i Right Left Thru Right Int. Total 07:00 AM 07:15 AM Ó 7 07:30 AM 07:45 AM Total 08:00 AM 08:15 AM 08:30 AM 08:45 AM Total Grand Total Apprch % 3.8 94.2 33.3 1.9 66.7 88.9 11.1 Total % 2.5 1.3 2.5 5.1 20.3 2.5 3.8

			rch St North				fee St n East				rch St South				ffee St West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left			App. Total	Left	Thru		App. Total	Int. Total
Peak Hour Analy	sis Fror	n 07:00	AM to C	08:45 AM	- Peak 1	of 1	•									1.5	
Peak Hour for Er																	
07:00 AM	0	22	0	22	1	0	1	2	0	2	0	2	0	0	0	0	26
07:15 AM	0	2	0	2	0	0	0	0	0	2	0	2	0	0	Ō	0	4
07:30 AM	0	7	1	8	0	0	1	1.	0	1	0	1	0	0	0	0	10
07:45 AM	1	3	0	4	1	0	0	1	0	0	0	0	0	0	0	0	5
Total Volume	1	34	1	36	2	0	2	4	0	5	0	5	0	0	0	0	45
% App. Total	2.8	94.4	2.8		50	0	50		0	100	0	-	0	Ō	0	Ū	
PHF	.250	.386	.250	.409	.500	.000	.500	.500	.000	.625	.000	.625	.000	.000	.000	.000	433

N/S Street: Church Street E/W Street : Chaffee Street City/State : New Bedford, MA

File Name : 88070001 Site Code | 88070001 Start Date 12/1/2020 Page No ::10

77.8

22.2

Weather : Cloudy

Apprch %

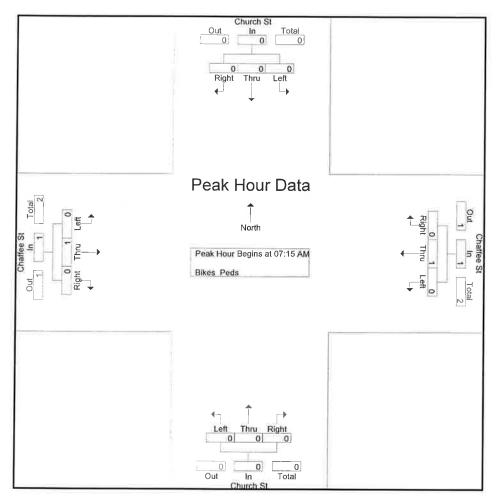
Total %

Groups Printed-Bikes Peds Church St Chaffee St Church St Chaffee St From North From East From South From West Thru Right Start Time Left Peds Thru Right Left Peds Thru Right Peds Left Left Thru Right Peds Exclu Total Inclu Total Int. Total 07:00 AM 07:15 AM 07:30 AM 07:45 AM Total MA 00:80 08:15 AM 08:30 AM 08:45 AM Total Grand Total

			rch St North				ffee St n East				rch St South				fee St West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fror	n 07:00	AM to C	8:45 AM	Peak 1	of 1	81	7.5%				3.01	(1.70)				
Peak Hour for En	ntire Inte	ersection	Begins	at 07:15	AM												
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	o l	0	1	0	1	1
07:45 AM	0	0	0	0	0	0	0	0	0	0	Ô	ō	0	0	Õ	0	0
08:00 AM	0	0	0	0	0	1	0	1	0	0	0	0	0	o.	ñ	0	1
Total Volume	0	0	0	0	0	1	0	1	0	0	0	0	0	1	0	1	2
% App. Total	0	0	0		0	100	0		0	ō	Ô	0	0	100	0		-
PHF	.000	.000	.000	.000	.000	.250	.000	.250	.000	.000	.000	.000	.000	-250	.000	250	500

N/S Street: Church Street E/W Street: Chaffee Street City/State : New Bedford, MA Weather : Cloudy

File Name : 88070001 Site Code : 88070001 Start Date : 12/1/2020 Page No : 11



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Реак	Hour	tor	Each	Approach	Begins at:	
			07:0	00 AM		

	07:00 AM				07:15 AM				07:00 AM				07:00 AM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	Õ	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
+45 mins.	0	0	0	0	0	1	0	1	0	0	0	0	o	0	0	0
Total Volume	0	0	0	0	0	. 1	0	1	0	0	0	0	0	1	0	1
% App. Total	0	0	0		0	100	0	570	0	Õ	0	7.5	Õ	100	n	
PHF	.000	.000	.000	.000	.000	.250	.000	-250	.000	.000	.000	.000	.000	250	.000	.250

N/S Street : Church Street E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

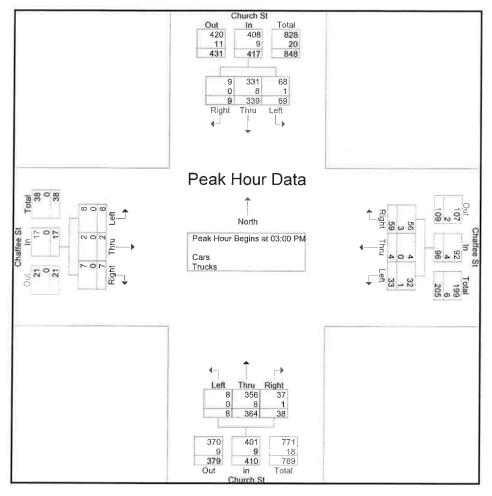
		naffee St om West			hurch St om South	Fro		affee St om East	Fr		hurch St om North	Fr	
Int. Tota	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
153	2	1	0	5	54	0	13	1	3	0	57	17	02:00 PM
175	0	2	2	4	64	0	17	2	2	0	62	20	02:15 PM
240	1	4	1	6	85	1	42	3	13	4	67	13	02:30 PM
201	3	2	1	10	73	2	15	1	7	3	71	13	02:45 PM
769	6	9	4	25	276	3	87	7	25	7	257	63	Total
289	3	0	4	12	134	1	25	0	6	2	84	18	03:00 PM
205	2	0	0	10	92	3	11	1	5	1	69	11	03:15 PM
238	1	1	1	7	72	0	13	3	9	5	109	17	03:30 PM
208	1	1	3	9	66	4	10	0	13	1	77	23	03:45 PM
940	7	2	8	38	364	8	59	4	33	9	339	69	Total
234	4	0	5	13	77	2	18	1	7	3	93	11	04:00 PM
216	0	0	6	16	66	2	12	4	8	3	84	15	04:15 PM
227	0	0	7	12	61	4	16	1	8	3	95	20	04:30 PM
207	4	4	4	10	57	3	11	3	6	9	79	17	04:45 PM
884	8	4	22	51	261	11	57	9	29	18	351	63	Total
234	5	5	9	6	84	1	11	1	12	3	80	17	05:00 PM
163	5	2	3	6	65	1	9	1	5	0	58	8	05:15 PM
146	2	2	0	6	57	1	10	3	6	2	43	14	05:30 PM
149	4	3	8	8	44	3	6	3	3	8	46	13	05:45 PM
692	16	12	20	26	250	6	36	8	26	13	227	52	Total
3285	37	27	54	140	1151	28	239	28	113	47	1174	247	Grand Total
	31.4	22.9	45.8	10.6	87.3	2.1	62.9	7.4	29.7	3.2	80	16.8	Apprch %
	1.1	0.8	1.6	4.3	35	0.9	7.3	0.9	3.4	1.4	35.7	7.5	Total %
3194	36	27	52	139	1129	28	214	28	111	46	1140	244	Cars
97.2	97.3	100	96.3	99.3	98.1	100	89.5	100	98.2	97.9	97.1	98.8	% Cars
91	1	0	2	1	22	0	25	0	2	1	34	3	Trucks
2.8	2.7	0	3.7	0.7	1.9	0	10.5	0	1.8	2.1	2.9	1.2	% Trucks

		From	rch St North				ffee St n East				rch St South				ffee St n West		
Start Time	Left			App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fror	n 02:00	PM to 0	5:45 PM -	Peak 1	of 1											
Peak Hour for Er	ntire Inte	ersection	n Begins	at 03:00	PM												
03:00 PM	18	84	2	104	6	0	25	31	1	134	12	147	4	0	3	7	289
03:15 PM	11	69	1	81	5	1	11	17	3	92	10	105	0	0	2	2	205
03:30 PM	17	109	5	131	9	3	13	25	0	72	7	79	1	1	1	3	238
03:45 PM	23	77	1	101	13	0	10	23	4	66	9	79	3	1	- 1	5	208
Total Volume	69	339	9	417	33	4	59	96	8	364	38	410	8	2	7	17	940
% App. Total	16.5	81.3	2.2		34.4	4.2	61.5		2	88.8	9.3		47.1	11.8	41.2		
PHF	.750	.778	.450	.796	-635	.333	.590	.774	.500	679	.792	.697	.500	.500	.583	.607	.813
Cars	68	331	9	408	32	4	56	92	8	356	37	401	8	2	7	17	918
% Cars	98.6	97.6	100	97.8	97.0	100	94.9	95.8	100	97.8	97.4	97.8	100	100	100	100	97.7
Trucks	1	8	0	9	1	0	3	4	0	8	1	9	0	0	0	0	22
% Trucks	1.4	2.4	0	2.2	3.0	0	5.1	4.2	0	2.2	2.6	2.2	0	0	0	0	2.3

N/S Street: Church Street E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

File Name : 88070001 Site Code : 88070001 Start Date : 12/1/2020

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Peak Hour Analysis From 02:00 PM to 05:45 PM - Peak 1 of 1

	03:30 PM				02:15 PM				02:30 PM				04:30 PM			
+0 mins	17	109	5	131	2	2	17	21	1	85	6	92	7	0	0	7
+15 mins.	23	77	1	101	13	3	42	58	2	73	10	85	4	4	4	12
+30 mins.	11	93	3	107	7	1	15	23	1	134	12	147	9	5	5	19
+45 mins.	15	84	3	102	6	0	25	31	3	92	10	105	3	2	5	10
otal Volume	66	363	12	441	28	6	99	133	7	384	38	429	23	11	14	48
App. Total	15	82.3	2.7		21.1	4.5	74.4		1.6	89.5	8.9		47.9	22.9	29.2	
PHF	.717	-833	-600	.842	.538	.500	.589	.573	.583	.716	.792	.730	.639	.550	.700	.632
Cars	66	354	12	432	27	6	80	113	7	372	37	416	23	11	14	48
% Cars	100	97.5	100	98	96.4	100	80.8	85	100	96.9	97.4	97	100	100	100	100
Trucks	0	9	0	9	1	0	19	20	0	12	1	13	0	0	0	0
% Trucks	0	2.5	0	2	3.6	0	19.2	15	0	3.1	2.6	3	0	Ω	0	0

N/S Street : Church Street E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

		naffee St om West			nurch St m South			affee St om East	Fr		hurch St om North	Fr	
Int. Tota	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
150	2	1	0	5	54	0	12	1	3	0	55	17	02:00 PM
168	0	2	1	4	60	0	17	2	2	0	58	19	02:15 PM
213	1	4	1	6	80	1	26	3	12	3	63	13	02:30 PM
186	2	2	1	10	71	2	14	1	7	3	61	12	02:45 PM
714	5	9	3	25	265	3	69	7	24	6	237	61	Total
28	3	0	4	12	129	1	23	0	6	2	83	18	03:00 PM
201	2	0	0	9	92	3	11	1	5	1	67	10	03:15 PM
233	1	1	1	7	72	0	12	3	8	5	106	17	03:30 PM
203	1	1	3	9	63	4	10	0	13	1	75	23	03:45 PM
918	7	2	8	37	356	8	56	4	32	9	331	68	Total
229	4	0	5	13	77	2	17	1	7	3	89	11	04:00 PM
213	0	0	5	16	64	2	12	4	8	3	84	15	04:15 PM
225	0	0	7	12	61	4	15	1	8	3	94	20	04:30 PM
205	4	4	4	10	56	3	11	3	6	9	78	17	04:45 PM
872	8	4	21	51	258	11	55	9	29	18	345	63	Total
233	5	5	9	6	84	1	10	1	12	3	80	17	05:00 PM
163	5	2	3	6	65	1	9	1	5	0	58	8	05:15 PM
145	2	2	0	6	57	1	9	3	6	2	43	14	05:30 PM
149	4	3	8	8	44	3	6	3	3	8	46	13	05:45 PM
690	16	12	20	26	250	6	34	8	26	13	227	52	Total
3194	36	27	52	139	1129	28	214	28	111	46	1140	244	Grand Total
	31.3	23.5	45.2	10.7	87.1	2.2	60.6	7.9	31.4	3.2	79.7	17.1	Apprch %
	1.1	0.8	1.6	4.4	35.3	0.9	6.7	0.9	3.5	1.4	35.7	7.6	Total %

		100	rch St North				ffee St n East				rch St South			-	ffee St West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App Total	Int. Total
Peak Hour Analy	sis Fror	n 02:00	PM to 0	05:45 PM -	- Peak 1	of 1	-										
Peak Hour for Er	ntire Inte	rsection	n Begins	s at 03:00	PM												
03:00 PM	18	83	2	103	6	0	23	29	1	129	12	142	4	0	3	7	281
03:15 PM	10	67	1	78	5	1	11	17	3	92	9	104	0	0	2	2	201
03:30 PM	17	106	5	128	8	3	12	23	0	72	7	79	1	1	1	3	233
03:45 PM	23	75	1	99	13	0	10	23	4	63	9	76	3	1	1	5	203
Total Volume	68	331	9	408	32	4	56	92	8	356	37	401	8	2	7	17	918
% App. Total	16.7	81.1	2.2		34.8	4.3	60.9		2	88.8	9.2		47.1	11.8	41.2		
PHF	.739	.781	.450	.797	.615	.333	.609	.793	.500	.690	.771	.706	.500	-500	.583	-607	.817

N/S Street : Church Street E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

		naffee St om West			nurch St om South	Fro		naffee St om East	Fr		hurch St om North	Fr	
Int. Tota	Right	Thru	Left	Start Time									
	0	0	0	0	0	0	1	0	0	0	2	0	02:00 PM
10	0	0	1	0	4	0	0	0	0	0	4	1	02:15 PM
27	0	0	0	0	5	0	16	0	1	1	4	0	02:30 PM
18	1	0	0	.0	2	0	1	0	00	0	10	1	02:45 PM
55	1	0	1	0	11	0	18	0	1	1	20	2	Total
3	0	0	0	0	5	0	2	0	0	0	1	0	03:00 PM
4	0	0	0	1	0	0	0	0	0	0	2	1	03:15 PM
	0	0	0	0	0	.0	1	0	1	0	3	0	03:30 PM
	0	0	0	0	3	0	0	.0	0	0	2	0	03:45 PM
22	0	0	0	1	8	0	3	0	1	0	8	1	Total
5	0	0	0	0	0	0	1	0	0	0	4	0	04:00 PM
3	0	0	1	0	2	0	0	0	0	0	0	0	04:15 PM
2	0	0	0	0	0	0	1	0	0	0	1	0	04:30 PM
2	0	0	0	0	1	0	0	0	0	0	11	0	04:45 PM
12	0	0	1	0	3	0	2	0	0	0	6	0	Total
1	0	0	0	0	0	0	1	0	0	0	0	0	05:00 PM
(0	0	0	0	0	0	0	0	0	0	0	0	05:15 PM
1	0	0	0	0	0	.0	1	0	0	0	0	0	05:30 PM
C	0	0	0	0	0	0	0	0	0	0	0	0	05:45 PM
2	0	0	0	0	0	0	2	0	0	0	0	0	Total
91	1	0	2	1	22	0	25	0	2	1	34	3	Grand Total
	33.3	0	66.7	4.3	95.7	0	92.6	0	7.4	2.6	89.5	7.9	Apprch %
	1.1	0	2.2	1.1	24.2	0	27.5	0	2.2	1.1	37.4	3.3	Total %

		From	ch St North				ffee St n East				rch St South				ffee St West		
Start Time	Left	Thru	Right	App Total	Left	Thru	Right	App. Total	Left	Thru	Right	App Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron				Peak 1	of 1									-		
Peak Hour for Er	itire Inte	rsection	Begins	at 02:15	PM												
02:15 PM	1	4	0	5	0	0	0	0	0	4	0	4	1	0	0	1	10
02:30 PM	0	4	1	5	1	0	16	17	0	5	0	5	0	ñ	0	0	27
02:45 PM	1	10	0	11	0	0	1	1	0	2	0	2	ō	ñ	1	1	15
03:00 PM	0	1	0	1	0	0	2	2	0	5	0	5	0	0	0	Ö	8
Total Volume	2	19	1	22	1	0	19	20	0	16	0	16	1	0	1	2	60
% App. Total	9.1	86.4	4.5		5	0	95		0	100	0		50	ō	50	~	
PHF	.500	.475	.250	.500	.250	.000	.297	-294	-000	.800	.000	.800	.250	.000	.250	.500	.556

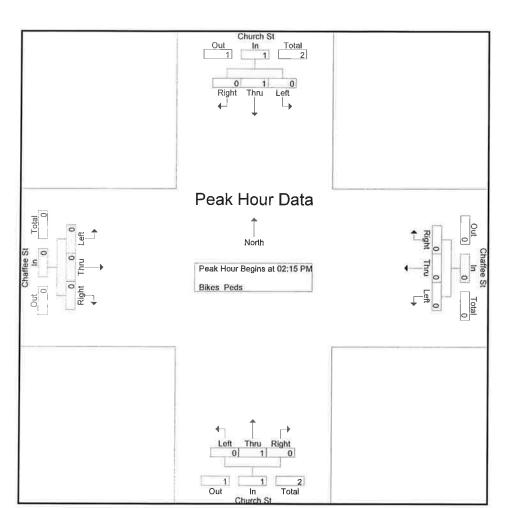
N/S Street : Church Street E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

								Groups	Printed	- Bikes	Peds								
		Chur	ch St			Chaff	ee St			Chur	ch St			Chaft	ee St		Ī		
		From	North			From	East			From	South			From	West				
Start Time	Left	Thru	Right	Peds	Left!	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total	Inclu Total	Int. Total
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
Total	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
03:00 PM	0	0	0	0	0	0	0	0	0	1	0	2	0	0	0	0	2	1	3
03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	- 0	0	1	1	2
Total	0	1	0	1	0	0	0	0	0	1	0	2	0	0	0	0	3	2	5
04:00 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	1
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	1
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	1	1	2
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	2	0	1	0	0	0	1	0	2	0	2	0	0	0	0	4	4	8
Apprch %	0	100	0		0	0	0		0	100	0		0	0	0				
Total %	0	50	0		0	0	0		0	50	0		0	0	0		50	50	

			rch St North				ffee St n East				rch St South				fee St West		
Start Time	Left	Thru	Right	App Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron				Peak 1	of 1											
Peak Hour for Er	ntire Inte	rsection	n Begins	s at 02:15	PM												
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:00 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1
Total Volume	0	1	0	1	0	0	0	0	0	1	0	1	0	0	0	0	2
% App. Total	0	100	0		0	0	0		0	100	0	W	0	0	0		
PHF	.000	250	.000	.250	.000	-000	.000	.000	.000	.250	.000	.250	.000	.000	.000	.000	.500

N/S Street: Church Street E/W Street: Chaffee Street City/State: New Bedford, MA Weather: Cloudy

File Name : 88070001 Site Code : 88070001 Start Date : 12/1/2020 Page No : 11



Peak Hour Analysis From 02:00 PM to 05:45 PM - Peak 1 of 1

Peak He	our for	Each A	Approach	Begins at:	

eak Hour for E	acn App	roach B	egins at:													
	02:00 PM				02:00 PM				02:15 PM				02:00 PM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	1	0	- 1	0	0	0	0	0	1	0	1	0	0	0	0
Total Volume	0	. 1	0	1	0	0	0	0	0	1	0	1	0	0	0	0
% App. Total	0	100	0		0	0	0		0	100	0		0	0	0	
PHF	.000	.250	.000	.250	.000	.000	.000	.000	.000	.250	.000	.250	-000	.000	.000	.000

N/S Street : Tarklin Place E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

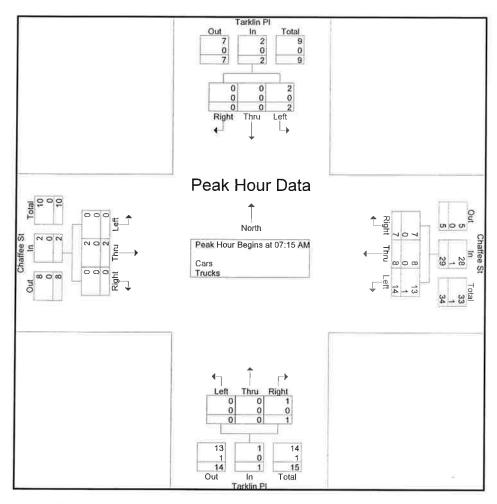
	Fr	arklin Pl om North			haffee St om East			arklin PI om South			naffee St om West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Tota
07:00 AM	2	0	0	0	3	0	0	0	0	0	0	0	5
07:15 AM	2	0	0	1	3	1	0	0	0	0	1	0	8
07:30 AM	0	0	0	6	1	3	0	0	0	0	0	0	10
07:45 AM	0	0	0	3	2	2	0	0	1	0	0	0	8
Total	4	0	0	10	9	6	0	0	1	0	1	0	31
08:00 AM	0	0	0	4	2	1	0	0	0	0	1	0	8
08:15 AM	0	0	0	1	2	0	0	0	2	0	1	0	6
08:30 AM	.1	0	0	2	8	0	0	0	0	0	1	0	12
08:45 AM	2	0	0	0	3	1	0	0	0	0	- 1	0	7
Total	3	0	0	7	15	2	0	0	2	0	4	0	33
Grand Total	7	0	0	17	24	8	0	0	3	0	5	0	64
Apprch %	100	0	0	34.7	49	16.3	0	0	100	0	100	0	
Total %	10.9	0	0	26.6	37.5	12.5	0	0	4.7	0	7.8	0	
Cars	7	0	0	16	24	8	0	0	2	0	4	0	61
% Cars	100	0	0	94.1	100	100	0	0	66.7	0	80	0	95.3
Trucks	0	0	0	1	0	0	0	0	1	0	1	0	3
% Trucks	0	0	0	5.9	0	0	0	0	33.3	0	20	0	4.7

			din Pl				ffee St				klin Pl			Cha	ffee St		
		From	North			Fron	n East			From	South			Fron	1 West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fror	n 07:00	AM to 0	8:45 AM -	Peak 1	of 1	- 3 -										
Peak Hour for Er	ntire Inte	ersection	Begins	at 07:15	AM												
07:15 AM	2	0	0	2	1	3	1	5	0	0	0	0	0	1	0	1	8
07:30 AM	0	0	0	0	6	1	3	10	0	0	0	0	0	0	0	0	10
07:45 AM	0	0	0	0	3	2	2	7	0	0	1	1	0	0	0	0	8
08:00 AM	0	0	0	0	4	2	1	7	0	0	0	0	0	1	0	1	8
Total Volume	2	0	0	2	14	8	7	29	0	0	1	1	0	2	0	2	34
% App. Total	100	0	0		48.3	27.6	24.1		0	0	100		0	100	0		
PHF	.250	.000	.000	.250	.583	.667	.583	.725	.000	.000	.250	.250	.000	.500	.000	.500	.850
Cars	2	0	0	2	13	8	7	28	0	0	1	1	0	2	0	2	33
% Cars	100	0	0	100	92.9	100	100	96.6	0	0	100	100	0	100	0	100	97.1
Trucks	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	1
% Trucks	0	0	0	0	7.1	0	0	3.4	0	0	0	0	n	0	0	0	2.9

N/S Street : Tarklin Place E/W Street : Chaffee Street City/State : New Bedford, MA

Weather : Cloudy

File Name : 88070002 Site Code : 88070002 Start Date : 12/1/2020 Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

ICII APP	TOACH D	egins at.													
07:00 AM				07:15 AM				07:30 AM				08:00 AM			
2	0	0	2	1	3	1	5	0	0	0	0	0	1	0	1
2	0	0	2	6	1	3	10	0	0	1	1	0	1	0	1
0	0	0	0	3	2	2	7	0	0	0	0	0	1	0	1
0	0	0	0	4	2	1	7	0	0	2	2	0	- 1	0	1
4	0	0	4	14	8	7	29	0	0	3	3	0	4	0	4
100	0	0		48.3	27.6	24.1		0	0	100		0	100	0	
.500	.000	.000	.500	.583	.667	.583	.725	.000	.000	.375	.375	.000	1.000	.000	1.000
4	0	0	4	13	8	7	28	0	0	2	2	0	3	0	3
100	0	0	100	92.9	100	100	96.6	0	0	66.7	66.7	0	75	0	75
0	0	0	0	1	0	0	1	0	0	1	1	0	1	0	1
0	0	0	0	7.1	0	0	3.4	0	0	33.3	33.3	0	25	0	25
	07:00 AM 2 2 0 0 4 100 500 4	07:00 AM 2	2 0 0 2 0 0 0 0 0 0 0 0 4 0 0 100 0 0 500 .000 .000 4 0 0	07:00 AM 2	07:00 AM 2 0 0 2 1 2 0 0 2 6 0 0 0 3 0 0 0 4 4 0 0 4 14 100 0 0 48.3 500 .000 .000 .500 .583 4 0 0 4 13 100 0 0 100 92.9 0 0 0 0 1	07:00 AM 2	07:00 AM 2	07:00 AM 2 0 0 2 1 3 1 5 2 0 0 0 2 6 1 3 10 0 0 0 0 3 2 2 7 0 0 0 0 4 2 1 7 4 0 0 4 14 8 7 29 100 0 0 0 500 583 667 583 .725 4 0 0 4 13 8 7 28 100 0 0 0 100 92.9 100 100 96.6 0 0 0 1 0 0 1	07:00 AM 2 0 0 2 1 3 1 5 00 2 0 0 0 2 6 1 3 10 0 0 0 0 0 3 2 2 7 0 0 0 0 0 4 2 1 7 0 100 0 0 4 14 8 7 29 0 1500 .000 .000 .500 .583 .667 .583 .725 .000 4 0 0 4 13 8 7 28 0 100 0 0 0 4 13 8 7 28 0 100 0 0 0 100 92.9 100 100 96.6 0 0 0 0 0 1 0 0 0 1 0	07:00 AM 07:15 AM 07:30 AM 2 0 0 2 1 3 1 5 0 0 2 0 0 2 6 1 3 10 0 0 0 0 0 3 2 2 7 0 0 4 0 0 4 14 8 7 29 0 0 100 0 0 48.3 27.6 24.1 0 0 500 .000 .000 .500 .583 .667 .583 .725 .000 .000 4 0 0 4 13 8 7 28 0 0 100 0 100 92.9 100 100 96.6 0 0 0 0 0 1 0 0 1 0 0	2 0 0 2 1 3 1 5 0 0 0 0 2 0 0 2 6 1 3 10 0 0 0 1 0 0 0 0 3 2 2 7 0 0 0 0 4 0 0 4 14 8 7 29 0 0 3 100 0 0 48.3 27.6 24.1 0 0 100 500 .000 .000 .500 .583 .667 .583 .725 .000 .000 .375 4 0 0 4 13 8 7 28 0 0 2 100 0 100 92.9 100 100 96.6 0 0 66.7 0 0 0 0 1 0 0 <th< td=""><td>07:00 AM 07:15 AM 07:30 AM 2 0 0 2 1 3 1 5 0 0 0 0 2 0 0 2 6 1 3 10 0 0 1 1 0 0 0 0 3 2 2 7 0 0 0 0 4 0 0 4 14 8 7 29 0 0 3 3 100 0 0 48.3 27.6 24.1 0 0 100 500 .000 .000 .500 .583 .667 .583 .725 .000 .000 375 .375 4 0 0 4 13 8 7 28 0 0 2 2 100 0 0 1 0 0 66.7 66.7 0 0<td>07:00 AM 2 0 0 2 1 3 1 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0</td><td>07:00 AM 07:15 AM 07:30 AM 08:00 AM 2 0 0 2 1 3 1 5 0 0 0 0 0 1 2 0 0 2 6 1 3 10 0 0 1 1 0 1 0 0 0 0 3 2 2 7 0 0 0 0 0 1 4 0 0 4 14 8 7 29 0 0 3 3 0 4 100 0 0 48.3 27.6 24.1 0 0 100 0 100 500 .000 .000 .583 .667 .583 .725 .000 .000 .375 .000 1.000 4 0 0 4 13 8 7 28 0 0 2 2 0</td><td>07:00 AM 07:15 AM 07:30 AM 08:00 AM 2 0 0 2 1 3 1 5 0 0 0 0 1 0 2 0 0 2 6 1 3 10 0 0 1 1 0 1 0 0 0 0 0 3 2 2 7 0 0 0 0 1 0 4 0 0 4 14 8 7 29 0 0 3 0 4 0 100 0 0 48.3 27.6 24.1 0 0 100 0 100 0 500 .000 .000 .583 .667 .583 .725 .000 .000 .375 .000 1,000 .000 4 0 0 4 13 8 7 28 0 <t< td=""></t<></td></td></th<>	07:00 AM 07:15 AM 07:30 AM 2 0 0 2 1 3 1 5 0 0 0 0 2 0 0 2 6 1 3 10 0 0 1 1 0 0 0 0 3 2 2 7 0 0 0 0 4 0 0 4 14 8 7 29 0 0 3 3 100 0 0 48.3 27.6 24.1 0 0 100 500 .000 .000 .500 .583 .667 .583 .725 .000 .000 375 .375 4 0 0 4 13 8 7 28 0 0 2 2 100 0 0 1 0 0 66.7 66.7 0 0 <td>07:00 AM 2 0 0 2 1 3 1 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0</td> <td>07:00 AM 07:15 AM 07:30 AM 08:00 AM 2 0 0 2 1 3 1 5 0 0 0 0 0 1 2 0 0 2 6 1 3 10 0 0 1 1 0 1 0 0 0 0 3 2 2 7 0 0 0 0 0 1 4 0 0 4 14 8 7 29 0 0 3 3 0 4 100 0 0 48.3 27.6 24.1 0 0 100 0 100 500 .000 .000 .583 .667 .583 .725 .000 .000 .375 .000 1.000 4 0 0 4 13 8 7 28 0 0 2 2 0</td> <td>07:00 AM 07:15 AM 07:30 AM 08:00 AM 2 0 0 2 1 3 1 5 0 0 0 0 1 0 2 0 0 2 6 1 3 10 0 0 1 1 0 1 0 0 0 0 0 3 2 2 7 0 0 0 0 1 0 4 0 0 4 14 8 7 29 0 0 3 0 4 0 100 0 0 48.3 27.6 24.1 0 0 100 0 100 0 500 .000 .000 .583 .667 .583 .725 .000 .000 .375 .000 1,000 .000 4 0 0 4 13 8 7 28 0 <t< td=""></t<></td>	07:00 AM 2 0 0 2 1 3 1 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	07:00 AM 07:15 AM 07:30 AM 08:00 AM 2 0 0 2 1 3 1 5 0 0 0 0 0 1 2 0 0 2 6 1 3 10 0 0 1 1 0 1 0 0 0 0 3 2 2 7 0 0 0 0 0 1 4 0 0 4 14 8 7 29 0 0 3 3 0 4 100 0 0 48.3 27.6 24.1 0 0 100 0 100 500 .000 .000 .583 .667 .583 .725 .000 .000 .375 .000 1.000 4 0 0 4 13 8 7 28 0 0 2 2 0	07:00 AM 07:15 AM 07:30 AM 08:00 AM 2 0 0 2 1 3 1 5 0 0 0 0 1 0 2 0 0 2 6 1 3 10 0 0 1 1 0 1 0 0 0 0 0 3 2 2 7 0 0 0 0 1 0 4 0 0 4 14 8 7 29 0 0 3 0 4 0 100 0 0 48.3 27.6 24.1 0 0 100 0 100 0 500 .000 .000 .583 .667 .583 .725 .000 .000 .375 .000 1,000 .000 4 0 0 4 13 8 7 28 0 <t< td=""></t<>

N/S Street : Tarklin Place E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

					Grou	ps Printed-	Cars						
		arklin PI om North			haffee St rom East			arklin PI om South			haffee St om West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru !	Right	Left	Thru	Right	Int, Total
07:00 AM	2	0	0	0	3	0	0	0	0	0	0	0	5
07:15 AM	2	0	0	1	3	1	0	0	0	0	1	0	8
07:30 AM	0	0	0	5	1	3	0	0	0	0	0	0	9
07:45 AM	0	0	0	3	2	2	0	0	1	0	0	.0	8
Total	4	0	0	9	9	6	0	0	1	0	1	0	30
08:00 AM	0	0	0	4	2	1	0	0	0	0	1	0	8
08:15 AM	0	0	0	1	2	0	0	0	1	0	0	0	4
08:30 AM	1	0	0	2	8	0	0	0	0	0	1	0	12
08:45 AM	2	0	0	0	3	1	0	0	0	0	1	0	7
Total	3	0	0	7	15	2	0	0	1	0	3	0	31
Grand Total	7	0	0	16	24	8	0	0	2	0	4	0	61
Apprch %	100	0	0	33.3	50	16.7	0	0	100	0	100	0	
Total %	11.5	0	0	26,2	39.3	13,1	0	0	3.3	0	6.6	0	

1			lin PI				ffee St				klin Pl				ffee St		
		From	North			Fron	n East			From	South			Fron	n West		
Start Time	Left	Thru	Right	App Total	Left	Thru	Right	App. Total	Left	Thru	Right	App Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fror	n 07:00	AM to 0	08:45 AM	- Peak 1	of 1											
Peak Hour for Er	ntire Inte	rsection	Begins	at 07:15	AM												
07:15 AM	2	0	Ō	2	1	3	1	5	0	0	0	0	0	1	0	1	8
07:30 AM	0	0	0	0	5	1	3	9	0	0	0	0	0	0	0	0	9
07:45 AM	0	0	0	0	3	2	2	7	0	0	1	1	0	0	0	0	8
08:00 AM	0	0	0	0	4	2	- 1	7	0	0	0	0	0	1	0	1	8
Total Volume	2	0	0	2	13	8	7	28	0	0	1	1	0	2	0	2	33
% App. Total	100	0	0		46.4	28.6	25		0	0	100		0	100	0	_	-
PHF	.250	.000	.000	.250	.650	.667	.583	.778	.000	.000	.250	.250	.000	.500	.000	.500	.917

N/S Street : Tarklin Place E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

					Group	s Printed-	Trucks						
		arklin PI om North			haffee St rom East			arklin Pl om South			haffee St om West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	1	0	0	0	0	0	0	0	0	1
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	1	0	0]	0	0	0	0	0	0	1
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	0	0	1	0	1	0	2
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	ō	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	Ö
Total	0	0	0	0	0	0	0	0	1	0	1	0	2
Grand Total	0	0	0	1	0	0	0	0	1	0	1	0	3
Apprch %	0	0	0	100	0	0	0	0	100	0	100	0	
Total %	0	0	0	33.3	0	0	0	0	33.3	0	33.3	0	

		From	din PI North				fee St n East				klin Pl South				ffee St 1 West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App Total	Int. Total
Peak Hour Analy	sis Fror					of 1											
Peak Hour for Er	ntire Inte	ersection	n Begins	at 07:30	AM												
07:30 AM	0	0	_0	0	1	0	0	1	0	0	0	0	0	0	0	0	1
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	0	0	0	0	1	1	0	1	0	1	2
Total Volume	0	0	0	0	1	0	0	1	0	0	1	1	0	1	0	1	3
% App. Total	0	0	.0		100	0	0		0	0	100		0	100	0	·	
PHF	.000	.000	.000	.000	.250	.000	.000	.250	.000	.000	.250	.250	.000	.250	.000	.250	.375

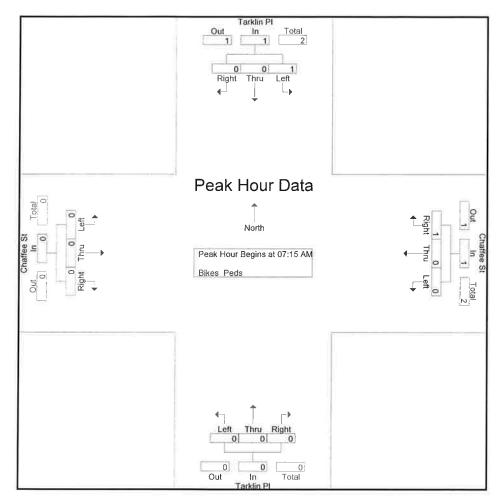
N/S Street : Tarklin Place E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

								Groups	Printed	l- Bikes	Peds								
			lin Pl North			Chaff From				Tark From	lin PI South			Chaff From	ee St West				
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu-Total	Inclu: Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	2	1	3
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	2	1	3
08:00 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	1	1	2
Grand Total	1	0	0	1	0	0	1	1	0	0	0	0	0	0	0	1	3	2	5
Apprch %	100	0	0		0	0	100		0	0	0		0	0	0				
Total %	50	0	0	1	0	0	50		0	0	0		0	0	0		60	40	

			din Pl North			- 1	ffee St n East				din PI South				ffee St West		
Start Time	Left	Thru	Right	App, Total :	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron	n 07:00	AM to 0	8:45 AM -	Peak 1	of 1						!==0:===				7//	
Peak Hour for Er	ntire Inte	rsection	n Begins	at 07:15 /	AM												
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	1
Total Volume	1	0	0	1	0	0	1	1	0	0	0	0	0	0	0	0	2
% App. Total	100	0	0	- **	0	0	100		0	0	0		0	0	0		
PHF	.250	.000	.000	.250	.000	.000	.250	.250	.000	.000	-000	.000	.000	.000	.000	.000	.500

N/S Street : Tarklin Place E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

File Name : 88070002 Site Code : 88070002 Start Date : 12/1/2020 Page No : 11



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour fo	Each Approach	Begins at:
--------------	---------------	------------

	07:00 AM				07:15 AM				07:00 AM				07:00 AM			
+0 mins	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0
Total Volume	1	0	0	1	0	0	1	1	0	0	0	0	0	Ö	0	0
% App. Total	100	0	0		0	0	100		0	0	0		0	0	0	
PHF	.250	.000	.000	250	.000	.000	.250	.250	.000	.000	.000	-000	.000	-000	.000	.000

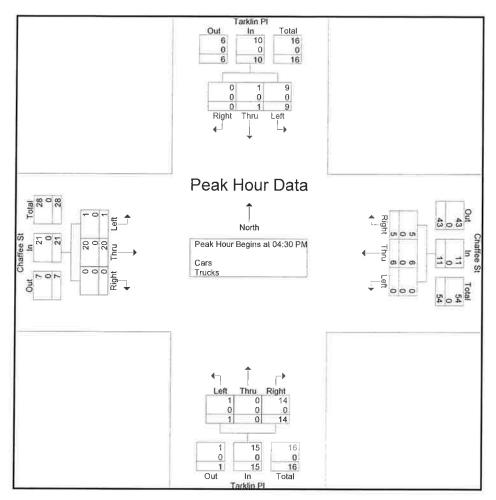
N/S Street : Tarklin Place E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

		haffee St	Ch		rklin Pl	Ta		naffee St	Cl		arklin Pl	Ta	
		om West			m South			om East			om North	Fro	
Int. Tota	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
	2	0	0	0	0	2	0	1	0	0	0	3	02:00 PM
	0	2	0	1	0	0	2	0	0	0	0	0	02:15 PM
1:	0	1	0	1	0	0	5	0	1	0	0	3	02:30 PM
1	0	0	0	3	0	0	2	111	2	0	0	3	02:45 PM
3:	2	3	0	5	0	2	9	2	3	0	0	9	Total
	0	2	0	0	0	0	1	0	1	0	0	1	03:00 PM
	0	0	0	0	0	0	2	0	0	0	0	0	03:15 PM
9	0	1	0	1	0	0	3	2	2	0	0	0	03:30 PM
- 1	0	2	1	1	0	0	1	4	0	0	0	2	03:45 PM
27	0	5	1	2	0	0	7	6	3	0	0	3	Total
1:	0	6	0	0	0	0	2	4	0	1	0	2	04:00 PM
10	0	4	0	2	0	0	3	0	1	0	0	0	04:15 PM
10	0	6	0	0	0	1	0	0	0	0	1	2	04:30 PM
16	0	4	0	- 1	0	0	1	5	0	0	0	5	04:45 PM
5	0	20	0	3	0	1	6	9	1	1	1	9	Total
20	0	8	1	7	0	0	2	1	0	0	0	1	05:00 PM
11	0	2	0	6	0	0	2	0	0	0	0	1	05:15 PM
6	0	0	0	0	0	0	1	1	0	0	0	4	05:30 PM
7	0	0	0	0	0	0	1	3	0	1	0	2	05:45 PM
44	0	10	1	13	0	0	6	5	0	1	0	8	Total
157	2	38	2	23	0	3	28	22	7	2	1	29	Grand Total
	4.8	90.5	4.8	88.5	0	11.5	49.1	38,6	12.3	6.2	3.1	90.6	Apprch %:
	1.3	24.2	1.3	14.6	0	1.9	17.8	14	4.5	1.3	0.6	18.5	Total %
153	2	37	2	22	0	3	27	22	7	2	1	28	Cars
97.5	100	97.4	100	95.7	0	100	96.4	100	100	100	100	96.6	% Cars
4	0	1	0	1	0	0	1	0	0	0	0	1	Trucks
2,5	0	2.6	0	4.3	0	0	3.6	0	0	0	0	3.4	% Trucks

			klin Pl North				fee St n East			1355	klin PI South						
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron	n 02:00	PM to 0	5:45 PM -	Peak 1	of 1											
Peak Hour for Er																	
04:30 PM	2	1	0	3	0	0	0	0	1	0	0	1	0	6	0	6	10
04:45 PM	5	0	0	5	0	5	1	6	0	0	1	1	0	4	0	4	16
05:00 PM	1	0	0	1	0	1	2	3	0	0	7	7	1	8	0	9	20
05:15 PM	1	0	0	1	0	- 0	2	2	0	0	6	6	0	2	0	2	11
Total Volume	9	1	0	10	0	6	5	11	1	0	14	15	1	20	0	21	57
% App. Total	90	10	0		0	54.5	45.5		6.7	0	93.3		4.8	95.2	0		
PHF	.450	.250	.000	.500	.000	.300	.625	.458	.250	.000	.500	.536	.250	.625	-000	.583	-713
Cars	9	1	0	10	0	6	5	11	1	0	14	15	1	20	0	21	57
% Cars	100	100	0	100	0	100	100	100	100	0	100	100	100	100	0	100	100
Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

N/S Street : Tarklin Place E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

File Name : 88070002 Site Code : 88070002 Start Date : 12/1/2020 Page No : 2



Peak Hour Analysis From 02:00 PM to 05:45 PM - Peak 1 of 1

	04:00 PM				03:30 PM				04:30 PM				04:15 PM					
+0 mins.	2	0	1	3	2	2	3	7	1	0	0	1	0	4	0	4		
+15 mins.	0	0	0	0	0	4	1	5	0	0	1	1	0	6	0	6		
+30 mins.	2	1	0	3	0	4	2	6	0	0	7	7	0	4	0	4		
+45 mins.	5	0_	0	5	1	0	3	4	0	0	6	6	1	8	0	9		
otal Volume	9	1	1	11	3	10	9	22	1	0	14	15	1	22	0	23		
6 App. Total	81.8	9.1	9.1		13.6	45.5	40.9		6.7	0	93.3		4.3	95.7	0			
PHF	450	.250	.250	.550	.375	.625	.750	.786	.250	.000	.500	.536	.250	.688	.000	.639		
Cars	9	1	1	11	3	10	9	22	1	0	14	15	1	22	0	23		
% Cars	100	100	100	100	100	100	100	100	100	0	100	100	100	100	0	100		
Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
% Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	(

N/S Street : Tarklin Place E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

		arklin Pl om North			haffee St om East	ps Printed-	Ta	arklin PI om South			haffee St om West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Tota
02:00 PM	3	0	0	0	1	0	2	0	0	0	0	2	8
02:15 PM	0	0	0	0	0	2	0	0	1	0	1	0	4
02:30 PM	3	0	0	1	0	4	0	0	1	0	1	0	10
02:45 PM	2	0	0	2	1	2	0	0	3	0	0	0	10
Total	8	0	0	3	2	8	2	0	5	0	2	2	32
03:00 PM	1	0	0	1	0	1	0	0	0	0	2	0	5
03:15 PM	0	0	0	0	0	2	0	0	0	0	0	0	2
03:30 PM	0	0	0	2	2	3	0	0	1	0	1	0	9
03:45 PM	2	0	0	0	4	1	0	0	1	1	2	0	11
Total	3	0	0	3	6	7	0	0	2	1	5	0	27
04:00 PM	2	0	1	0	4	2	0	0	0	0	6	0	15
04:15 PM	0	0	0	1	0	3	0	0	1	0	4	0	9
04:30 PM	2	1	0	0	0	0	1	0	0	0	6	0	10
04:45 PM	5	0	0	0	5	1	0	0	1	0	4	0	16
Total	9	1	1	1	9	6	1	0	2	0	20	0	50
05:00 PM	1	0	0	0	1	2	0	0	7	1	8	0	20
05:15 PM	1	0	0	0	0	2	0	0	6	0	2	0	11
05:30 PM	4	0	0	0	1	1	0	0	0	0	0	0	6
05:45 PM	2	0	1	0	3	1	0	0	0	0	0	0	7
Total	8	0	1	0	5	6	0	0	13	1	10	0	44
Grand Total	28	1	2	7	22	27	3	0	22	2	37	2	153
Apprch %	90.3	3.2	6.5	12.5	39.3	48.2	12	0	88	4.9	90.2	4.9	
Total %	18.3	0.7	1.3	4.6	14.4	17.6	2	0	14.4	1.3	24.2	1.3	

			din Pl North		Chaffee St From East					1727	din Pl South						
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Riaht	App. Total	Int: Total
Peak Hour Analy	sis Fror	n 02:00	PM to 0	05:45 PM -	Peak 1	of 1											
Peak Hour for Er	ntire Inte	rsection	n Begins	s at 04:30	PM												
04:30 PM	2	1	0	3	0	0	0	0	1	0	0	1	0	6	0	6	10
04:45 PM	5	0	0	5	0	5	1	6	0	0	1	1	0	4	0	4	16
05:00 PM	1	0	0	1	0	1	2	3	0	0	7	7	1	8	0	9	20
05:15 PM	1	0	0	1	0	0	2	2	0	0	6	6	0	2	0	2	11
Total Volume	9	1	0	10	0	6	. 5	11	1	0	14	15	1	20	0	21	57
% App. Total	90	10	0		0	54.5	45.5		6.7	0	93.3		4.8	95.2	0		
PHF	.450	.250	.000	.500	.000	.300	.625	.458	.250	.000	-500	.536	.250	.625	.000	.583	.713

Accurate Counts 978-664-2565

N/S Street: Tarklin Place E/W Street: Chaffee Street City/State: New Bedford, MA Weather: Cloudy

File Name : 88070002 Site Code : 88070002 Start Date : 12/1/2020 Page No : 7

		naffee St om West			arklin PI om South	Fro		naffee St om East	Fr		arklin PI om North	Fro	
Int. Tota	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
C	0	0	0	0	0	0	0	0	0	0	0	0	02:00 PM
1	0	1	0	0	0	0	0	0	0	0	0	0	02:15 PM
1	0	0	0	0	0	0	1	0	0	0	0	0	02:30 PM
1	0	0	0	0	0	0	0	0	0	0	0	1	02:45 PM
3	0	1	0	0	0	0	1	0	0	0	0	1	Total
0	0	0	0	0	0	0	0	0	0	0	0	0	03:00 PM
0	0	0	0	0	0	0	0	0	0	0	0	0	03:15 PM
0	0	0	0	0	.0	0	0	0	0	0	0	0	03:30 PM
.0	0	0	0	0	0	0	0	0	00	0	0	0	03:45 PM
0	0	0	0	0	0	0	0	0	0	0	0	0	Total
0	0	0	0	0	0	0	0	0	0	0	0	0	04:00 PM
1	0	0	0	1	0	0	0	0	0	0	0	0	04:15 PM
0	0	0	0	0	0	0	0	0	0	0	0	0	04:30 PM
0	0	0	0	0	0	- 0	0	0	0	0	0	0	04:45 PM
1	0	0	0	1	0	0	0	0	0	0	0	0	Total
0	0	0	0	0	0	0	0	0	0	0	0	0	05:00 PM
0	0	0	0	0	0	0	0	0	0	0	0	0	05:15 PM
0	0	0	0	0	0	0	0	0	0	0	0	0	05:30 PM
0	0	0	0	0	0	0	0	0	0	0	0	0	05:45 PM
0	0	0	0	0	0	0	0	0	0	0	0	0	Total
4	0	1	0	1	0	0	1	0	0	0	0	1	Grand Total
	0	100	0	100	0	0	100	0	0	0	0	100	Apprch %
	0	25	0	25	0	0	25	0	0	0	0	25	Total %

			din Pl North				fee St n East				klin PI South				fee St West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron	n 02:00	PM to 0)5:45 PM -	Peak 1	of 1											
Peak Hour for Er																	
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	ō	0	1	Ô	1	1
02:30 PM	0	0	0	0	0	0	1	1	0	0	0	ō	0	0	0	0	1
02:45 PM	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
Total Volume	1	0	0	1	0	0	1	1	0	0	0	0	0	1	0	1	3
% App. Total	100	0	0		0	0	100		0	0	0		0	100	0		
PHF	.250	-000	-000	.250	000	.000	250	.250	.000	.000	.000	.000	.000	.250	.000	.250	750

Accurate Counts 978-664-2565

N/S Street: Tarklin Place E/W Street: Chaffee Street City/State: New Bedford, MA Weather: Cloudy

File Name : 88070002 Site Code : 88070002 Start Date : 12/1/2020 Page No : 10

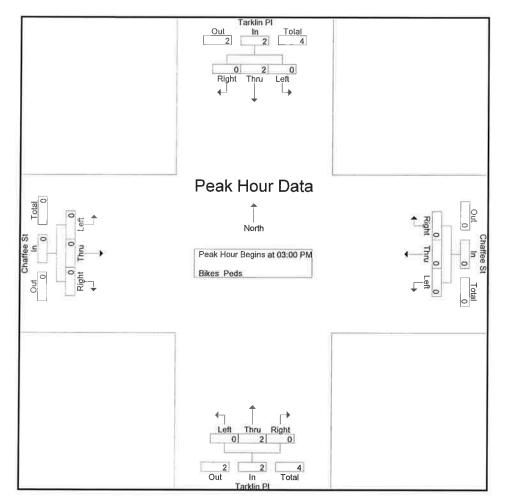
								Groups	Printed										
			lin Pl			Chaff				Tark				Chaff	ee St				
			North			From				From				From	West				
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu Total	Inclu. Total	Int. Total
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
02:30 PM	0	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	4	0	4
02:45 PM	0	0	0	0	0	0	0	0	.0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	3	0	0	0	2	0	0	0	0	0	0	0	0	5	0	5
03:00 PM	0	0	0	0	0	0	0	1	0	0	0	2	0	0	0	0	3	0	3
03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	2	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	4	4
Total	0	2	0	0	0	0	0	1	0	2	0	2	0	0	0	0	3	4	7
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	1
Total	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	1
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	1
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	1
Grand Total	0	2	0	3	0	0	0	4	0	2	0	3	0	0	0	0	10	4	14
Apprch %	0	100	0		0	0	0		0	100	0		0	0	0				
Total %	0	50	0	1	0	0	0	- 1	0	50	0		0	0	0		71.4	28.6	

		From	din PI North				fee St n East				klin PI South				fee St West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron	n 02:00	PM to 0)5:45 PM -	Peak 1	of 1	- 2				- 2						
Peak Hour for Er	ntire Inte	rsection	n Begins	s at 03:00	PM												
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	2	0	2	0	0	0	0	0	2	0	2	0	0	0	0	4
Total Volume	0	2	0	2	0	0	0	0	0	2	0	2	0	0	0	0	4
% App. Total	0	100	0		0	0	0	67.1	0	100	0		0	0	0		
PHF	.000	.250	.000	.250	.000	.000	.000	.000	.000	.250	.000	.250	.000	.000	.000	.000	.250

Accurate Counts 978-664-2565

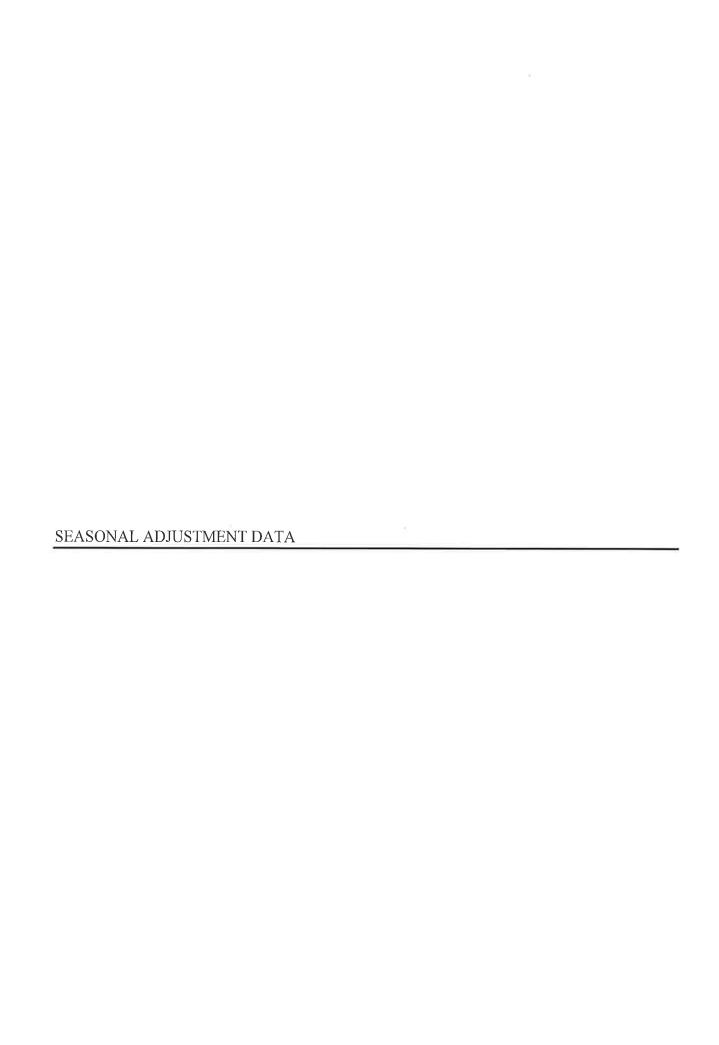
N/S Street : Tarklin Place E/W Street : Chaffee Street City/State : New Bedford, MA Weather : Cloudy

File Name \$88070002 Site Code : 88070002 Start Date : 12/1/2020 Page No : 11



Peak Hour Analysis From 02:00 PM to 05:45 PM - Peak 1 of 1

	03 00 PM				02:00 PM				03:00 PM				02:00 PM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	2	0	2	0	0	0	0	0	2	0	2	0	0	0	0
otal Volume	0	2	0	2	0	0	0	0	0	2	0	2	0	0	0	0
& App. Total	0	100	0		0	0	0		0	100	0		0	0	0	
PHF	.000	.250	.000	.250	.000	.000	.000	.000	.000	.250	.000	.250	.000	.000	.000	.000



Massachusetts Highway Department 38: Monthly Hourly Volume for December 2019

		QC Status	Accepted																														
		TOTAL	33173	48978	38196	55074	56922	58576	44765	35968	50847	54726	46960	56757	58829	41325	34905	55849	48536	56950	58929	61095	46557	38516	58995	50581	27693	47343	52088	42285	32668	45845	46548
		23:00	318	403	376	436	476	781	678	358	421	448	455	526	729	789	402	468	449	509	653	768	723	435	989	882	365	512	579	265	388	384	514
		22:00	402	554	558	725	869	1123	1080	553	583	889	728	771	1099	1001	550	674	673	761	1024	1087	1063	661	986	1404	705	989	894	950	527	580	654
		21:00	206	728	727	981	985	1338	1266	833	753	863	938	1075	1454	1237	794	950	828	1080	1214	1422	1370	1011	1267	1556	1021	1007	1259	1166	762	803	820
		20:00	654	1017	1002	1422	1469	1522	1291	1203	1054	1424	1342	1528	1605	1413	1208	1380	1220	1417	1742	1826	1578	1362	1701	1584	1490	1329	1579	1383	982	1139	1010
		19:00	937	1277	1268	1719	1817	1997	1664	1269	1460	1704	1571	1982	2075	1583	1394	1677	1514	1928	2028	2047	1591	1579	2187	1434	1743	1563	1749	1625	1256	1447	1334
		18:00	1242	2354	1944	2673	2871	2977	2391	1655	2322	2665	2460	2712	72977	2155	1873	2708	2489	2697	2899	3148	2176	2031	2880	2114	2002	2278	2564	2115	1812	2173	1988
		17:00	1475	3992	3294	4474	4525	4379	3027	2080	4037	4192	4182	4511	4478	7272	2321	4574	4087	4555	4549	4545	2658	2533	4335	2881	2131	3403	3859	2772	2194	3592	2675
reast	reast	16:00	2071	4454	3749	4852	5233	4991	3320	2861	4454	4887	4374	2066	4978	3106	2788	4957	4577	5115	4687	4946	3281	3122	5034	3120	2085	4104	4565	3126	2661	4091	3530
U1-Southeast	U1-Southeast	15:00	2661	4024	3334	4591	4584	4844	3403	3147	4244	4416	3994	4609	4918	3240	2713	4666	4223	4760	4770	4912	3584	3233	4911	3552	2027	4189	4112	3447	2484	3887	4131
	ם	14:00	2992	3522	2731	3728	3947	4305	3344	3132	3511	3901	3293	3821	4168	3261	2692	3895	3385	3962	4067	4333	3538	3239	4391	3888	2141	3774	4089	3365	2697	3473	4042
Seasonal Factor Group: Daily Factor Group:	:dno.g	13:00	3219	2843	2532	3224	3310	3558	3304	3185	3079	3163	2581	3368	3626	3175	2969	3317	2712	3398	3387	3583	3579	3331	3970	4045	2419	3383	3726	3681	2806	3113	3452
Factor tor Gro	tor Gro Factor (12:00	3277	2923	2039	3167	3306	3430	3576	3067	3006	3226	2359	3283	3463	3395	3270	3324	2682	3269	3399	3581	3834	3449	3964	4152	2684	3521	3775	3501	3182	3057	3586
Seasonal Factor Gro Daily Factor Group:	Axle Factor Group: Growth Factor Group	11:00	3270	2700	2013	2913	3072	3388	3531	3140	2892	3069	2122	3275	3468	3375	2982	3245	2600	3234	3362	3454	3744	3318	3881	3700	2102	3280	3537	3369	2805	3006	3155
SO	₹ Ø	10:00	2744	2651	2073	2924	2997	3228	3096	2596	2866	2939	2222	3135	3280	2736	2584	3213	2289	3074	3216	3277	3427	2640	3722	3400	1418	2885	3163	3014	2355	2690	3019
		9:00	2345	2810	2241	3205	3266	3062	2813	2087	2956	3216	2455	3298	3236	2372	2053	3068	2713	3256	3256	3329	2883	2119	3390	3016	1087	2598	2930	2479	1925	2543	2636
		8:00	1592	3780	2551	4174	4254	4029	2144	1493	4063	4084	3300	4102	3928	1857	1367	3939	3701	4287	4462	4246	2296	1487	3514	2924	634	2578	2843	1822	1139	2984	2967
		7:00	1200	4266	2525	4660	4596	4472	1780	1157	4189	4603	3889	4467	4310	1482	932	4684	3721	4532	4681	4477	1990	984	3707	2890	427	2723	2867	1487	857	3039	3141
		00:9	969	2666	1687	2885	3053	2843	1252	715	2812	3044	2491	2926	2775	865	612	2919	2465	2910	3134	2955	1220	909	2405	2057	309	1920	2018	916	558	2071	2100
	ın	5:00	351	1122	826	1273	1248	1224	655	436	1189	1236	1146	1252	1173	427	310	1210	1070	1186	1331	1445	652	286	1101	913	115	800	888	406	280	881	875
	1 INTERSTATE 195	4:00	193	375	324	408	423	402	227	197	408	394	417	395	397	213	126	429	433	424	421	563	323	163	349	323	99	305	341	211	147	323	326
38 Bristol	L NTERST	3:00	143	151	193	172	165	149	163	86	143	148	170	164	159	115	100	141	164	138	154	228	226	123	141	151	47	112	176	148	105	141	141
	~ <u>-</u>	2:00	177	77	127	80	117	103	149	148	96	82	113	95	110	168	194	50	129	102	109	173	191	138	108	122	80	94	120	153	149	93	95
	SSI	1:00	267	115	128	138	118	140	232	204	122	125	128	135	152	215	237	120	137	123	144	322	218	258	150	173	201	132	191	222	231	104	138
Location ID: County:	Funcationl Class Location:	0:00		174	194	242	221	291	379	354	187	206	1 230	2 264	3 271	4 418	5 434	206	7 275	8 233	9 240	0 428	1 412	408	3 265	300	373	5 167	7 264	335	998 6	231	1 219
ÕĚ	Ē Š		-	7	m	4	S	Ф	7	00	σ	10	7	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31

52,820

Yearly Average =

1.102

Adjustment Factor =

47,951

Monthly Average =

Massachusetts Highway Department 38: Monthly Hourly Volume for December 2020

	QC Status	Accepted	Accepted	Accepted	Accepted	Accepted	Accepted	Accepted	Accepted	Accepted	Accepted	Accepted		Accepted	Accepted	Accepted	Accepted													
	TOTAL	48873	50413	50914	29775	30972	48047	49478	50965	50213	51800	36665	31313	48199	53418	52689	17846	51256	42215	32734	52253	57766		43437	17190	34228	30139		42.688	1
	23:00	416	394	438	335	360	409	434	453	421	439	394	275	434	463	331	272	515	450	327	438	2260		488	367	396	294			
	22:00	588	584	647	499	464	503	635	598	662	788	595	444	565	575	459	386	780	724	513	611	2144		711	464	554	512		verage =	0
	21:00	802	804	945	648	689	721	837	832	862	1073	854	629	748	880	629	382	1185	957	694	838	1740		834	691	878	683		Monthly Average =	
	20:00	1047	1153	1210	739	1048	978	1084	1263	1154	1178	905	805	1025	1213	1087	521	1367	1163	995	1174	1547		1012	792	1038	921		_	
	19:00	1438	1603	1488	987	1156	1350	1417	1534	1591	1770	1177	1168	1452	1807	1648	269	1652	1472	1233	1688	1881		1150	994	1326	1223			
	18:00	2161	2370	2350	1222	1449	2183	2280	2413	2473	2582	1587	1498	2047	2622	2473	958	2606	2116	1673	2572	2676		1579	1146	1698	1728			
	17:00	3656	3708	3672	1638	2055	3497	3716	3575	3803	3710	2224	2059	3371	3958	3961	1438	3940	2791	2209	3906	4015		2229	1157	2264	2057			
U1-Southeast U1-Southeast	16:00	4378	4459	4477	2040	2367	4270	4497	4460	4331	4512	2489	2534	4035	4773	4713	1794	4347	3189	2500	4536	4608		2815	1342	2688	2298			
U1-Sou U1-Sou	15:00		4260	4561	2204	2678	4102	4211	4383	4185	4463	2821	2786	4246	4451	4815	1648	4508	3358	2703	4581	4148		3399	1405	2814	2573			
<u> </u>	14:00		3671	4040	2293	2751	3338	3506	3611	3597	3821	2955	2915	3471	3795	4019	1498	3808	3383	2903	3922	3962		3541	1413	2833	2688			
or Grou roup: ·oup: r Group	13:00		3033	3212	2312	2771	2972	3054	3182	3079	3349	2941	3008	3059	3245	3244	1131	3348	3447	2821	3440	3588		3824	1449	3124	2680			
Seasonal Factor Group: Daily Factor Group: Axle Factor Group: Growth Factor Group:	12:00		3030	3160	2583	2886	2882	2860	3098	2883	3130	3172	2956	3014	3147	3255	1120	3318	3488	3117	3361	3440		3876	1551	2993	2692			
Seasor Daily F Axle Fa Growt	11:00		2830	3018	2585	2644	2707	2707	3142	2796	2979	3226	2678	2876	3016	3111	686	3118	3526	2858	3174	3216		3551	1213	2979	2466			
	10:00	2666	2746	2875	2369	2219	2752	2722	2927	2725	2997	2896	2252	2808	2980	3037	855	2944	3130	2437	3123	3055		3169	933	2499	2078			
	9:00	2689	2748	2714	1989	1725	2686	2703	2903	2736	2803	2363	1713	2730	7 2962	3100	816	2722	2574	1810	2994	3069		2737	672	1912	1630			
) 8:00 9 3194	3282	3455	3326	1682	1254	3460	3610	3375	3517	3475	1994	1202	3447	3797	3503	858	3217	3 2032	1305	3530	3559		2528	457	1479	1157			
	7:00	3906	5 4133	3795	1514	964	4019	4177	3862	4099	3803	1554	933	3845	4247	4018	824	3363	1613	982	3672	3822		2498	340	1121	893			
	0 6:00	3028	3135	5 2850	3 925	1 615	3029	5 2909	3 2955	3075	5 2752	1128	566	2929	3059	3037	629	5 2513	1199	652	, 2722	1 2804		1716	246	670	622			
195	0 5:00 5 1133	1276) 1307	3 1176	1 453	301	1 1273	2 1225	3 1198	3 1180	5 1236	7 552	3 256	1272	2 1312	1217	312	5 1136	586	9 279	3 1127	1204		815	3 121	339	3 298			
38 Bristol 1 INTERSTATE 195	0 4:00	5 384	3 400	383	3 181	1 130	1 361	1 342	393	1 363	355	3 227	3 128	364	3 472	391	160	3 316	3 235	7 159	353	391		5 285	1 43	178	3 163			
38 Bristol 1 INTER	0 3:00 3 120	5 115	1 148	3 156	5 123	7 81	7 164	8 144	3 240) 154	9 144	9 133	1 78	3 129	5 183	5 169	2 121	3 203	0 173	3 117	136	140		1 126	1 34	2 75	1 98			
	0 2:00 6 103	2 95	6 91	7 98	5 106	3 77	6 107	3 108	4 128	7 130	68 6	5 109	4 81	0 83	3 126	3 105	5 92	88	7 140	5 103	08 €	1 141		1 104	0 64	1 92	1 91			
): Class	0 1:00 5 116	0 112	5 126	6 107	3 135	5 103	8 116	7 113	6 184	0 147	3 149	6 156	5 134	06 6	2 123	4 123	9 136	8 104	771 2	8 126	68 9	2 121		9 161	9 80	4 84	0 104			
Location ID: County: Funcationl Class Location:	0:00	2 200	3 225		5 213	6 185	7 168	8 187	9 256	10 250	11 203	12 216	13 185	14 159	15 212	16 214	17 209	18 158	19 292	20 218	21 186	22 202	23	24 289	25 216	26 194	27 190	28	29	30



Accurate Counts 978-664-2565

	ממנטותי ונונו														8807SP01
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Time	15	20	25	30	35	40	45	50	55	90	55	70	75	000	T.
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Total	27	176	267	1460	1212	320	41	4	2	0	0	o	0	0	C
Daily		15th	15th Percentile	23 M	PH										
		50th	50th Percentile	28 M	Н										
		85t	85th Percentile	34 M.	HH										
		951	95th Percentile :	37 MPH	РН										
		Mean Spee	Mean Speed(Average)	29 MPH	PH										
		TO MPH P	10 MPH Pace Speed : Number in Pace :	26-35 MPH 2672	MPH 2672										
	Ž	Percent in Pace Number of Vehicles > 30 MPH	Percent in Pace	70.1%	70.1%										
	O C	Percent of Vehicles > 30 MPH	- 30 MPH	41.5%	701										

Accurate Counts 978-664-2565

Location: Church Street
Location: South of Chaffee Street
City/State: New Bedford, MA
SB

		Ì	otal	23	F	13	7	10	40	135	302	200	000	245	240	2/4	187	278	267	400	378	265	175	142	88	58	29	45	3895						7704				
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		20		o c) -	- c	0 0	> (7	2	9	o j	12	13	13	7	11	80	14	15	14	· LO) [2 1			- 0		151	15th Percentile. 50th Percentile. 85th Percentile. 95th Percentile.		Mean Speed(Average) 10 MPH Pace Speed Number in Pace	Number of Vehicles > 30 MPH Percent of Vehicles > 30 MPH		327 11	15th Percentile: 50th Percentile: 85th Percentile: 95th Percentile:	Mean Speed(Average)	10 MPH Pace Speed Number in Pace Percent in Pace Number of Vehicles > 30 MPH	Percent of Vehicles > 30 N
		15			٠ -												2	3					10				0 0					10	Number of V		55 37		Mea	Number of V	Percent of V
			0	. (, ,		5 1	o ′	o '	o ,	5	0	0	7	0	0	0											<i>></i>						=			
SB	Start	lime	12/02/20	01:00	00.00	03:00	00:00	00.40	0000	06:00	00:70	08:00	00:60	10:00	11:00	12 PM	13:00	14:00	15:00	16:00	17:00	18:00	19:00	20:00	21.00	00:00	22:00	7.07 5.07	lotal	Daily					Grand Total	Overall			

Accurate Counts 978-664-2565

Location: Church Street Location: South of Chaffee Street City/State: New Bedford, MA

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		4	_	

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Accurate Counts 978-664-2565

Location: Church Street
Location: South of Chaffee Street
City/State: New Bedford, MA
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Accurate Counts 978-664-2565

Location: Church Street
Location: South of Chaffee Street
City/State: New Bedford, MA
SB, NB

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Accurate Counts 978-664-2565

Location: Church Street
Location: South of Chaffee Street
City/State: New Bedford, MA
SB, NB

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Accurate Counts 978-664-2565

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Accurate Counts 978-664-2565

Location: Tarkiln Hill Place Location: South of Chaffee Street City/State: New Bedford, MA NB

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Accurate Counts 978-664-2565

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37.		Numk	ver of Vehicles	> 20 MPH	26.27	• ^										
		Perce	ant of Vehicles	> 20 MPH	37.0%											

Accurate Counts 978-664-2565

Location: Tarkiln Hill Place Location: South of Chaffee Street City/State: New Bedford, MA SB

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Accurate Counts 978-664-2565

Location: Tarkiln Hill Place
Location: South of Chaffee Street
City/State: New Bedford, MA
NB: SB

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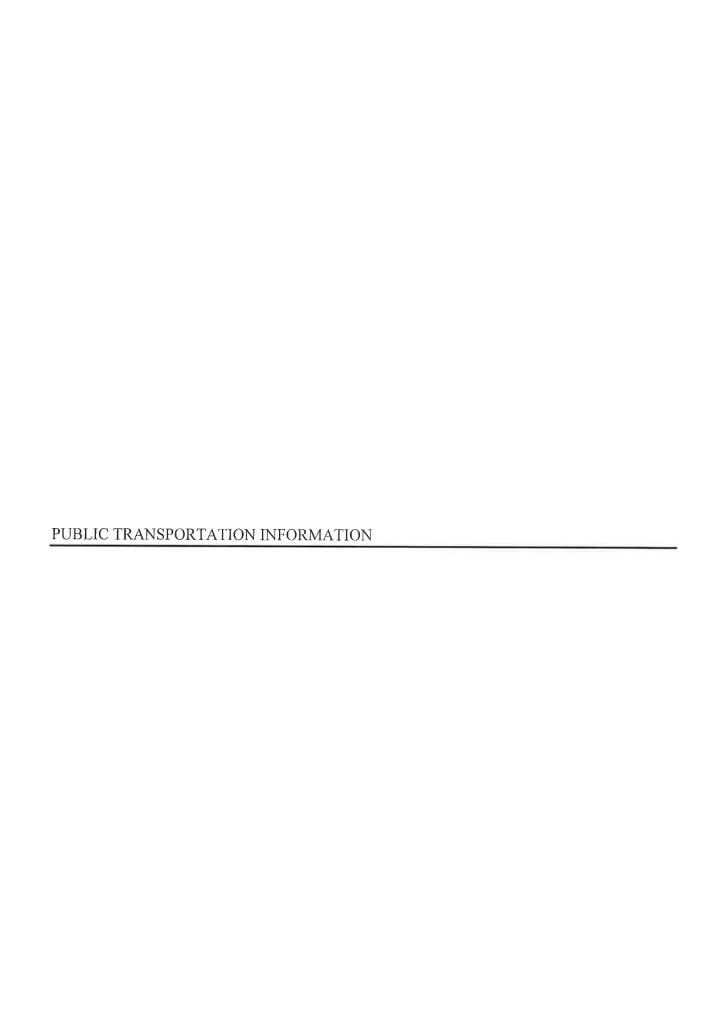
Total

Accurate Counts 978-664-2565

Location : Tarkiln Hill Place Location : South of Chaffee Street City/State: New Bedford, MA

04 60 0 0 0 0 16 35 0000004--480884-8000000 28 4 23 10 MPH 18 MPH 22 MPH 25 MPH 17 MPH 15-24 MPH 70 67,3% 33 17 MPH 15-24 MPH 142 67.3% 71 33.8% 10 MPH 18 MPH 22 MPH 25 MPH 12 Mean Speed(Average):
10 MPH Pace Speed:
Number in Pace:
Percent in Pace:
Number of Vehicles > 20 MPH:
Percent of Vehicles > 20 MPH: Mean Speed(Average)
10 MPH Pace Speed
Number in Pace
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Number of Vehicles > 20 MPH
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211



	1	2	3	4	5
	Industrial	Trucchi's	Ashley Blvd. at	Ashley Blvd. at	SRTA
	Park		Tarkiln Hill Rd.	Holly St.	Terminal
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		7:40	7:43	7:48	8:00
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		9:10	9:13	9:18	9:30
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		3:40	3:43	3:48	4:00
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		5:10	5:13	5:18	5:30
		5:40	5:43	5:48	6:00

Please note that schedule times are approximate.

Operated by SouthCoast Transit Management, Inc.

134Elm Street New Bedford, MA 02740 508-999-5211 www.srtabus.com

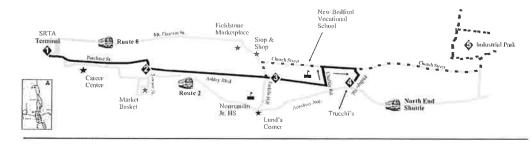


August 24, 2020



Southeastern Regional Transit Authority

Route 4
Ashley Blvd.



	1	2	3	4	5
	SRTA	Ashley Blvd. at	Ashley Blvd. at	Industrial	Trucchi's
	Terminal		Daniel St.		Trucciii 3
Market III	reminal	Sawyer St.		Park	
			WEEKDAYS		
AM	5:40	5:47	5:55	6:10	
	6:05	6:12	6:20	6:35	
	6:50	6:57	7:05		7:13
	7:15	7:22	7:30		7:38
-	7:45	7:52	8:00		8:08
	8:15	8:22	8:30		8:38
	8:45	8:52	9:00		9:08
	9:15	9:22	9:30		9:38
	9:45	9:52	10:00		10:08
	10:15	10:22	10:30		10:38
	10:45	10:52	11:00		11:08
	11:15	11:22	11:30		11:38
	11:45	11:52	12:00		12:08
PM	12:15	12:22	12:30		12:38
	12:45	12:52	1:00		1:08
	1:15	1:22	1:30		1:38
-	1:45	1:52	2:00		2:08
	2:15	2:22	2:30		2:38
	2:45	2:52	3:00		3:08
	3:15	3:22	3:30		3:38
	3:45	3:52	4:00		4:08
	4:15	4:22	4:30		4:38
	4:45	4:52	5:00		5:08
	5:15	5:22	5:30		5:38
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h			SATURDAY	NAME OF STREET	it is a thoughtour t
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	8:45	8:52	9:00		9:08
	9:15	9:22	9:30		9:38
	9:45	9:52	10:00		10:08
	10:15	10:22	10:30		10:38
	10:45	10:52	11:00		11:08
	11:15	11:22	11:30		11:38
	11:45	11:52	12:00		12:08
PM	12:15	12:22	12:30		12:38
	12:45	12:52	1:00		1:08
	1:15	1:22	1:30		1:38
	1:45	1:52	2:00		2:08
	2:15	14:22	2:30		2:38
	2:45	14:52	3:00		3:08
	3:15	3:22	3:30		3:38
	3:45	3:52	4:00		4:08
	4:15	4:22	4:30		4:38
	4:45	4:52	5:00		5:08
	5:15	5:22	5:30		5:38

Please note that schedule times are approximate.

6-11 Yrs. Old—50.75 per trip Senior Citizens—50.75 per trip

accompanied by adult (Limit 2

children per adult)

Children under 6-Free when

Regular Fare—\$1,50 per trip

FARES

Registered Disabled—\$0,75 per trip Medicare Recipients—\$0.75 per trip **Charlie Card Fares**

Reduced Fare—\$0.70 per trip

All buses are

Regular Fare—\$1.40 per trip

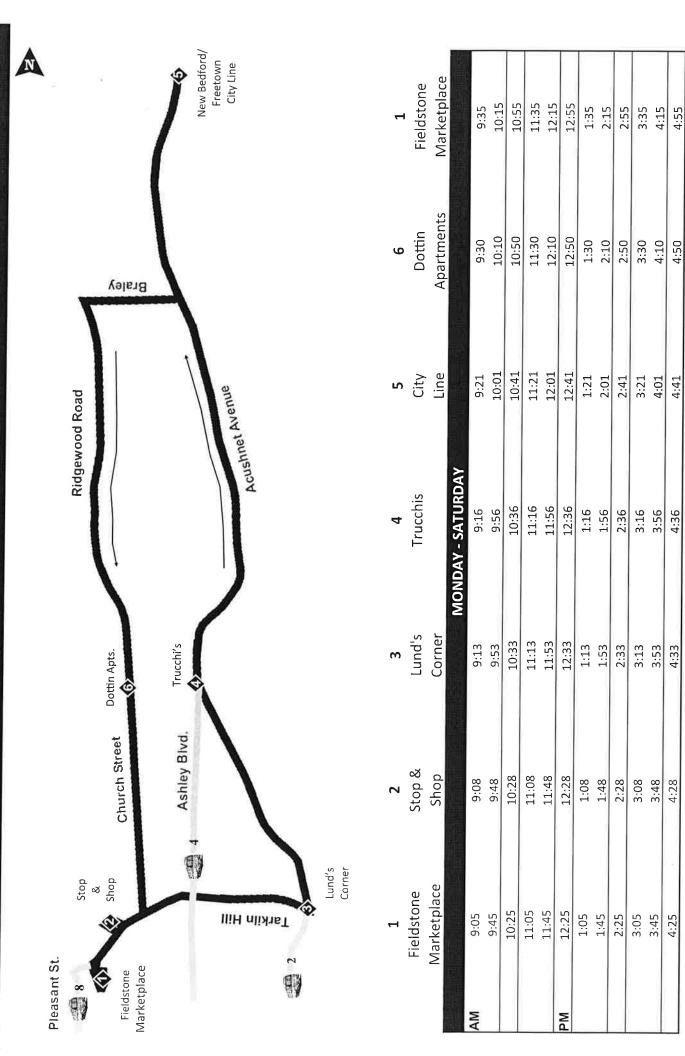
wheelchair accessible. **NO SUNDAY SERVICE**

AOKING: Smoking is prohibited on all buses.

The consuming of food or beverage of any kind on the bus is prohibited. Serious injury may occur when trash is left aboard the vehicle, **JOD AND BEVERAGES:**

BJECTIONABLE PERSONS:

him or herself, or whose conduct is such or likely to be such as to make who is incapable of taking care of him or her objectionable to other influence of alcohol or drugs, or This Authority and its Operator reserve the right to refuse to transport a person under the passengers or prospective



Please note that schedule times are approximate.

FARES

- Regular Fare—\$1.50 per trip
 Children under 6—Free when
 accompanied by adult (Limit 2
 children per adult)
 - 6-11 Yrs. Old—\$0.75 per trip
- Senior Citizens—\$0.75 per trip
- Registered Disabled—\$0.75 per trip
- Medicare Recipients—\$0.75 per trip

Charlie Card Fares

- Regular Fare—\$1.40 per trip
- Reduced Fare—\$0.70 per trip

All buses are wheelchair accessible.

NO SUNDAY SERVICE

iMOKING: Smoking is prohibited on all ouses.

FOOD AND BEVERAGES:

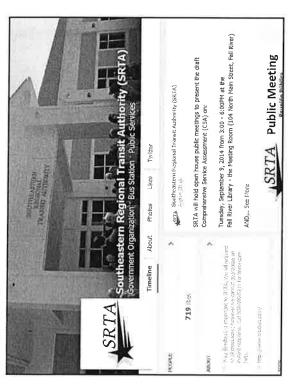
The consuming of food or beverage of any kind on the bus is prohibited.
Serious injury may occur when trash is left aboard the vehicle.

OBJECTIONABLE PERSONS:

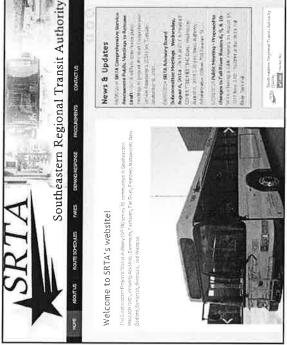
This Authority and its Operator reserve the right to refuse to transport a person under the influence of alcohol or drugs, or who is incapable of taking care of him or herself, or whose conduct is such or likely to be such as to make him or her objectionable to other passengers or prospective passengers.



LIKE US ON FACEBOOK!



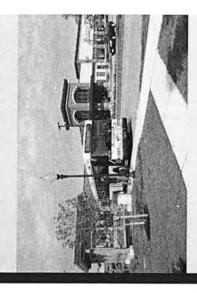
Visit our website: WWW.SRTABUS.COM





North End Shuttle

Southeastern Regional Transit Authority



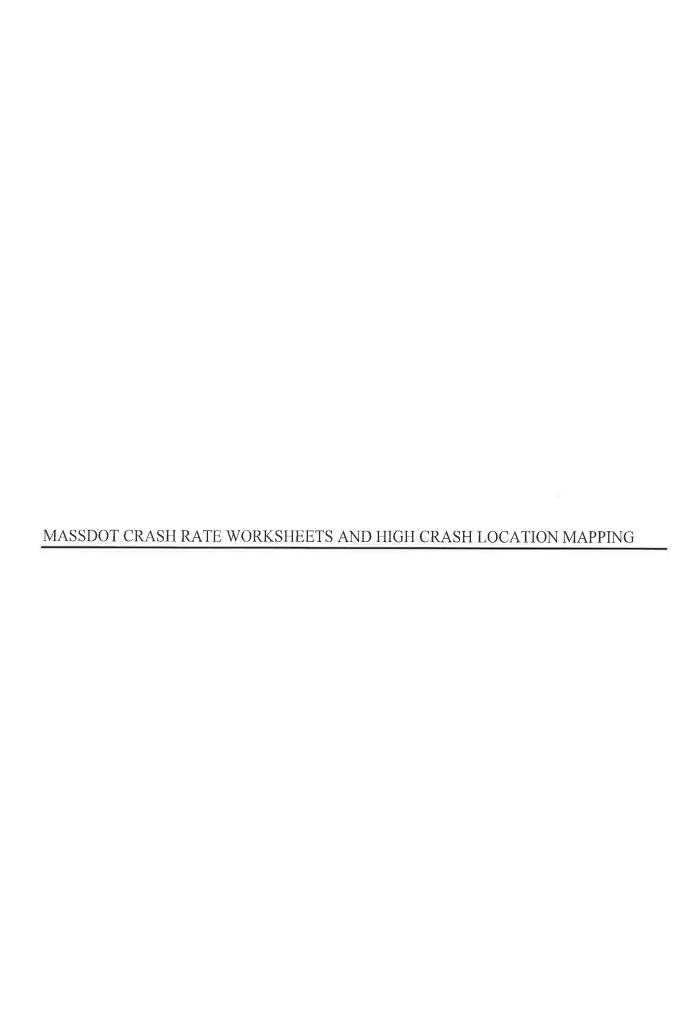
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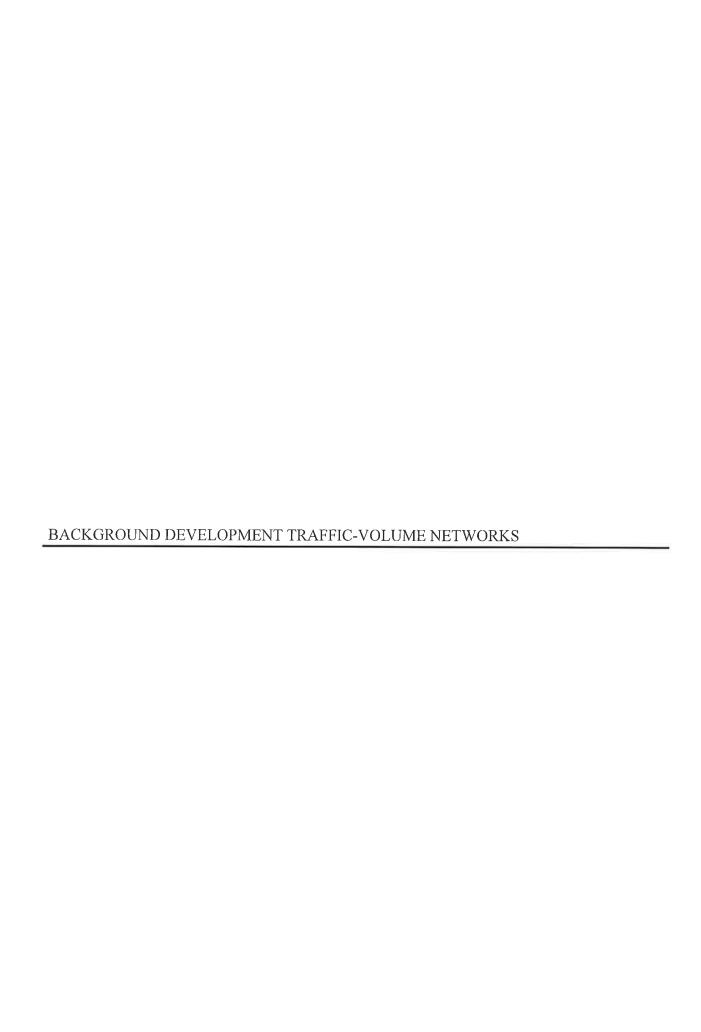
Operated by SouthCoast Transit Management, Inc.





INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN:	New Bedford	db	· ·	COUNT DA	TE:	Dec-20
DISTRICT: 5	UNSIGN	IALIZED:	Х	SIGNA	ALIZED :	
		~ IN	TERSECTIO	N DATA ~		
MAJOR STREET :	Church Stree	et			- 111	
MINOR STREET(S):	Chaffee Stre	et				
INTERSECTION DIAGRAM (Label Approaches)	North Chaff	el	Church stract		street	£0
ADDDOAGU			PEAK HOUF			Total Peak
APPROACH:	1	2	3	4	5	Hourly
DIRECTION:	EB	WB	NB	SB		Approach Volume
PEAK HOURLY VOLUMES (PM)	21	119	513	521		1,174
"K" FACTOR:	0.090	INTERS	ECTION ADT APPROACH	, ,	AL DAILY [13,044
TOTAL # OF CRASHES	7	# OF YEARS :	5	CRASHES	GE#OF PER YEAR (.):	1.40
CRASH RATE CALCU	LATION:	0.29	RATE =	(A * 1,0	000,000) * 365)	
Comments :	Below MassE	OT District 5	crash rate			
Project Title & Date:	Proposed Me	dical Office -	December 20)20		



R:\8807\8807NT1.dwg, 1/4/2021 2:36:01 PM, HP Color LaserJet M750 PCL 6.pc3

WEEKDAY MORNING PEAK HOUR (7:00 - 8:00 AM)



WEEKDAY EVENING PEAK HOUR (3:00 - 4:00 PM)





Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale



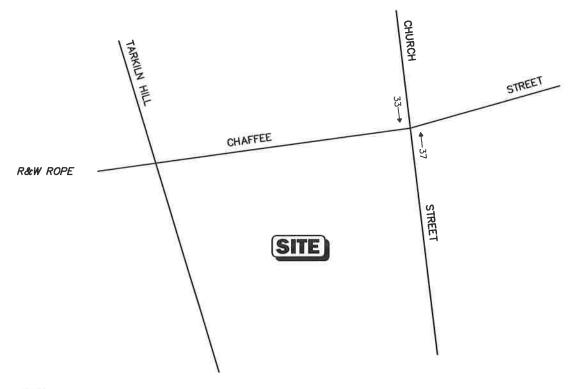
Figure A-1

U-Haul Moving and Storage Facility 429 Church Street Peak Hour Traffic Volumes

WEEKDAY MORNING PEAK HOUR (7:00 - 8:00 AM)



WEEKDAY EVENING PEAK HOUR (3:00 - 4:00 PM)





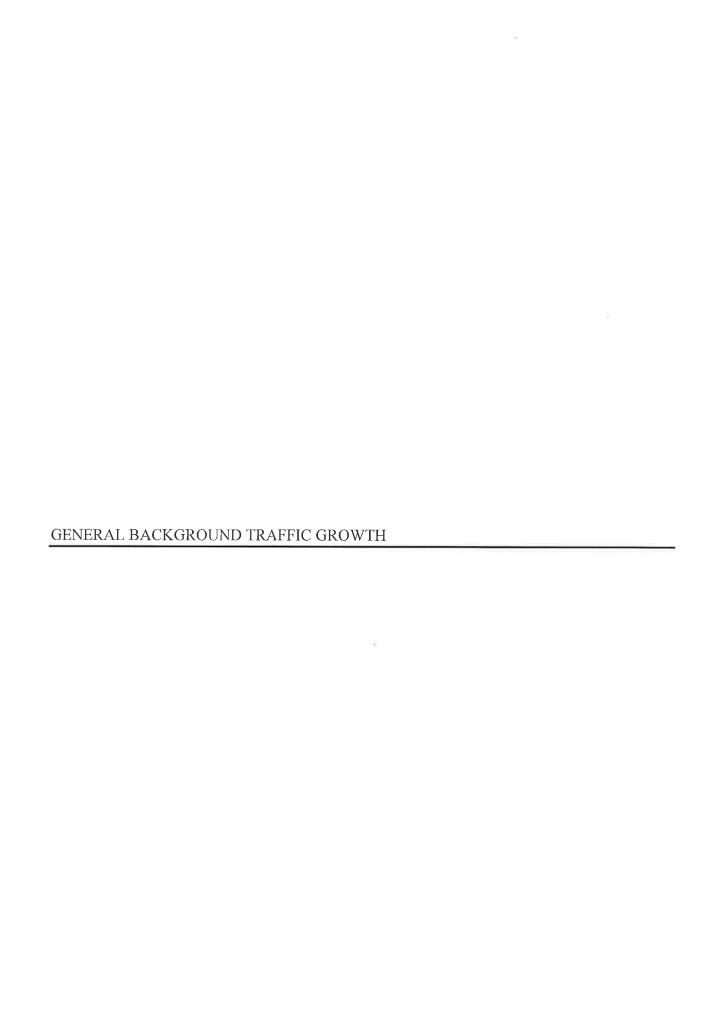
Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale

Figure A-2



Proposed Alma Del Mar II Charter School 739 Church Street Peak Hour Traffic Volumes



Proposed Medical Office - New Bedford, MA

General Background Traffic Growth

46.908 48.639 47,430 50,371 52,000 57,728 5	Fairhaven 1-195 East of Acushnet River 55,200 48,956 46,908 48,639 47,430 50,371 52,000 5	STA.	CITY/TOWN	ROUTE/STREET	LOCATION	2010	2011	2012	2013	2014	2015	2016	2017	2018	3010	Average Annua
THIND EAST OF ACUSTMENT KIVET 55,200 48,956 46,908 48,639 47,430 50,371 52,000 57,728 5	55,200	35	Fortharcan		F C 4 1 . m:								404	2010	7107	CIOWIII NAIC
07/10 0000 1000 0011 0000		2	t antimaxen	1-173	East of Acushnet Kiver	55.200		48.956	46 908	48 639	47 430	50371	52 000	57 778	62 620	1 000%
												2000	74,000	21.120	72,020	0/00-1



Medical-Dental Office Building

(720)

Vehicle Trip Ends vs: Employees On a: Weekday

> Setting/Location: General Urban/Suburban

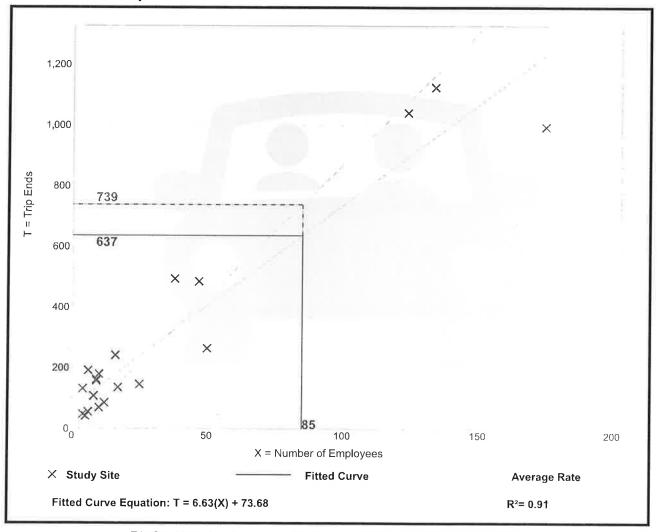
Number of Studies: 20 Avg. Num. of Employees:

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Employee

Average Rate	Range of Rates	Standard Deviation
8.70	5.32 - 33.00	4.25

Data Plot and Equation



Medical-Dental Office Building

(720)

Vehicle Trip Ends vs: **Employees**

> On a: Weekday,

> > Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

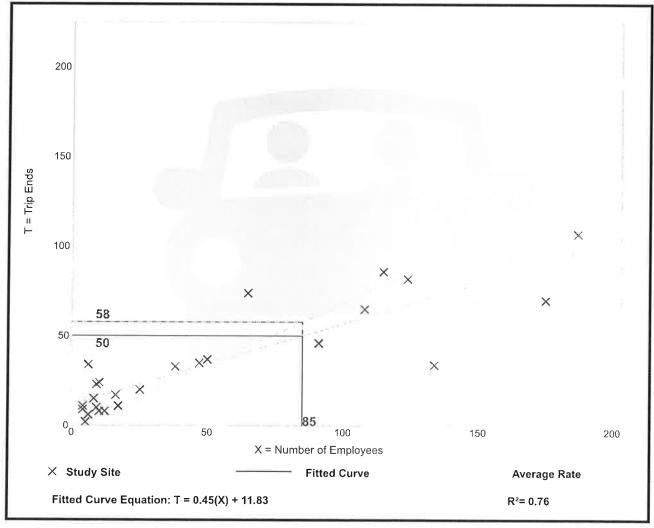
Number of Studies: 25 Avg. Num. of Employees: 51

Directional Distribution: 78% entering, 22% exiting

Vehicle Trip Generation per Employee

Average Rate	Range of Rates	Standard Deviation
0.68	0.25 - 5.67	0.51

Data Plot and Equation



Medical-Dental Office Building

(720)

Vehicle Trip Ends vs: **Employees**

> On a: Weekday,

> > Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

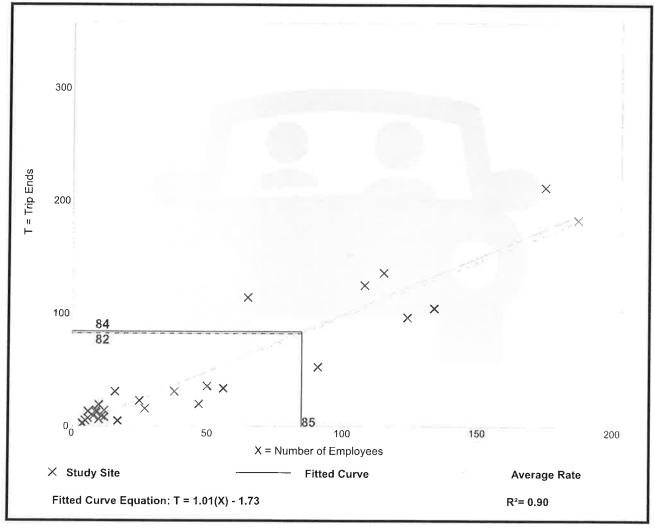
Number of Studies: 29 Avg. Num. of Employees: 48

Directional Distribution: 34% entering, 66% exiting

Vehicle Trip Generation per Employee

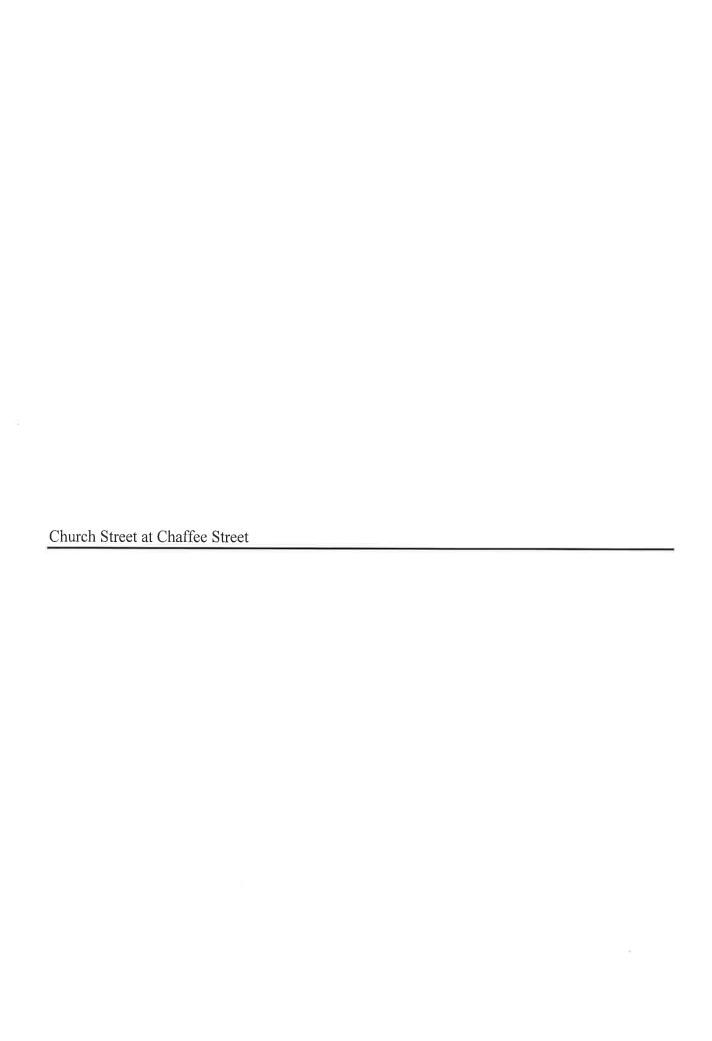
Average Rate	Range of Rates	Standard Deviation
0.97	0.29 - 2.17	0.35

Data Plot and Equation



CAPACITY ANALYSIS WORKSHEETS

Church Street at Chaffee Street
Tarkiln Hill Place at Chaffee Street
Church Street at the Project Site Driveway
Chaffee Street at the Project Site Driveway
Tarkiln Hill Place at the Project Site Driveway



Intersection	5.1	SHIPPING	0.025	Of Call Street		AHERER.	A BUILDING		- 41-14	10 SQ115 (1)	THE PERSON NAMED IN	28-21-11	D WILES	OLESSE.	0.028
Int Delay, s/veh	5.1														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		10 GO GO	
Lane Configurations		4			4			4			4				
Traffic Vol, veh/h	2		4	29	9	71	8	170	27	116	333	17			
Future Vol, veh/h	2	1	4	29	9	71	8	170	27	116	333	17			
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0			
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free			
RT Channelized		*	None		III OII	None	-	1 3	None	57.0		None			
Storage Length				=	- 5	5	-					-			
Veh in Median Storage	,# -	0			0		1135.15	0	-		0				
Grade, %	:*)	0			0	-		0	· ·	340	0	-			
Peak Hour Factor	44	44	44	65	65	65	73	73	73	77	77	77			
Heavy Vehicles, %	0	0	0	8	0	4	0	4	0	1	13	7			
Mvmt Flow	5	2	9	45	14	109	11	233	37	151	432	22			
Major/Minor	Minor2	110,050		Minort	9317/1900 E	0.01810.0	Majord	CR (620)		Mair-0				OF STREET	
		1007		Minor1	4000		Major1			Major2	Triffe Pin	SATAL STATE	All OF ST	15 - 15	With a control
Conflicting Flow All	1080	1037	443	1025	1030	252	454	0	0	270	0	0			
Stage 1	745	745	A .	274	274	199			17 7		*				
Stage 2	335	292		751	756			(∰)		2-22					
Critical Hdwy	7.1	6.5	6.2	7.18	6.5	6.24	4.1	(4)		4.11					
Critical Hdwy Stg 1	6.1	5.5		6.18	5.5	:4:	•	-	-	-	2	-			
Critical Hdwy Stg 2	6.1	5.5	-	6.18	5.5			× 2	-			-L - k			
Follow-up Hdwy	3.5	4		3.572	4	3.336	2.2		3	2.209					
Pot Cap-1 Maneuver	197	233	619	208	235	782	1117			1299					
Stage 1	409	424		719	687		:::			-	=	5,5			
Stage 2	683	675		394	419		777					(*)			
Platoon blocked, %	021002	100 400 400	or territory		*******	57-4511015			. *						
Mov Cap-1 Maneuver	140	194	619	177	196	782	1117		¥	1299		1967			
Mov Cap-2 Maneuver	140	194	200	177	196	:42	141	=	-	=	-	3.5			
Stage 1	404	358	292	710	679		21	- 8	-		12	-			
Stage 2	569	667	724	326	354	-	*	-		1		(3)			
Approach	EB	on our car	5/5/3	WB	THE RIDE	1 3789	NB			SB		Material Science	inche l		
HCM Control Delay, s	19.2			23.2		/ 11	0.3			2	- Control of				10000
HCM LOS	C			C			.0.0			- 2					
Minor Lane/Major Mymt		NBL	NBT	NDD	EBLn1V	VRI n4	SBL	SBT	SBR	in Table	S. Cun		0 (5)	100000	22 700
Capacity (veh/h)		1117	INDI	NONE	270	363	1299	001	JUC	THE REAL PROPERTY.	Carle Line	Self-	all, all hell 200	1 1000170	
HCM Lane V/C Ratio			39.5					•							
		0.01	-	•		0.462		_							
HCM Control Delay (s)		8.3	0		19.2	23.2	8.1	0							
HCM Lane LOS		A	А	3	C	C	A	Α	1.5						
HCM 95th %tile Q(veh)		0	-	7	0.2	2.4	0.4	•							

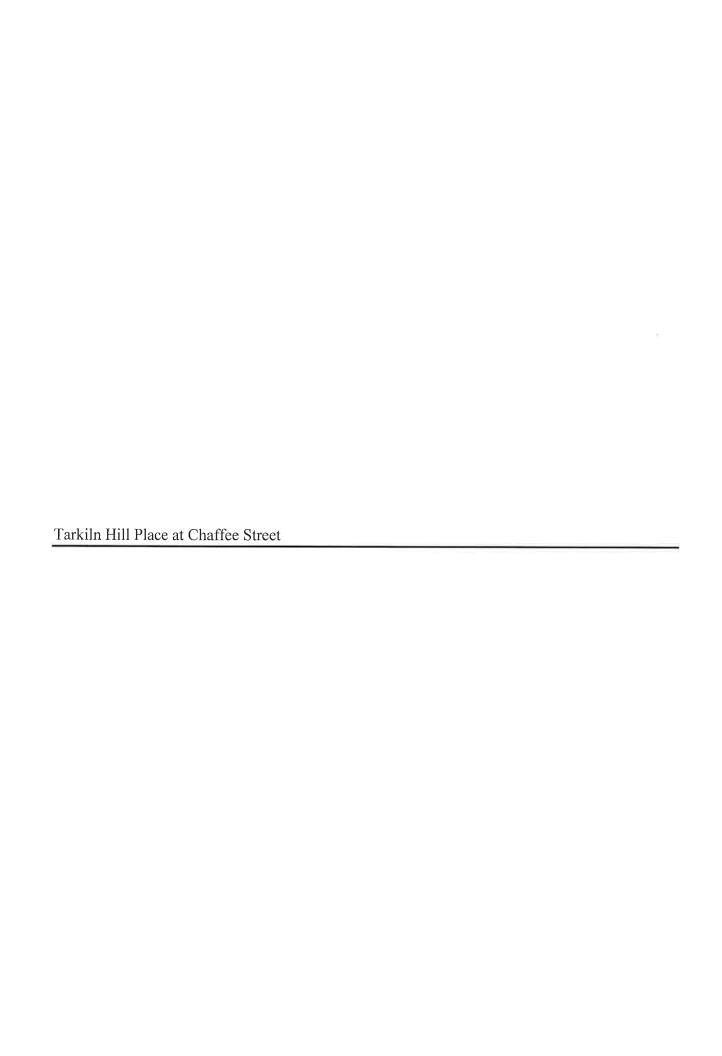
I Was a Division of the Control of t	West Trans	10) =70	02000						and a summer						DOMESTICAL DESCRIPTION OF THE PERSON OF THE
Intersection Int Delay, s/veh	11.7	II TOPEC		plate by	10 100			A SELON		(Olige)	No.	(Guanis			MOU/AIDA
harries and the second				C TOTALS	* (Wasan	MALE VIEWER		NAME OF TAXABLE PARTY.	a de la companya de l	75 JOSE		Marie			
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR			
Lane Configurations		4			4			4			4				
Traffic Vol, veh/h	10		9	41	4	74	10	455	48	86	424	11			
Future Vol, veh/h	10		9	41	4	74	10	455	48	86	424	11			
Conflicting Peds, #/hr	0		0	0	0	0	0	0	0	0	0	0			
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free			
RT Channelized	T _ *		None			None	1		None	i V		None			
Storage Length	}	-	-	-	-	-				-	•				
Veh in Median Storage	,# -	0						0			0				
Grade, %	.5	0		7.	0	:		0	:=:		0				
Peak Hour Factor	61	61	61	77	77	77	70	70	70	80	80	80			
Heavy Vehicles, %	0	0	0	3	0	5	0	2	3	1	2	0			
Mvmt Flow	16	3	15	53	5	96	14	650	69	108	530	14			
Major/Minor	Minor2	SIEWIS (III	5902	Minor1	COLUMN	U-Real W	Major1	5-12.14 h		Major2	OTRI DIS			1007	161 LUN 10
Conflicting Flow All	1516	1500	537	1475	1473	685	544	0	0	719	0	0			
Stage 1	753	753	-	713	713	000	344	-		713		-			
Stage 2	763	747	-	762	760										
Critical Hdwy	7.1	6.5	6.2	7.13	6.5	6.25	4.1	12/		4.11					
Critical Hdwy Stg 1	6.1	5.5	0.2	6.13	5.5	0.20	- H. I	-	-	4.11					
Critical Hdwy Stg 2	6.1	5.5		6.13	5.5			St B		10	n û				
Follow-up Hdwy	3.5	4	3.3	3.527	4	3.345	2.2	-		2.209					
Pot Cap-1 Maneuver	99	123	548	104	128	443	1035			887	-	-			
Stage 1	405	420	340	421	438	443	1033			001					
Stage 2	400	423		396	417		a fi								
Platoon blocked, %	400	423		390	417					1.5		5.5			
Mov Cap-1 Maneuver	63	99	548	84	103	443	1035		:5	887		5			
	63	99			103	443	1035		=		1.5	•			
Mov Cap-2 Maneuver			(*)	84		(**)	-		-		*	-			
Stage 1	396	347	101	411	428		-	•			-				
Stage 2	302	413	261	315	344					mi	-				
Approach	EB	F45/4		WB	To all the	14 1 70	NB		N DE	SB	VA.II III		my.	1070	E DIME
HCM Control Delay, s	53.8			99.4			0.2			1.6					
HCM LOS	F			F											
Minor Lane/Major Mvm		NBL	NBT	MPD	EBLn1V	VPI n1	SBL	SBT	SBR			- X-			
			INDI	NON	THE PERSON NAMED IN COLUMN 1	153P EFLOORATION	× 17124111	ומט	ODA	0 = 1	A LUI	VEID 12 M	Mary Harrison		7.23 KH 3
Capacity (veh/h)		1035		(8)	107	171	887								
HCM Lane V/C Ratio		0.014	-	-		0.904		-	*						
HCM Control Delay (s)		8.5	0	-	53.8	99.4	9.6	0	*						
HCM Lane LOS		A	Α	-	F	F	Α	Α	2						
HCM 95th %tile Q(veh)		0	-	-	1.3	6.7	0.4	-							

Intersection	Tale!	1116.3		101 M	(SIEVE		245 (15)	Heat Police		The Course		K. T.	
Int Delay, s/veh	7.5												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			€\$			4		
Traffic Vol, veh/h	2	1	4	31	10	77	9	233	29	126	414	18	
Future Vol, veh/h	2	1	4	31	10	77	9	233	29	126	414	18	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized			None			None		100	None		C I N	None	
Storage Length		*	*	×		-	(#)	120	7	-	19	-	
Veh in Median Storage	,# -	0			0	24	1.51	0			0	na v v	
Grade, %	-	0		-	0	- 2	-	0			0		
Peak Hour Factor	44	44	44	65	65	65	73	73	73	77	77	77	
Heavy Vehicles, %	0	0	0	8	0	4	0	4	0	1	13	7	
Mvmt Flow	5	2	9	48	15	118	12	319	40	164	538	23	
Major/Minor N	Minor2		# al	Minor1	HE CO		Majort	Macelean	101000	Majora	T-000	- T. T. T. C. S. S.	
Conflicting Flow All	1308	1261	550	1246	1050		Major1	C		Major2	0	netalbini	E) M. Elentinessisiii Mil
•	878				1252	339	561	0	0	359	0	0	
Stage 1		878	=	363	363		•		v :				
Stage 2 Critical Hdwy	430 7.1	383	- 60	883	889	0.04	4.4	121			11		
Critical Hdwy Stg 1	6.1	6.5 5.5	6.2	7.18 6.18	6.5 5.5	6.24	4.1	*		4.11			
Critical Hdwy Stg 2	6.1	5.5		6.18	5.5			-					
Follow-up Hdwy	3.5	3.5	3.3	3.572	3.5	3.336	2.2			2.209			
Pot Cap-1 Maneuver	138	172	539	146	174	699	1020			1205			
Stage 1	345	368	555	644	628	099	1020	•		1205			
Stage 2	607	616		332	364			· · · · · · · · · · · · · · · · · · ·	*		100		
Platoon blocked, %	007	010		332	304								
Mov Cap-1 Maneuver	88	136	539	119	137	699	1020			1205	4	(E	
Mov Cap-1 Maneuver	88	136	333	119	137	033	1020		•	1200		37.	
Stage 1	340	295	_	634	619		-			•			
Stage 2	484	607	_	260	292				- â	-	4 16		
Stage 2	404	007		200	292		-	أسا	ı		7. 4 1		
Approach	EB	Misself	Service.	WB	YE's	N. HE	NB		110000	SB		120 F 187	
HCM Control Delay, s	26			43.3			0.3			1.9			
HCM LOS	D			Ε									
Minor Lane/Major Mvmt	Mala,	NBL	NBT	NRR F	BLn1V	VBI n1	SBL	SBT	SBR	V Total			WATER BLANCE
Capacity (veh/h)		1020			187	266	1205		5511			- III-II II (0/2)	
HCM Lane V/C Ratio		0.012	90		0.085		0.136	8	2				
HCM Control Delay (s)		8.6	0		26	43.3	8.5	0	2				
HCM Lane LOS		Α	A		D	43.5 E	0.5 A	A	2				
HCM 95th %tile Q(veh)		0	-	_	0.3	4.5	0.5	_	-				
		U			0.0	7.0	0.0						

Intersection	1 / A	la su	S. Carre		3 - E 100		(No. of	(E) (H)	The L	Tikket k		also ser	BA FA		PANT S
Int Delay, s/veh	29.2														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	AT USE		
Lane Configurations		43	ē		4			4			4				
Traffic Vol, veh/h	11	2	10	44		80	11	530	52	93	492	12			
Future Vol, veh/h	11	2	10	44	4	80	11	530	52		492	12			
Conflicting Peds, #/hr	0	0	0	0		0	0	0	0		0	0			
Sign Control	Stop	Stop	Stop			Stop		Free	Free	Free	Free	Free			
RT Channelized			None			The second second			None			None			
Storage Length		_			-			-		_		-			
Veh in Median Storage		0			0			0			0				
Grade, %	-	0			723	-		0		-	0				
Peak Hour Factor	61	61	61	77		77	70	70	70		80	80			
Heavy Vehicles, %	0	0	0	3		5	0	2	3	1	2	0			
Mymt Flow	18	3	16	57	5	104	16	757	74	116	615	15			
WWITETIOW	10		10	37	3	104	10	101	14	110	010	13			
Major/Minor	Minor2		in the second	Minor1	STORE IN		Major1	E00	541C	Major2	Envision	Love.		H-FORES	I South
Conflicting Flow All	1736	1718	623	1690	1688	794	630	0	.0	831	0	0			
Stage 1	855	855	-	826	826		-			001		_			
Stage 2	881	863		864	862			-				921			
Critical Hdwy	7.1	6.5	6.2	7.13	6.5	6.25	4.1			4.11					
Critical Hdwy Stg 1	6.1	5.5	0,2	6.13	5.5	0.20	79.1			7913		(22)			
Critical Hdwy Stg 2	6.1	5.5	-	6.13	5.5						1.6	-			
Follow-up Hdwy	3.5	4	3.3	3.527	4	3.345	2.2			2.209					
Pot Cap-1 Maneuver	69	91	490	73	95	383	962	ì		806					
Stage 1	356	378	430	365	389	300	302			000	1.0				
Stage 2	344	374		347	375		0 1 .		=:			:•:			
Platoon blocked, %	344	31.4	•	347	3/3	-			•						
	20	60	400	EE	70	202	060	*	-	000	::00	·			
Mov Cap-1 Maneuver	39	69	490	~ 55	72	383	962	•		806					
Mov Cap-2 Maneuver	39	69	-	~ 55	72		**	-	-	-	2				
Stage 1	345	294	-	354	377	3			•	1.0		170			
Stage 2	240	362		258	292	=	-	•	=	0,23	5 7 5	350			
Approach	EB		A. HOLV	WB		VIII W	NB		Va Au	SB	A=15114				- Si 10624
HCM Control Delay, s	13111-223	-		283.6	-		0.2	PARTIE VALUE		1.6					CI CILITA
HCM LOS	107.0			F			0.2			1.0					
TOW LOO	- Ú														
Minor Lane/Major Mvm	t	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR	J. 1913	Col Inc.	16-66	V. 100 1 1 1 1	mital d	100
Capacity (veh/h)		962	-	- 2	69	120	806	12	1/25						
HCM Lane V/C Ratio		0.016	_		0.546				-						
HCM Control Delay (s)		8.8	0	-	107.6		10.2	0							
HCM Lane LOS		A	A		F	F	В	A	-						
HCM 95th %tile Q(veh)		0.1			2.3	11.3	0.5								
		0.1	Charles .		2.0	1170	0.0		-						
Votes		2000	113/13		583	123	ST ST	THE Y	6°70'8	720	107-1			12 7 E I	200
: Volume exceeds cap	acity	\$: De	lay exc	eeds 3	00s -	t: Com	outation	Not De	fined	*: All r	najor v	olume ir	platoon		

Intersection		1/11/283			150			VIST.	F3.45			NI GAME	il Richi D	453.4	
Int Delay, s/veh	10.1														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	STOUT IS		
Lane Configurations		4			4			4			4				
Traffic Vol, veh/h	5	2	4	33		77	9	235	30	126	424	28			
Future Vol, veh/h	5	2	4	33	14	77	9	235	30	126	424	28			
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0			
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free			
RT Channelized	0 10 10	r Liji 💌	None			None	-	100	None	(4)	100	None			
Storage Length	~	S =	2	2	2	2	7.4	:0	-	-	12	~			
Veh in Median Storage	,# -	0	NY E		0		72	0		-	0	1. 3.			
Grade, %	2	0			0			0			0	¥			
Peak Hour Factor	44	44	44	65	65	65	73	73	73	77	77	77			
Heavy Vehicles, %	0	0	0	8	0	4	0	4	0	1	13	7			
Mvmt Flow	11	5	9	51	22	118	12	322	41	164	551	36			
Major/Minor N	Minor2		TO MILE	Minor1	#E/N30	Duality of	Major1	alwis.	B. 31	Major2	THE SHEET			S. OTHER	Mills Beet
Conflicting Flow All	1334	1284	569	1271	1282	343	587	0	0	363	0	0	III WEST TO SE	COUNTRY	
Stage 1	897	897	-	367	367	040	307	-	-	303	-	-			
Stage 2	437	387	-	904	915		^								
Critical Hdwy	7.1	6.5	6.2	7.18	6.5	6.24	4.1	-		4.11	5				
Critical Hdwy Stg 1	6.1	5.5	0.2	6.18	5.5	0.24	7.1			-					
Critical Hdwy Stg 2	6.1	5.5	7-0-	6.18	5.5										
Follow-up Hdwy	3.5	4			4	3.336	2.2			2.209		- /			
Pot Cap-1 Maneuver	132	166	525	141	167	695	998			1201	1 1				
Stage 1	337	361	-	640	626	-	330		- 2	1201					
Stage 2	602	613	in Living	323	354					-	_				
Platoon blocked, %	002	010		020	004				- A	A. 118					
Mov Cap-1 Maneuver	80	130	525	113	131	695	998			1201					
Mov Cap-2 Maneuver	80	130	020	113	131	-	000			1201					
Stage 1	332	287	7/11	630	617						-				
Stage 2	475	604		249	282						-	-			
E bayes and a subject	410	004		240	202		-1-								
Approach	EB	Williams	Massall	WB	I Washin	WAR.	NB	8 67 1	//*-U/	SB		n Sulat Fil		30,190	
HCM Control Delay, s	39.5			58.5			0.3			1.8					
HCM LOS	Е			F											
Minor Lane/Major Mvmt		NBL	NBT	NBRI	EBLn1V	VBLn1	SBL	SBT	SBR						
Capacity (veh/h)		998	-	-	129	243	1201	-							7,100
HCM Lane V/C Ratio		0.012	3.00			0.785		-							
HCM Control Delay (s)		8.7	0		39.5	58.5	8.5	0							
HCM Lane LOS		A	Ä		E	F	Α.	A							
HCM 95th %tile Q(veh)		0	-		0.7	5.8	0.5	~							
.c oour ware aftern)					J.1	0.0	0.0	-							

Intersection	W18/9	185.1	VE IS A	5867	II.			W 27	STEV HE	(Date of		1957 E.	THE R		ARU a	当人は多り
Int Delay, s/veh	54.6															
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR			1	A STATE
Lane Configurations		4			4			4			4					
Traffic Vol, veh/h	25		10	46	7	80	11	544	52	93	499	19				
Future Vol, veh/h	25	7	10	46	7	80	11	544	52	93	499	19				
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0				
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free				
RT Channelized	1		None			None			None	-		None				
Storage Length	-	_				:=		-	74	- 12	- 2	¥				
Veh in Median Storage	e,# -	0	100	. Ne	0			0			0					
Grade, %	· · · · · · · · · · · · · · · · · · ·			V=	0		-	0			0					
Peak Hour Factor	61	61	61	77	77	77	70	70	70	80	80	80				
Heavy Vehicles, %	0	0	0	3	0	5	0	2	3	1	2	0				
Mvmt Flow	41	11	16	60	9	104	16	777	74	116	624	24				
Major/Minor	Minor2		of the same	Minari	olimine.	105 m V 100	Malaud	10000000	FIGURE 1	M-12-0		ri-francisco	Self-Self-	11110753	Ministra	Second Sec
				Minor1	4700		Major1	TENENS		Major2	0	A CONTRACTOR OF THE PERSON OF	MHY MI	E31 75		OT BU
Conflicting Flow All	1771	1751	636	1728	1726	814	648	0	0	851	0	0				
Stage 1	868	868		846	846	*	-	-		-	-	-				
Stage 2	903	883		882	880	0.05		-	2		-	- 8				
Critical Hdwy	7.1	6.5	6.2	7.13	6.5	6.25	4.1			4.11						
Critical Hdwy Stg 1	6.1	5.5		6.13	5.5		-	, 5 7			•					
Critical Hdwy Stg 2	6.1	5.5	-	6.13	5.5			1000								
Follow-up Hdwy	3.5	4	3.3		4	3.345	2.2		-	2.209		2.6				
Pot Cap-1 Maneuver	66	87	481	69	90	373	947	*	* ×	792						
Stage 1	350	372		356	381		*	14		¥	\ <u>=</u>					
Stage 2	335	367		340	368	PAR.			. 2							
Platoon blocked, %	1212	95/24	1180	8330	065				-		*					
Mov Cap-1 Maneuver	~ 34	65	481	~ 47	67	373	947	- 19	•	792						
Mov Cap-2 Maneuver	~ 34	65	•	~ 47	67	-				=	1.00	25				
Stage 1	338	286	*	344	368			= 115	*	-	5.92	1963				
Stage 2	228	355	-	243	283		*				:#:	###				
Approach	EB	14.50		WB	(IIII)	St. 42.524	NB			SB	BESSEY.	HINTE	eveni.	SE WIN	20108	(0,000)
HCM Control Delay, s\$			S	422.7	0 1		0.2			1.6					-	
HCM LOS	F			F			0.2			,,,,						
		91/2009							ELECTRICITY OF THE PARTY OF THE							
Minor Lane/Major Mvm	ıt	NBL	NBT	NBR E	BLn1V		SBL	SBT	SBR		500.00	, matter	2119		831 ₂₁₂	10 30
Capacity (veh/h)		947			49	102	792									
HCM Lane V/C Ratio		0.017				1.693		*								
HCM Control Delay (s)		8.9	0	-\$	403.7\$		10.3	0								
HCM Lane LOS		Α	Α	*	F	F	В	Α	160							
HCM 95th %tile Q(veh)		0.1	*		6.5	13.6	0.5		-							
Notes			GSHA	-multis				THE REAL				N. S.	RIE E		/ = jls	
 Yolume exceeds cap 	pacity	\$: De	lay exc	eeds 30	Us -	F: Com	outation	Not De	fined	*: All r	najor v	olume ir	platoc	n		



0.9														
0.9														
EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Maria A	E SING	J. S.S.Y
	4			4			4			4				
0	2	. 0	17	10	9	0	0	- 1	2	0	0			
0	2	0	17	10	9	0	0	1	2	0	0			
0	0	0	0	0	0	0	0	0	0	0	0			
Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free			
-		None		44 7	None			None	-		None			
5 7 .5						.(#)	; * :	(#)	:=0:		-			
# -	0	1.7	U./ () ¥	0	10 De		0	1.00		0				
(4)	0		=	0	7 i	247		-	14.5		4			
50		50	73		73	25		25	25		25			
		11/2/2												
0	4	0	23	14	12	0	0	4	8	0	0			
linor2		K Dig	Minor1	Jan 21	To said	Major1	200	1	Major2		7			MA
31	20	0	20	18	2	0	0	0	4	0	0			
16	16		2	2		(*)	1.00	:-	VII *					
15	4	*	18	16	: 4:	3 4 0	-	18	:=	-				
7.1	6.5	6.2	7.17	6.5	6.2	4.1			4.1	- 2	- 2			
6.1	5.5	٠	6.17	5.5	:				2	2	- 43			
						- 2	- 4							
		3.3			3.3	2.2		-	22		-			
						Und .		, IT 0		7.				
					-	:=0		-	-					
		111							15°5 .					
.0.0			000	000			-	_						
955	874	0.27	220	876	1088		- V	7 - 3	1631	= 1				
		120					2				1721			
							-	-	- 3					
											10.50			
000	007		010	002										
EB	175		WB	0 0		NB	VIE H		SB	dip log	0.000	ONE OF THE	A DEFE	Ding:
						0			7.2					
(- :			**											
	NBL	NBT	NBR E	BLn1W	VBLn1	SBL	SBT	SBR	31 J. W.	No. of the	netta 8		Aguaries.	8 235
	143	- 1		-										
		(2)	20	-			_	2						
	0						0							
		5 m												
	- '	195/1				0								
	0 0 0 Stop 50 0 0 0 4inor2 31 16 15 7.1 6.1 6.1 3.5 982 1009 1010 955 955 1009 983 EB	0 2 0 2 0 0 0 Stop Stop # - 0 - 0 50 50 0 0 4 Minor2 31 20 16 16 15 4 7.1 6.5 6.1 5.5 6.1 5.5 6.1 5.5 6.1 5.5 6.1 5.5 874 982 878 1009 886 1010 897 955 874 1009 882 983 897 EB	0 2 0 0 0 2 0 0 0 0 0 Stop Stop Stop - None None 0 - - 0 - 50 50 50 0 0 0 0 0 4 0 16 16 - 15 4 - 7.1 6.5 6.2 6.1 5.5 - 6.1 5.5 - 6.1 5.5 - 3.5 4 3.3 982 878 - 1009 886 - 1010 897 - 955 874 - 955 874 - 955 874 - 955 874 - 955 874 - 983 897 -	0 2 0 17 0 2 0 17 0 0 0 0 0 Stop Stop Stop Stop - None None 0 0 50 50 50 73 0 0 0 7 0 4 0 23 Minor2 Minor1 31 20 0 20 16 16 - 2 15 4 - 18 7.1 6.5 6.2 7.17 6.1 5.5 - 6.17 6.1 5.5 -	0 2 0 17 10 0 2 0 17 10 0 0 0 0 0 0 Stop Stop Stop Stop Stop None # - 0 0 50 50 50 50 73 73 0 0 0 7 0 0 4 0 23 14 Minor2 Minor1 31 20 0 20 18 16 16 - 2 2 15 4 - 18 16 7.1 6.5 6.2 7.17 6.5 6.1 5.5 - 6.17 5.5 6.1 5.5	0 2 0 17 10 9 0 2 0 17 10 9 0 0 0 0 0 0 0 0 Stop Stop Stop Stop Stop Stop - None - None O - O - O - 50 50 50 73 73 73 0 0 0 0 7 0 0 0 4 0 23 14 12 Minor2	Color			Color	Color	Color	Color	Color

Intersection	Ikins		quente e	San Ba	THE WIE	Media.	(The Section	S W.	1 500		B (2.57	161-113	3750	S. Chang		
Int Delay, s/veh	6.7															
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	172 ki	-01	Auth Si	
Lane Configurations		44			4			4			4					
Traffic Vol, veh/h	1	25	0	0	8	7	1	0	17	11	1	0				
Future Vol, veh/h	1	25	0	0	8	7	1	0	17	11	1	0				
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0				
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free				
RT Channelized	-	1 1-	None		7 N	None	4		None		-	None				
Storage Length	*	;(⊕;	•	₹.	0.00				-	-	-	(Æ:				
Veh in Median Storage,	# -	0			0			0			0	145				
Grade, %	2	0		-	0	-	-	0	=	-	0	-				
Peak Hour Factor	58	58	58	46	46	46	54	54	54	50	50	50				
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0				
Mvmt Flow	2	43	0	0	17	15	2	0	31	22	2	0				
												76				
Major/Minor M	linor2			Minor1		趣,利用	Major1		N	Major2	Worth.				5 THE	(14)
Conflicting Flow All	82	81	2	88	66	16	2	0	0	31	0	0				
Stage 1	46	46		20	20		24.55	-	-	77	-	74				
Stage 2	36	35	-	68	46	2	- 4	2	2	2	16	848				
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1	- 1					
Critical Hdwy Stg 1	6.1	5.5		6.1	5.5		-	ĕ	- 8			9				
Critical Hdwy Stg 2	6.1	5.5	J.	6.1	5.5	100	150			1/5	1.5	8.00				
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	=		2.2	(sec)	0.5				
Pot Cap-1 Maneuver	910	813	1088	902	829	1069	1634		1 10	1595		:::				
Stage 1	973	861	- 34	1004	883		*	-	7.00		:					
Stage 2	985	870		947	861			14	T (e)	194	243	(2)				
Platoon blocked, %								116	114		141	120				
Mov Cap-1 Maneuver	873	801	1088	855	817	1069	1634	1/2		1595						
Mov Cap-2 Maneuver	873	801	3	855	817		-		-	-	-	150				
Stage 1	972	849	7	1003	882	× :		1,7								
Stage 2	951	869	5	886	849	π.	•		3.5			(*)				
Approach	EB		18	WB	V. 76		NB			SB		NI COL	35-51		884 I	His
HCM Control Delay, s	9.7			9.1			0.4			6.7						
HCM LOS	А			А												
Minor Lane/Major Mvmt		NBL	NBT	NBR E	BLn1V	VBLn1	SBL	SBT	SBR		Se fill	a Table			51/16	3.80
Capacity (veh/h)		1634			804	918	1595									
HCM Lane V/C Ratio		0.001	-	-												
HCM Control Delay (s)		7.2	0		9.7	9.1	7.3	0								
HCM Lane LOS									(#)							
HCM 95th %tile Q(veh)		0			0.2	0.1	0	-	145							
Minor Lane/Major Mymt Capacity (veh/h) ICM Lane V/C Ratio ICM Control Delay (s) ICM Lane LOS		1634 0.001 7.2 A	- - 0 A	A NBR E	804 0.056 9.7 A	918 0.036 9.1 A	SBL 1595 0.014 7.3 A	0 A	SBR	6.7						

Intersection	HEST V				SIND N	1000						diam't a		EUV.	
Int Delay, s/veh	8.0														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Lus Fill	L. Os	
Lane Configurations		4			4			4			4				
Traffic Vol, veh/h	0	2	0	18	10	10	0	0	1	2	0	0			
Future Vol, veh/h	0	2	0	18	10	10	0	0	1	2	0	0			
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0			
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free			
RT Channelized		-	None	14		None	(4)		None	*		None			
Storage Length	-	-	-	::•:	100	(4)	-	52	¥	E	¥.	¥			
Veh in Median Storage	,# -	0	-	-	0		2	0		= ===	0	-			
Grade, %	-	0	(4)	-	0			0	•		0				
Peak Hour Factor	50	50	50	73	. 73	73	25	25	25	25	25	25			
Heavy Vehicles, %	0	0	0	7	0	0	0	0	0	0	0	0			
Mvmt Flow	0	4	0	25	14	14	0	0	4	8	0	0			
Major/Minor N	Minor2			Minor1	HIER	(1) (a) (b) (b)	Major1	510-114	Carrie 1	Major2	SERVICE D			Very Man	A STATE
Conflicting Flow All	32	20	0	20	18	2	0	0	0	4	0	0	US FAIRL	10000	SELECTION OF SELEC
Stage 1	16	16	U	20	2	0 -		-	-	4	11602	-			
Stage 2	16	4	-	18	16	W 1	1					2			
Critical Hdwy	7.1	6.5	6.2	7.17	6.5	6.2	4.1			4.1		3.5			
Critical Hdwy Stg 1	6.1	5.5	0.2	6.17	5.5							17.0			
Critical Hdwy Stg 2	6.1	5.5	.70	6.17	5.5					0.00	()				
Follow-up Hdwy	3.5	4	3.3	3.563	4	3.3	2.2			2.2		•			
Pot Cap-1 Maneuver	981	878	5.5	981	880	1088	2.2	-	AL EVI	1631					
Stage 1	1009	886		1008	898	1000		-	•	1031	•				
Stage 2	1009	897	-	988	886				-		- And	10 720			
Platoon blocked, %	1003	031		300	000					10.115					
Mov Cap-1 Maneuver	954	874	- 2		876	1088				1631					
Mov Cap-1 Maneuver	954	874			876	1000						:51			
Stage 1	1009	882		1008	898		5.					1.00			
	981	897	-	979	882			•		**		:#0			
Stage 2	901	097		919	002			ı.i.		-		÷(-			
Approach	EB			WB		47/4	NB	. (7.EH	106	SB		BH 7452	they may	N SAME I	CANAN SE
HCM Control Delay, s				- 2			0			7.2		100			
HCM LOS	141			-											
Minor Lane/Major Mymt		NBL	NBT	NRRE	BLn1W	/Bl n1	SBL	SBT	SBR	UP HOLDE					
Capacity (veh/h)		HUL	1101	HUNL		DLITT	1631		COIL			Mineral III	SEES IN		
HCM Lane V/C Ratio		Ž.					0.005	1,54							
HCM Control Delay (s)		0					7.2	0	252						
HCM Lane LOS			•		1.5			0	(#)						
HCM 95th %tile Q(veh)		Α			1.65	11.00	A 0	Α	₩() =0.0						
HOW JOHN WHIE CALVELL)		-	-				U	-	340						

Intersection	no W				WHY SI			New York		2 A P S	5.76	Taks.	STEEL ST
Int Delay, s/veh	6.7												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		44			4			4			4		
Traffic Vol, veh/h	1	25	0	0	8	8	1	0	18	12	- 1	0	
Future Vol, veh/h	1	25	0	0	8	8	1	0	18	12	1	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized			None	200		None			None		V .	None	
Storage Length			-		-	0,6	0,40	*	(4)	-	- 4	*	
Veh in Median Storage,	# -	0	. 4		0			0			0	21	
Grade, %	-	Ü	=	-	0	2	12	0	==1	- 2	0	<u> </u>	
Peak Hour Factor	58		58	46	46	46	54	54	54	50	50	50	
Heavy Vehicles, %	0		0	0	0	0	0	0	0	0	0	0	
Mvmt Flow	2	43	0	0	17	17	2	0	33	24	2	0	
Major/Minor N	linor2	Williams.		Minor1		10 a	Major1	50	10.2,10	Major2			
Conflicting Flow All	88		2	93	71	17	2	0	0	33	0	0	
Stage 1	50			21	21		-	21 - S		-			
Stage 2	38		12	72	50		-	*	g.		2		
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1			
Critical Hdwy Stg 1	6.1	5.5		6.1	5.5						-	-	
Critical Hdwy Stg 2	6.1	5.5	X 18	6.1	5.5		-						
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2			2.2		g e s	
Pot Cap-1 Maneuver	902	807	1088	895	823	1068	1634			1592		-	
Stage 1	968	857	•	1003	882	:#C	-			*		246	
Stage 2	982	868	4	943	857	- 4	(40)	_ 5			-	100	
Platoon blocked, %											-	020	
Mov Cap-1 Maneuver	862	794	1088	848	810	1068	1634	7		1592	1,10		
Mov Cap-2 Maneuver	862	794		848	810		-	,	-			157.5	
Stage 1	967	844	-	1002	881		-				151	875	
Stage 2	946	867	576	881	844							(120)	
Approach	EB			WB		VEITS.	NB	U VSER		SB			
HCM Control Delay, s	9.8	411.111.114.12		9.1			0.4			6.7			
HCM LOS	A			Α			311			0.7			
Minor Lane/Major Mvmt		NBL	NBT	NRR	EBLn1V	VBI n1	SBL	SBT	SBR	W-102	15010		1170 ST 100 HT 1, 400
Capacity (veh/h)		1634		-	796	921	1592	-	0011				
HCM Lane V/C Ratio		0.001			0.056		0.015	9					
HCM Control Delay (s)		7.2	0),T()	9.8	9.1	7.3	0	2				
HCM Lane LOS		Α	A		3.0 A	Α.Τ	7.3 A	A					
HCM 95th %tile Q(veh)		0		120	0.2	0.1	0		2				
TOWN JOHN JOHNE Q(VEII)		U	-	(=)	0.2	0.1	U						

Intersection			STREET,			NAME OF STREET	39.4	Parison Territori	SEP OF				在 。有限 的 直接处理不是从事
Int Delay, s/veh	0.8												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	- 0	2	0	18	10	10	0	0	2	2	0	0	
Future Vol, veh/h	0	2	0	18	10	10	0	0	2	2	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized			None	No.		None			None	40	1018	None	
Storage Length	-	14		~	2	-	-	-	-	:=:	¥	≘	
Veh in Median Storage	е,# -	0			0	1/25	7-5	0		-	0	- 18	
Grade, %		0		8	0			0		•	0		
Peak Hour Factor	50	50	50	73	73	73	25	25	25	25	25	25	
Heavy Vehicles, %	0	0	0	7	0	0	0	0	0	0	0	0	
Mvmt Flow	0	4	0	25	14	14	0	0	8	8	0	0	
Major/Minor	Minor2	0.93		Minor1	H (49, W)		Major1	X V 0 V	1	Vajor2		SEN S	
Conflicting Flow All	34	24	0	22	20	4	0	0	0	8	0	0	
Stage 1	16	16		4	4		-						
Stage 2	18	8	145	18	16		2	-		-			
Critical Hdwy	7.1	6.5	6.2	7.17	6.5	6.2	4.1	4.3		4.1	ē	S.E.	
Critical Hdwy Stg 1	6.1	5.5	-	6.17	5.5	-	- 10.1			-			
Critical Hdwy Stg 2	6.1	5.5	- X.	6.17	5.5			100		60	J. TOU	1-023	EVEN NA EXCENSE
Follow-up Hdwy	3.5	4	3.3	3.563	4	3.3	2.2		-	2.2		-	
Pot Cap-1 Maneuver	978	873		978	878	1085	76.2	25/4	I ILIY	1625		ell,	A THE WAY THE PERSON
Stage 1	1009	886	(*)	1005	897			_	2	-	2	7-2	DESCRIPTION OF THE PROPERTY OF THE
Stage 2	1006	893	15	988	886		1 2		2				
Platoon blocked, %	110000	- C-7/		0.75					-				
Mov Cap-1 Maneuver	951	869		VI .	874	1085	1	I reg		1625	u.	1-1	
Mov Cap-2 Maneuver	951	869			874			-	-	-			
Stage 1	1009	882		1005	897			-	7.74	le La			
Stage 2	978	893	*	979	882		*		*	*		3.00	
Approach	EB	1910 811	north and	WB	A LONG	75 P	NB		REFER	SB		0.4	
HCM Control Delay, s					100		0			7.2			
HCM LOS	**			122						1.4			
Minor Lane/Major Mym	£	NBL	NBT	NRPE	BLn1W	/RI n1	SBL	SBT	SBR	E VE	STATE		
Capacity (veh/h)	Company of the		INDI	NDN L	DLITTY	PULIT	1625	001	חטט		F. BA	EUG DAS	
HCM Lane V/C Ratio			7.0	7.0	- 5	•							
HCM Control Delay (s)		0		:50		-	0.005	-					
HCM Lane LOS		0				2.	7.2	0	•				
		Α	:-	-	**	•	A	Α	-				
HCM 95th %tile Q(veh)		-	•				0	-	1,21				

Int Delay, s/veh	6.4												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			43			4			4	00,11	10.710
Traffic Vol, veh/h	1		0	0	8	8	- 1	0	21	12	1	0	
Future Vol, veh/h	1		0	0	8	8	1	0	21	12	1	0	
Conflicting Peds, #/hr	0		0	0	0	0		0	0	0	0	0	
Sign Control	Stop		Stop	Stop	Stop	Stop		Free	Free	Free	Free	Free	
RT Channelized			None	Milit		None			None			None	
Storage Length	-): ;=	*	-	-	-	14	(4)	INPESWEES:	240	32	A/95590.5	
Veh in Median Storage,	# -	0	III DIE		0			0	-	151	0	0.2	
Grade, %	-	0	2	_	0	-	-	0			0	-	
Peak Hour Factor	58	58	58	46	46	46	54	54	54	50	50	50	
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0	
Mvmt Flow	2	43	0	0	17	17	2	0	39	24	2	0	
Major/Minor M	inor2	DY LAY		Minor1	STORY.	Tip	Major1			Major2	E (Thin)	04,791	品的道
Conflicting Flow All	91	93	2	96	74	20	2	0	0	39	0	0	
Stage 1	50			24	24	11834							
Stage 2	41	43	24	72	50		120	-	_			2	
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1	(Q.) G		
Critical Hdwy Stg 1	6.1	5.5		6.1	5.5		-			-			
Critical Hdwy Stg 2	6.1	5.5		6.1	5.5		N 35 -					- 72	
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2			2.2			
Pot Cap-1 Maneuver	898	801	1088	891	820	1064	1634		(#	1584	B	1.5	
Stage 1	968	857		999	879) (- A)	-	:=	2		2	×	
Stage 2	979	863	-	943	857	A vis	81" 88	āL,	1.78		- 1		
Platoon blocked, %								4	2		-	-	
Mov Cap-1 Maneuver	858	788	1088	844	807	1064	1634	QL ' 3	X Par	1584			
Mov Cap-2 Maneuver	858	788	•	844	807	-	-						
Stage 1	967	844	N. F	998	878							100	
Stage 2	943	862		881	844	-	*			*	*		
Approach	EB	12210	52119	WB	SOR SOR	last last	NB			SB	SLAT!		835
HCM Control Delay, s	9.8			9.1	T.E.		0.3			6.7			
HCM LOS	Α			Α									
Minor Lane/Major Mymt		NBL	NBT	NBR F	EBLn1V	VBLn1	SBL	SBT	SBR	900 100	LE CO		
Capacity (veh/h)		1634		100	790	918	1584	-	-	-			avendil.
HCM Lane V/C Ratio		0.001			0.057				- 5				
HCM Control Delay (s)		7.2	0		9.8	9.1	7.3	0	16				
HCM Lane LOS		A	A		Α	A	Α.	A					
HCM 95th %tile Q(veh)		0	- 1	27.7	0.2	0.1	0						



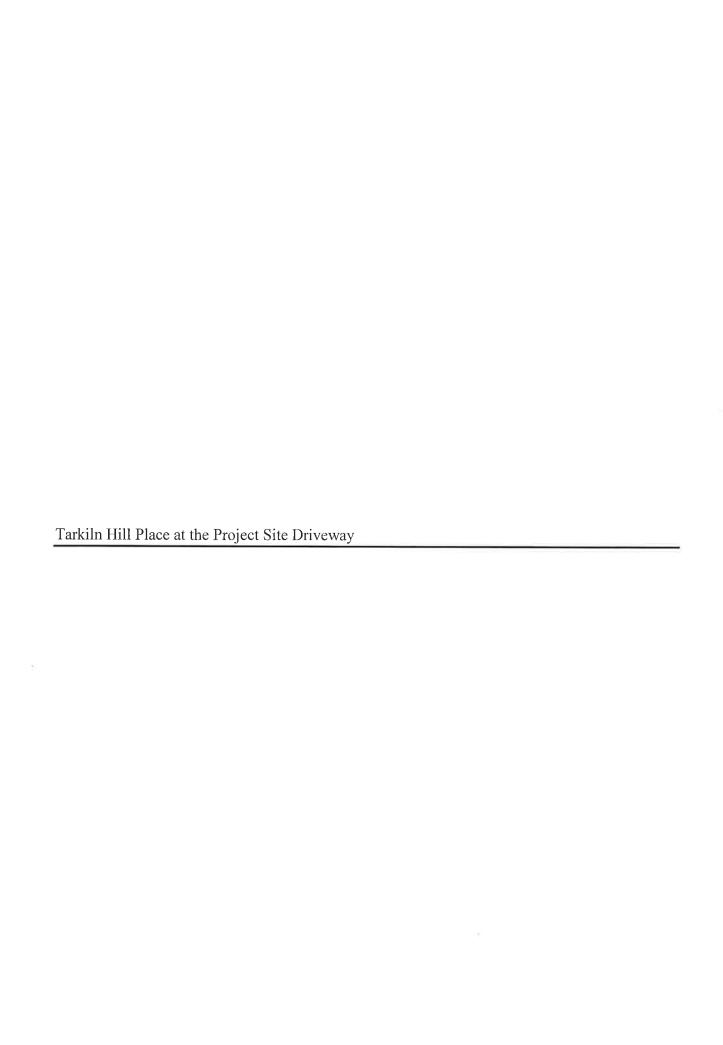
Intersection	KALLA.	NAME OF	min Nati	W.C. 2.	6,37	GE LIDE	
Int Delay, s/veh	0.3						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	你是想要是你。"然后我看到了我说,这个人的女
Lane Configurations	**	5		4	\$		
Traffic Vol, veh/h	3	4	13	271	449	12	
Future Vol, veh/h	3	4	13	271	449	12	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	11112	- Augustina		None		None	
Storage Length	0	-	-	-	7.	145	
Veh in Median Storage		1/03		0	0		
Grade, %	0	2		0	0	125	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	
Mymt Flow	3	4	14	295	488	13	
WWITELIOW	J	. 4	14	255	400	13	
Major/Minor	Minor2	FORDY	Major1	Villed N	Major2		
Conflicting Flow All	818	495	501	0	:0	0	
Stage 1	495			-		000	
Stage 2	323	ALCOHOL:	2	-		-	
Critical Hdwy	6.42	6.22	4.12		1 1		
Critical Hdwy Stg 1	5.42	0.22	7,12	72			
Critical Hdwy Stg 2	5.42				07.16		
Follow-up Hdwy		3.318	2.218			150	TO SHOULD BE A SAME SHOULD BE SEEN SHOULD SHOULD SEEN SHOULD SHOULD SEEN SHOULD SHOULD SEEN SEEN SHOULD SEEN SHOUL
Pot Cap-1 Maneuver	346	575	1063		10.2		
	613	3/3	1003			186	
Stage 1				S#4		:*:	
Stage 2	734		-			-	
Platoon blocked, %	0.10		4000	-	120	140°	
Mov Cap-1 Maneuver	340	575	1063		*		
Mov Cap-2 Maneuver	340	•	1.0		*	:50	
Stage 1	603			100			
Stage 2	734	0 	3.00	(2)	::::	:• ?	
			Wiles			-	
Approach	EB		NB	1766	SB	the filter	
HCM Control Delay, s	13.2		0.4		0		
HCM LOS	В						
Minor Lane/Major Mvm	10000000	NBL	NBT	DI n1	SBT	SBR	
				444	001	JUN	
Capacity (veh/h)		1063	•		*	*	
HCM Lane V/C Ratio		0.013		0.017	•		
HCM Control Delay (s)		8.4	0	13.2			
HCM Lane LOS		Α	Α	В		*	
HCM 95th %tile Q(veh)		0	-	0.1	-		

Intersection		STEEL STATE				THE	
Int Delay, s/veh	0.7	,					
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y	W.		4	7		
Traffic Vol, veh/h	17	19	10	593	546	9	
Future Vol, veh/h	17		10	593	546	9	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop		Free	Free	Free	Free	
RT Channelized		None		None	10/8 e	None	
Storage Length	0		¥	#		75	
Veh in Median Storage	01,000			0	0	- 8	
Grade, %	0		2	0	0		
Peak Hour Factor	92		92	92	92	92	
Heavy Vehicles, %	2		2	2	2	2	
Mvmt Flow	18	21	11	645	593	10	
Major/Minor	Minor2		Major1	W 45 N	Vajor2		
Conflicting Flow All	1265		603	0		0	
Stage 1	598			1111		0.3 es	
Stage 2	667		:4:	74:		- 1	
Critical Hdwy	6.42		4.12	C 10 2		1	THE RESERVE OF THE PARTY OF THE PARTY OF THE PARTY.
Critical Hdwy Stg 1	5.42	(-	3				
Critical Hdwy Stg 2	5.42		S 11 (3)	245		1005	
Follow-up Hdwy	3.518	3.318	2.218		(*)		
Pot Cap-1 Maneuver	187	502	975				
Stage 1	549			741		-	
Stage 2	510	9- 4		19 - C	100	- 2	
Platoon blocked, %				:20	127	-	
Mov Cap-1 Maneuver	184	502	975	49	1 3		
Mov Cap-2 Maneuver	184		*	-		7	
Stage 1	539		/ V .*	1	184.		
Stage 2	510		(5)	. 20			
Approach	EB	Walley.	NB		SB		
HCM Control Delay, s	20.2		0.1		0		
HCM LOS	С						
Minor Lane/Major Mym	it	NBL	NBT E	BLn1	SBT	SBR	
						-	
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)	t	975 0.011 8.7 A	0 A	276 0.142 20.2 C 0.5		# *	



Intersection	(A) (A)		Die Ting		15,410	
Int Delay, s/veh	2.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	N/W	
Traffic Vol, veh/h	- 8	0	14	37	0	3
Future Vol, veh/h	8	0	14	37	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None	0.1*		FEI	VALUE STATES
Storage Length	-		¥	*	0	
Veh in Median Storage,	# 0			0	0	
Grade, %	0	-	-	0	0	12
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	0	15	40	0	3
Major/Minor N	lajor1	REP VI	Major2	45,00	Minor1	
Conflicting Flow All	0	0	9	0	79	9
Stage 1		tud x	115	2.	9	
Stage 2	4	-	2	2	70	-
Critical Hdwy	2		4.12		6.42	6.22
Critical Hdwy Stg 1	-	2	- 100.00		5.42	
Critical Hdwy Stg 2	J. J.	102			5.42	
Follow-up Hdwy	-		2.218		3.518	3.318
Pot Cap-1 Maneuver	X41.	ik der	1611		924	1073
Stage 1			-		1014	
Stage 2				(45	953	
Platoon blocked, %	2	2		848		
Mov Cap-1 Maneuver			1611	- 4	915	1073
Mov Cap-2 Maneuver	- 2	-	-		915	-
Stage 1	WILL.	7 2	110	200	1014	
Stage 2					943	(#)
Tage 2					040	W =
Approach	EB	5000	WB		NB	1
HCM Control Delay, s	0		2		8.4	
HCM LOS			-		A	
Minor Lane/Major Mvmt	١	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1073			1611	
HCM Lane V/C Ratio		0.003	3.	-	0.009	
HCM Control Delay (s)		8.4			7.3	0
HCM Lane LOS		Α	547	997	Α	Α
HCM 95th %tile Q(veh)		0		(*)	0	
		_			•	

					The San of	
Intersection	07	100	THEN	1000	TENT	
Int Delay, s/veh	2.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	13			4	N/	
Traffic Vol, veh/h	26	0	10	27	0	16
Future Vol, veh/h	26	0	10	27	0	16
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized		None	mile.	None	1977	None
Storage Length	+		*		0	
Veh in Median Storage,	# 0			0	0	
Grade, %	0	2	¥	0	0	::e:
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	28	0	11	29	0	17
Major/Misson M	ni and		(Aniora)	San Jan	Minard	W-258
	ajor1		Vajor2		Minor1	00
Conflicting Flow All	0	0	28	0	79	28
Stage 1	-	1, 2,5,			28	
Stage 2	-	*	00000		51	0.00
Critical Hdwy		¥.	4.12		6.42	6.22
Critical Hdwy Stg 1	_	¥	2	-	5.42	(A=:
Critical Hdwy Stg 2			W-T	-	5.42	
Follow-up Hdwy	-		2.218		3.518	3.318
Pot Cap-1 Maneuver		-	1585	100	924	1047
Stage 1	*			18	995	/(e /
Stage 2	*		10.5		971	
Platoon blocked, %	×	*				
Mov Cap-1 Maneuver	-	-1 = 4	1585	All IN	918	1047
Mov Cap-2 Maneuver	-	12	2	145	918	100
Stage 1		2	100	ATIONS	995	12
Stage 2	-		÷		964	0.0
Approach	EB	WOT THE	WB		NB	The Wilder
Approach	0	SECTION .	2		8.5	SOUTH PROPERTY.
HCM Control Delay, s	U				Α.5	
HCM LOS					A	
Minor Lane/Major Mvmt	1	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1047		-	1585	1.00
HCM Lane V/C Ratio		0.017	2	-	0.007	-
HCM Control Delay (s)		8.5	- 1	-	7.3	0
HCM Lane LOS		Α		-	Α	Α
HCM 95th %tile Q(veh)		0.1			0	



YUST.	FORTH ST	10 12	THE ST	The CLER		
0.4						-
WBL	WBR	NBT	NBR	SBL	SBT	
*		1			4	
0	1	1	0	0	18	
0	1	1	0	0	18	
0	0	0	0	0	0	
Stop	Stop	Free	Free	Free	Free	
	None		None	*	None	
0	=	¥	*	-	(-)	
	11.5	0		2	0	
	-		2	-		
				2		
0	1	1	0	0	20	
linor1					S Y	Super State of the State of the Control of the Cont
	1	0	0	1	0	
1		12	11 10			
	-	i les	16	720	120	
6.42	6.22	-		4.12		
	÷		-	-	350	
5.42	-	1.50	10 (9 0 4			
	3.318	V.	10.00		(*)	
	1084	100		1622		
	:(•:		((≠)		(€):	
1003		- 1	140	1		
		7.0	72		==0	
	1084			1622	*	
		-	-	17.5	2.0	
	100	•	•	(8)		
1003	100		:•:	**		
WB		NB		SB		
		0		0		
Α				- 3		
	NBT	NBRV	VBLn1	SBL	SBT	
		30	1084	1622	- 8	
	(20)	=	0.001			
			8.3	0		
	90					
	*	*	0	0		
	WBL 0 0 0 0 Stop 0 # 0 0 92 2 0 linor1 21 1 20 6.42 5.42 5.42 5.42 3.518 996 1022 1003 996 996 1022 1003 WB 8.3	WBL WBR O 1 0 0 1 0 0 Stop Stop - None 0 - # 0 - 92 92 2 2 0 1 Inor1 1 1 - 20 - 6.42 6.22 5.42 - 5.42 - 5.42 - 5.42 - 1003 - 996 1084 1022 - 1003 - WB 8.3 A	WBL WBR NBT 0 1 1 0 0 0 0 Stop Stop Free - None - 0 # 0 - 0 92 92 92 2 2 2 2 0 1 1 Inor1 Major1 21 1 0 1 20 6.42 6.22 - 5.42 5.42 5.42 5.42 5.42 1003 996 1084 - 1022 1003 WB NB 8.3 0 A NBT NBRW	WBL WBR NBT NBR	WBL WBR NBT NBR SBL WF D <	WBL WBR NBT NBR SBL SBT

Intersection			WHEE PERSON	10.00	15 88	58 775
Int Delay, s/veh	1.1					No. of London
		N. Venez	THE PERSON NAMED IN	UNITED DESCRIPTION	WICE COMM	Militare
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	N/F		7>			4
Traffic Vol, veh/h	0	3	19	0	0	1
Future Vol, veh/h	0	3	19	0	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	W 125	None		None	C L West	None
Storage Length	0	•	ž		14	*
Veh in Median Storage	e, # 0	01 P.T.	0	3 77/52	11	0
Grade, %	0	-	0		-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	3	21	0	0	1
Management of the second	41 // 5				•	
	Minor1		Major1		Major2	
Conflicting Flow All	22	21	0	0	21	0
Stage 1	21				*	
Stage 2	1	-	-	0.5	O.P.4	
Critical Hdwy	6.42	6.22		9 59	4.12	958
Critical Hdwy Stg 1	5.42			X.#2	(*)	
Critical Hdwy Stg 2	5.42				100	- 1
Follow-up Hdwy	3.518	3.318	240	-	2.218	191
Pot Cap-1 Maneuver	995	1056			1595	
Stage 1	1002		/ Z	621	,000	120
Stage 2	1022		W 72			II (a)
Platoon blocked, %	1022				350	
	005	1050		974	4505	
Mov Cap-1 Maneuver	995	1056	•		1595	**
Mov Cap-2 Maneuver	995	10,000	(#)		(€6	190
Stage 1	1002	•		•		
Stage 2	1022	(1 4 5	:*:	340	(=):	(3)
Approach	WB	V0)=400 -	NB	MUL	SB	W
HCM Control Delay, s	8.4	-	0	-	0	
HCM LOS	A		v		U	
TIOW LOG	WSA A					
Minor Lane/Major Mvm	ıt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-		1056	1595	
HCM Lane V/C Ratio		s ⊕ 2		0.003		
HCM Control Delay (s)		**	140	8.4	0	3
HCM Lane LOS		943	-	A	Ă	2
HCM 95th %tile Q(veh)			- 1	0	0	-
medical roll of toll)				Ÿ		